



Functional Servicing Report

River's Edge Subdivision Town of Grand Valley Draft Plan of Subdivision #22T-12001

GMBP File: 104104

October 2023

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RIVER'S EDGE SUBDIVISION
TOWN OF GRAND VALLEY
DRAFT PLAN #22T-12001
OCTOBER 2023
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1.0 INTRODUCTION

GM Blue Plan Engineering Limited has prepared this Functional Servicing Report to address the site servicing and stormwater management requirements for the site in support of the Draft Plan of Subdivision prepared by GSP Group.

The proposed development is located in the northeast end of Grand Valley, adjacent to the Grand River. **Figure 1** shows the location of the proposed development and the surrounding area. The boundaries of the development include the Grand River to the north and east, an existing wetland to the southeast, Highway No. 25 and existing residential development to the west, and Scott Street and residential lands to the south. The applicant owns additional lands on the north side of the Grand River, which are not included in the current development.

2.0 EXISTING CONDITIONS

3.1 Land Use

Under existing conditions, the northerly portion of the site is used for agricultural purposes while the southerly portion was used for aggregate extraction. The areas formerly used for aggregate extraction are no longer in production. The easterly portion of the site consists mainly of wooded areas and slope towards the Grand River.

The lands to the west and south of the site have been developed as residential lands consisting of mostly single family housing. An existing townhome condominium block is located southeast of the site. The Grand River borders the site at the westerly property boundary followed by additional wooded areas and agricultural lands further west of the Grand River.

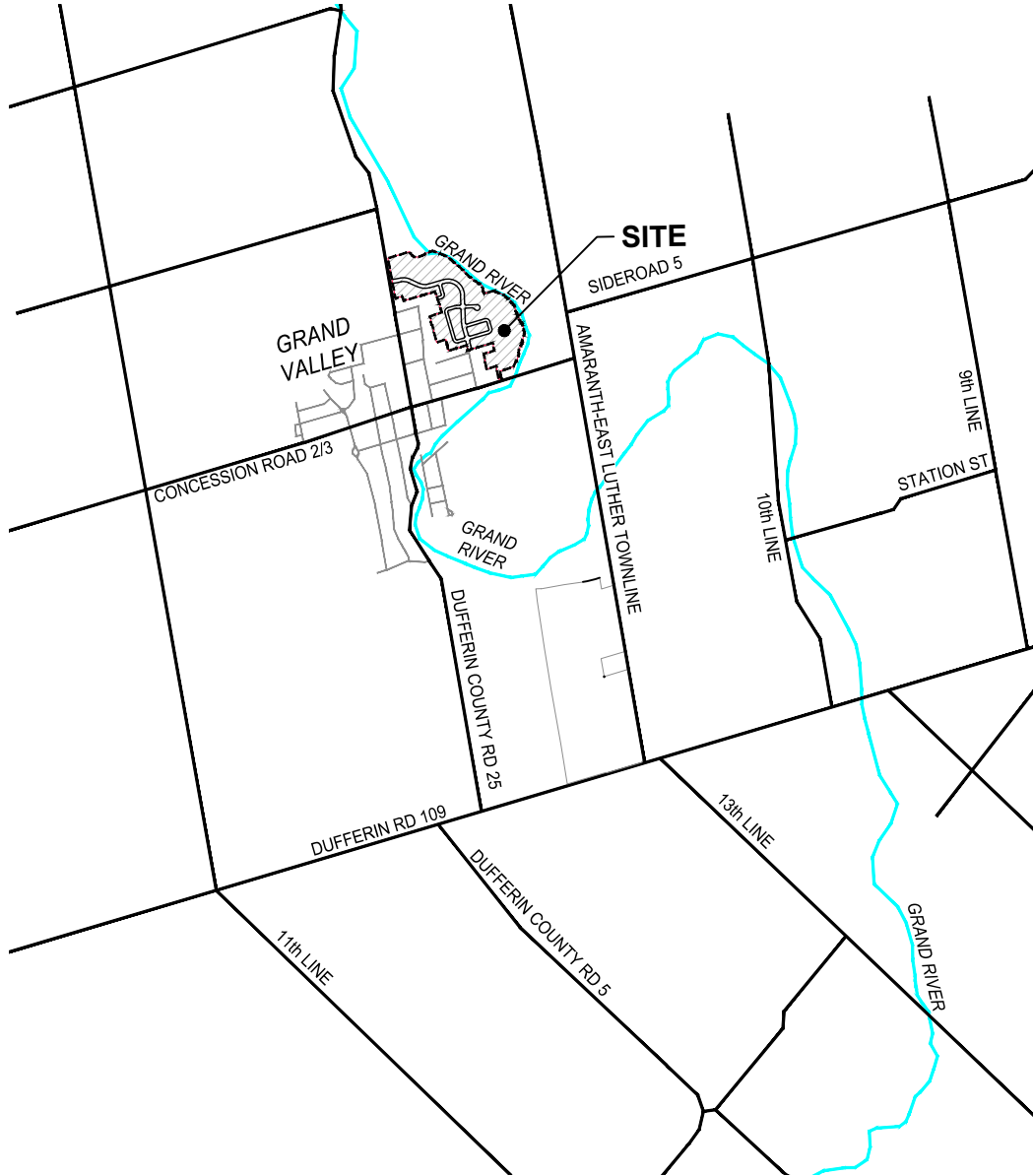
3.2 Topography

Topography across the site varies, with grades ranging from 4% to in excess of 15% along the banks adjacent to the Grand River. There is approximately 25m of fall across the site in the northwest to southeast direction. The site elevation ranges from approximately 481m at the northwest end of the site then falls to about 455m towards the easterly and southeasterly portions of the site.

The majority of the site drains overland in the easterly direction towards the Grand River. The central portion of the site drains in the easterly direction to an existing wetland located at the southeast portion of the site ultimately discharging to the Grand River. Flows exceeding the capacity of the wetland will then sheetflow in the easterly direction to the Grand River.

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LEGEND

----- PROPOSED SUBDIVISION

SITE LOCATION

Figure No. 1



FILE:W:\Gueph\104-2004\104104\5 Work In Progress\Drafting\SHEETS\FIGURES\104104-SiteLocation.dwg LAYOUT:SITE LOCATION PLAN
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3.3 Soils

Peto MacCallum Ltd. completed a geotechnical investigation of the property in April 2009 followed by a slope assessment report in 2012. Based on the 2009 geotechnical investigation, there appears to be between 600 and 800mm of topsoil at the north and west portions of the site and close to 200mm thick topsoil towards the easterly portion of the site. The subsurface site soils consist of a mix of clayey silt till, sand and gravelly sand deposits. The clayey silt till materials are found predominantly at the north end of the property while the sands and gravels are found towards the southerly end.

JLP Services Inc. completed a follow-up geotechnical investigation at the southerly end of the site in 2022. Based on the JLP report, the topsoil layer at the southernly end of the site ranges between 50 to 900mm at boreholes MW1, MW2 and MW3. Subsurface topsoil buried at a depth of 2.4m and 0.3m was encountered at boreholes MW3 and MW4 respectively. Deposits of loose to compacted fill have also been encountered at boreholes MW1, MW3 and MW4. The fill consists of sandy silts, some gravel, trace clay and some organics. It is recommended that the areas of buried topsoil be removed and replaced with engineered fill and the areas of loose fill be excavated, placed and compacted.

Both the Peto MacCallum and JLP geotechnical investigations recommend an impermeable liner for the proposed stormwater management facility construction due to the high permeability of the site soils at the proposed stormwater management facility location. The JLP investigation recommends a minimum 1m thick clay liner or approved equivalent synthetic liner. Furthermore it is noted that the clay liner could be constructed from some of the clayey till material on site.

Copies of the slope assessment report by Peto MacCallum (2012) and geotechnical investigations by Peto MacCallum (2009) and JLP Services Inc. (2022) are included in **Appendix A**.

3.4 Groundwater

As a component of the geotechnical investigation, Peto MacCallum Ltd. installed groundwater monitoring wells in three of the nine boreholes.

Initial readings taken from the three monitoring wells showed groundwater measurements below the bottom of the well (>5.05m below ground surface) at Borehole 3 located at the north end of the site, groundwater approximately 4.8 metres below ground surface at Borehole 4 located at the center of the site and groundwater approximately 1.1 metres below ground surface at Borehole 9 located at the southeasterly portion of the site.

JLP Services installed monitoring wells in each of the four boreholes as part of the 2022 geotechnical investigation. MW4 located at the southeast portion of the site, south of Scott Street was dry (groundwater level >3.1m below ground surface), while MW1, MW2 and MW3 located north of Scott Street showed groundwater water levels at 6m, 3m and 6m below ground surface, respectively.

Additional details regarding groundwater elevations are included in the geotechnical investigations by Peto MacCallum (2009) and JLP Services Inc. (2022) in **Appendix A**.

GM BluePlan Engineering Limited has been recording groundwater elevations to capture the seasonal high groundwater levels across the site. From 2009 to 2015, manual groundwater readings have been recorded at BH 3, 4 and 9. In 2022, dataloggers have been installed to record the groundwater elevations at BH 3, 4 and 9 as well as MW 1 to 4 and MH 101 to 103. Hydrographs showing the groundwater readings as of Fall 2023 are included in **Appendix B**.

3.0 PROPOSED DEVELOPMENT

The proposed development generally consists of single family and semi-detached lots, a townhome block, apartment block, two park blocks, open space areas, internal roadways and a stormwater management block. **Figure 2**, illustrates the proposed Draft Plan of Subdivision prepared by GSP Group.

An extension of Bielby Street from Scott Street to Highway No. 25 will form the main connection for this development. A third road connection to the existing road network will be provided by extending Luther Road from the existing cul-de-sac to the Bielby Street extension.

As illustrated on the Draft Plan of Subdivision, a pedestrian walkway connection has been provided from Crozier Street to the proposed park block.

3.1 Site Grading

The site grading for the proposed residential lots, open spaces, internal roads and stormwater management facilities is shown on the Preliminary Grading Plans. The grade and elevation of the internal streets is controlled by the elevation of the sanitary sewer outlet and existing roadway elevations at Main Street, Scott Street and Luther Road.

The proposed site grading will match the existing elevations along the west, north and southerly property limits. At the easterly portions of the site, the proposed development will match the existing elevations along the existing woodlot and Grand River Floodplain.

Internal roadways are graded with slopes ranging from 0.5% to 5%, while lots are generally graded with slopes ranging from 2% to 6% per the Town of Grand Valley Engineering Standards. 3:1 transition slopes are proposed in rear yard areas of deeper lots to accommodate the grade relief across the site. Due to the high elevation differential across the site, retaining walls are proposed in areas where transition slopes into existing ground elevations would exceed the maximum allowable grade of 3:1.

It is noted that some of the walkout lots located along the easterly site perimeter may have basements taller than the standard 2.43m (8') to provide larger elevation differential across the lots and mitigate the height and locations of 3:1 transition slopes and retaining walls.

3.2 Streets

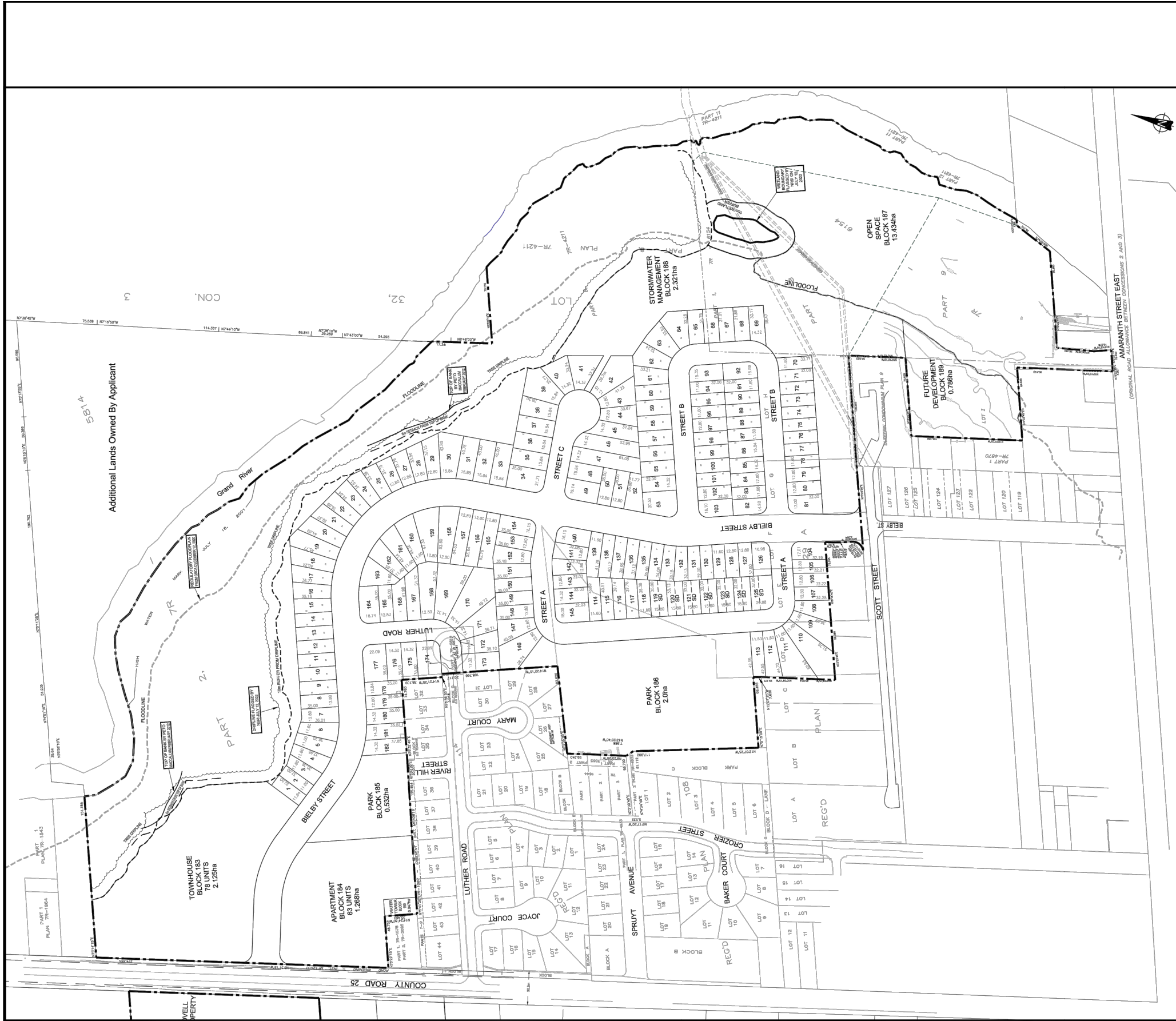
The internal roadways will be constructed as an urban cross section complete with concrete curb and gutter. The Luther Road extension, Street A, B and C are considered local roads and will be constructed using the Town of Grand Valley standard 20m wide urban cross section, while Bielby Street is considered a collector road and will be constructed using a 26m wide urban cross section.

Concrete sidewalks, with a 1.5 metre width, will be constructed per Town of Grand Valley standard cross-section.

3.3 Water Supply

The proposed development will be serviced with three watermain connections to the existing municipal watermain system. Connections will be made at the following locations; existing 200mm diameter watermain on Bielby Street at Scott Street, 150mm diameter watermain at the Luther Road Cul-de-sac and 300mm diameter watermain on Highway 25. It is anticipated that the existing 300mm diameter watermain on Highway 25 will be extended in the northerly direction from its current terminus at the existing water tower to the Highway 25 and Bielby Street intersection.

A 200mm diameter watermain will be extended through the site along Bielby Street from Scott Street to Highway 25. The remainder of the watermain sizes on internal roadways will be 150mm in diameter.



DRAFT PLAN OF SUBDIVISION THOMASFIELD HOMES RIVER'S EDGE

PART OF LOT 31, CONCESSION 3
 GEOGRAPHIC TOWNSHIP OF EAST LUTHER
 ALL OF BLOCK C
 REGISTERED PLAN 114
 TOWN OF GRAND VALLEY
 COUNTY OF DUFFERIN



PLANNING | URBAN DESIGN | LANDSCAPE ARCHITECTURE
 gpp@gsp.ca
 Date: August 28, 2023
 Drawn By: MN
 Checked By: JMS
 Project No: 22045
 Draw File Name: gsp2023mk.dwg

ADDITIONAL INFORMATION
 (UNDER SECTION 5(1) OF THE PLANNING ACT)
 INFORMATION REQUIRED BY CLAUSES a,b,c,d,e,f,g) and I ARE AS SHOWN ON THE DRAFT PLAN.
 i) Silt, Clayey Silt, Silty Gravel, Gravelly Sand, Sand, Unsubdivided Silt
 k) All sanitary and storm sewers as required

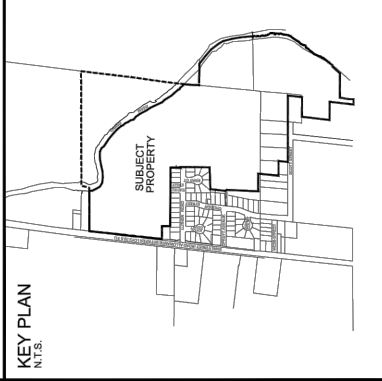
OWNER'S CERTIFICATE
 I AUTHORIZE THE GSP GROUP INC. TO PREPARE AND SUBMIT THIS DRAFT PLAN OF SUBDIVISION TO THE TOWN OF GRAND VALLEY IN ITS RELATIONSHIP TO THE ADJACENT LANDS ARE CORRECTLY SHOWN.

Tom Keenan
 Areas Glen Developments Limited
 DATE

SURVEYOR'S CERTIFICATE
 I CERTIFY THAT THE BOUNDARIES OF THE LAND TO BE SUBDIVIDED AND THEIR RELATIONSHIP TO THE ADJACENT LANDS ARE CORRECTLY SHOWN.

Jessica M. Lewis
 Ontario Licensed Surveyor
 DATE

DESCRIPTION	LOTS/BLOCKS	UNITS	AREA (m ²)
SINGLE DETACHED LOTS			9,271
11.8m	1-6, 52, 70-76, 82-83, 87-88, 108-118, 129-139, 160-163	54	
12.8m	7-29, 42-44, 50-51, 79-81, 84, 97-107, 126-128, 140-143, 146-158, 164-166, 176-179	71	
14.32m	40-46, 173, 175-176, 180-182, 30-39, 48-49, 53, 86, 174, 177	34	
15.84m	119-125	14	0,373
SEMI DETACHED	183	78	2,125
TOWNHOUSES	184	63	1,268
APARTMENT	185-188		2,532
PARK	187		13,434
OPEN SPACE	189		2,321
STORMWATER MANAGEMENT	190		0,786
FUTURE DEVELOPMENT TO BE ADDED TO WATER TOWER ROADS	190		0,047
TOTAL	190	330	36,982ha



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DRAFT PLAN OF
 SUBDIVISION
 Figure No. 2



A 25mm diameter water service lateral will be provided for each dwelling. Fire hydrants are proposed to be placed at a minimum distance of 100m to provide fire protection, in accordance with Town Standards.

RJ Burnside and Associated Ltd. prepared a Master Servicing Plan update for the Town of Grand Valley in 2014. The Master Servicing Plan update identified water distribution system improvements required by the projected 2031 buildout. It is noted that flows from the subject property and other greenfield developments within the Town's urban boundary are included in this study along with proposed intensification to the existing developed areas. It is anticipated that the Town will grow from the 2011 population of 1,482 persons to 6,145 persons by 2031.

The following items are identified to be upgraded by the projected 2031 buildout:

- The current water tower has a capacity of 1,600m³ and would need to be expanded by 1,732m³ for a total volume of 3,332m³ to meet the additional storage demands. A 0.047ha block has been allocated on the Draft Plan of Subdivision to accommodate the water tower expansion.
- Upgrades to the existing water supply system to increase the firm capacity (well supply capacity with the largest well out of service) from 1,963m³/d to 3,792m³/d.

The Master Servicing Plan recommends commencing the upgrades when the current needs exceed 80% of the water distribution system capacity (population reaches 2,900 people). The Class EA for the water supply and storage expansions should be initiated sooner, roughly when the population reaches 2,300 people to allow for processing time.

3.4 Sanitary Sewer

The proposed development will be serviced by a gravity sanitary sewer system connecting to the existing 200 mm diameter sanitary sewer on Bielby Street at the Scott Street intersection. Preliminary sizing indicates that a 200 mm diameter sanitary sewer will provide sufficient capacity for the River's Edge Subdivision. Each dwelling will be serviced via a proposed 125mm diameter sanitary lateral per Town of Grand Valley Standards.

As noted in the section above, RJ Burnside and Associated Ltd. prepared a Master Servicing Plan update for the Town of Grand Valley in 2014. The Master Servicing Plan update identified wastewater collection and treatment system improvements required by the projected 2031 buildout.

The following items are identified to be upgraded by the projected 2031 buildout.

- The Existing Water Pollution Control Plant (WPCP) has been designed for an average flow of 1,244m³/d to service an average population of 2,950 persons. It is anticipated that by the 2031 buildout, the total WPCP capacity could be expanded to 2,547m³/d to service a population of 6,050 people.
- Sewers on Emma Street, Mill Street, Ponsford Street, Amaranth Street and Leeson Street are generally identified to be increased in diameter by one pipe size. The existing sewers are noted to be over capacity by less than 8%. It is noted that the upsizing of the sanitary sewer on Emma Street is currently under construction.
- The Emma Street sanitary pumping station peak flow is estimated to be 106L/s which exceeds the current capacity of 89L/s/

The above noted upgrades would require a Scheduled EA. The Master Servicing Plan recommends assessing the capacity of the wastewater system on an annual basis and initiating the required EA processes well in advance of need just in case if there are any delays.

3.5 Storm Sewer

The storm sewer system on the internal local roads (20m right-of-way) will be sized to convey the 5-year design storm while the storm sewers on collector roads (i.e. Bielby Street – 26m right-of-way) will be sized to convey the 10-year design storm to the stormwater management facility. Major storm events will be conveyed overland through the municipal right of ways to the stormwater management facility.

The stormwater management facility will outlet to the Grand River.

A 150mm diameter storm service is proposed to be installed to each residential unit to provide gravity foundation drainage for the proposed development. Sump pumps are proposed for lots where a gravity foundation connection is not possible – the sump pump will pump the foundation drainage to the storm sewer service lateral. Lots requiring sump pumps will be identified on the preliminary grading and servicing drawings.

150mm diameter subdrains are proposed to be constructed under the curb for the full length of the roadway per Town of Grand Valley Engineering Standards.

4.0 STORMWATER MANAGEMENT CRITERIA

The studies, policies and guidelines used to develop the stormwater management plan are as follows:

- 1) The Stormwater Management Practices Planning and Design Manual, 1994
- 2) Stormwater Management Planning and Design Manual, 2003
- 3) The Interim Stormwater Quality Control Guidelines, 1991
- 4) The Stormwater Quality Best Management Practices Manual, 1991
- 5) The MTO Drainage Management Technical Guidelines, 1989
- 6) The Ontario Urban Drainage Design Guidelines, 1987

The stormwater management criteria for the River's Edge Subdivision, are as follows:

- 1) Post-development runoff generated from site is to be directed to the Grand River. Due to the proximity of the site to the Grand River and to limit the impact of coincidental peak flows from upstream developments, stormwater management quantity controls for the subject property are not required.
- 2) Provide Enhanced level of quality control (80% Total Suspended Solids (TSS) removal) from all runoff discharging from the site.
- 3) Major storm flows are to be routed overland to an appropriate outlet.

The method used to evaluate and design the stormwater management plan is as follows:

A three-hour duration rainfall event was used to generate the mass rainfall data required to model the 2, 5 and 100-year design storms. The Fergus Shand Dam Chicago parameters and the total depth of rainfall for each storm are shown below in **Table 1**.

Table 1: Fergus Shand Dam - Chicago Storm Parameters

	2 Year	5 Year	100 Year
a =	695.047	1459.072	6933.019
b =	6.387	13.690	34.699
c =	0.793	0.850	0.998
r =	0.380	0.380	0.380
t _d =	180.00	180.00	180.00
Rainfall depth (mm)	33.01	49.79	97.92

The Regional Storm (Hurricane Hazel) was also modelled.

The Horton infiltration method was used in the runoff calculations summarized in **Table 2**.

From the Ontario Soil Surveys, Report No. 38, Dufferin County, the soils on site are described as Huron Loam and Burford Loam. The hydrologic soil classifications for these soils are BC and AB, respectively.

The Geotechnical Investigation completed by Peto MacCallum Ltd. (2009) classified the surficial soils as consisting primarily of clayey silt tills with sand and gravel deposits. Due to the relatively poor drainage capacity of clayey silt tills, the Horton Infiltration parameters for a Type D soil have been used in the analysis of the stormwater management system. The typical maximum infiltration rate for a hydrologic Type D soil is 75 mm/hour, as per the MTO Drainage Management Manual.

Table No. 2: MIDUSS Horton Parameters

	Impervious Areas	Pervious Areas
Maximum Infiltration	0.0 mm/hr	75.0 mm/hr
Minimum Infiltration	0.0 mm/hr	5.0 mm/hr
Lag Constant	0.05 hr	0.25 hr
Depression Storage	1.5 mm	5.0 mm

The hydrologic model MIDUSS was used to create the runoff hydrographs and to route the flows through the storage structure.

5.0 STORMWATER MANAGEMENT PLAN

5.1 Pre-Development Conditions

For pre-development conditions analysis purposes, the site was modelled as eight (8) drainage catchments. Four (4) out of the eight (8) drainage catchments represent the subject site while the remaining four (4) represent external areas draining through the site. The pre-development condition drainage catchments are shown on **Figure 3** and described below. The existing conditions MIDUSS computer modelling is attached in **Appendix C**.

Internal Catchments:

Catchment 100 (4.92 hectares, 0% impervious) represents the north-westerly portion of the site. Runoff generated from Catchment 100 sheetflows overland in the northerly direction to the Grand River.

Catchment 101 (8.49 hectares, 0% impervious) represents the north-easterly portion of the site. Runoff generated from Catchment 101 sheetflows overland in the easterly direction to the Grand River.

Catchment 102 (8.68 hectares, 1% impervious) represents the central portion of the site. Runoff generated from Catchment 102 sheetflows overland to an existing wetland located in the southeast corner of the site then ultimately to the Grand River.

Catchment 103 (6.33 hectares, 0% impervious) represents the southern portion of the site. Runoff generated from Catchment 103 sheetflows overland to the Grand River.

External Catchments:

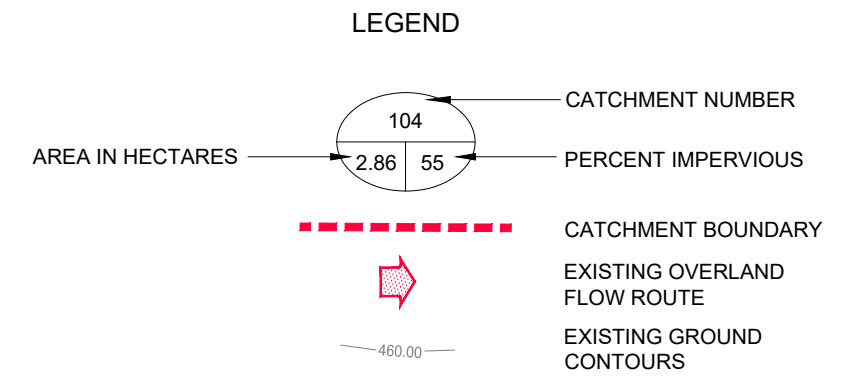
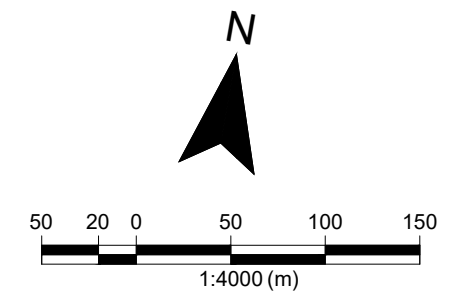
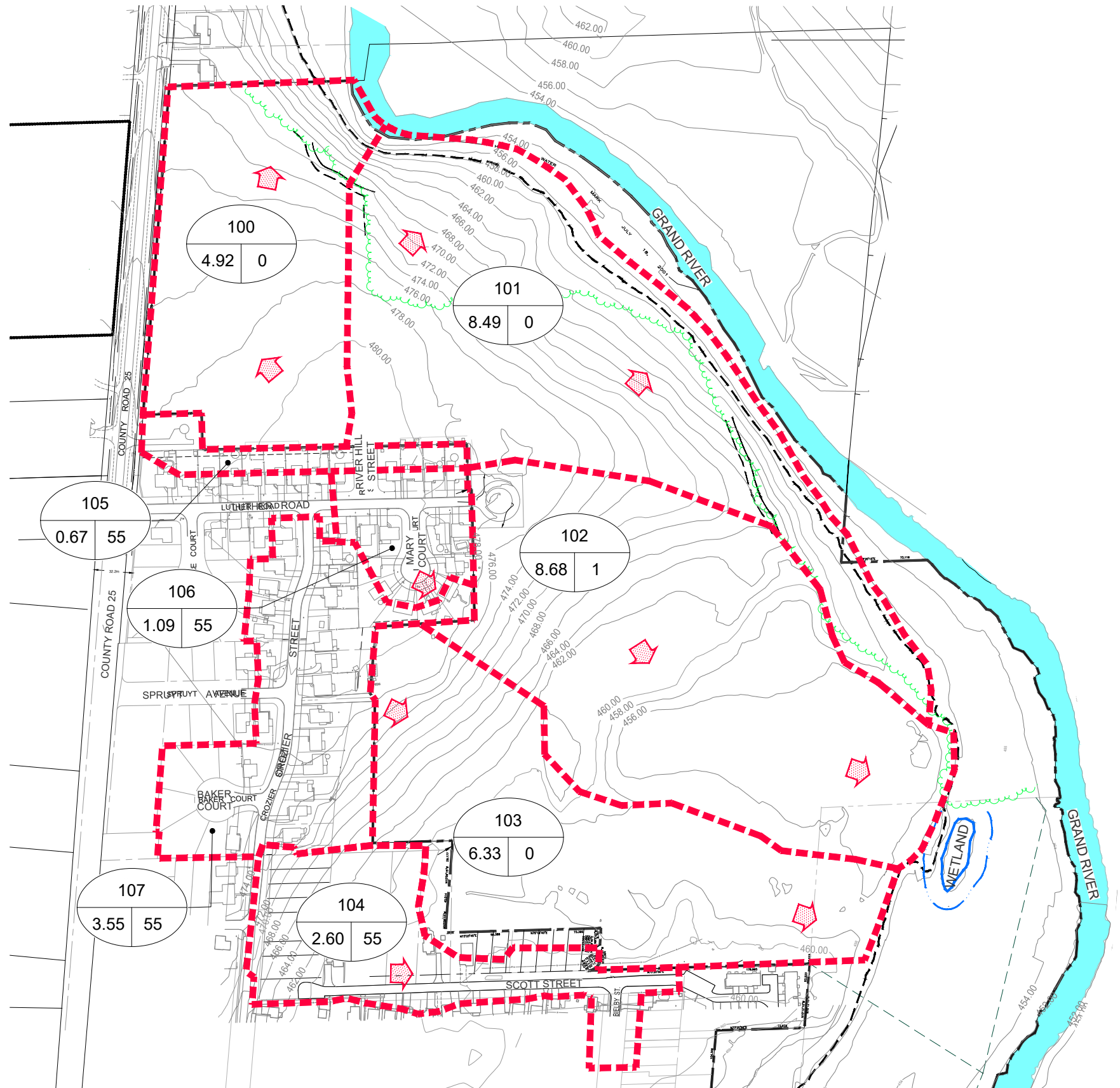
Catchment 104 (2.60 hectares, 55% impervious) represents the existing development on Scott Street. The storm sewer on Scott Street discharges into the site, north of Bielby Street. Runoff from Catchment 104 discharges overland through the site to the Grand River.

Catchment 105 (0.67 hectares, 55% impervious) represents rear yard areas from the existing residential lots located along the north side of Luther Road and the existing water tower site. Runoff generated from Catchment 105 sheetflows overland through the site to the Grand River.

Catchment 106 (1.09 hectares, 55% impervious) represents existing Mary Court and easterly part of Luther Road. The existing storm sewer on Luther Road conveys runoff from the eastern half of Luther Road and Mary Court and discharges to the site at the east end of Luther Road. The drainage area contributing to the storm sewer has been included in Catchment 106. Runoff generated from Catchment 106 sheetflows overland through Catchment 102 to an existing wetland located in the southeast corner of the site then ultimately to the Grand River.

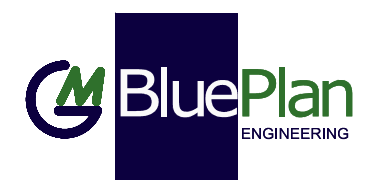
Catchment 107 (3.55 hectares, 55% impervious) represents the existing drainage from the lots on Crozier Street and Baker Court. Runoff generated from Catchment 107 is being discharged overland through the site, to the Grand River.

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PRE DEVELOPMENT
 DRAINAGE PLAN

Figure No. 3



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Table 3 lists the flow rates discharging from each catchment under pre-development conditions.

Table 3: Pre-Development Condition Flow Rates

	2-Year	5-Year	100-Year	Regional Storm
Catchment 100	0.042	0.294	1.192	0.538
Catchment 101	0.093	0.610	2.442	0.896
Catchment 102	0.061	0.439	1.891	0.950
Catchment 103	0.059	0.404	1.633	0.686
Catchment 104	0.300	0.405	0.730	0.282
Catchment 105	0.072	0.104	0.233	0.078
Catchment 106	0.125	0.169	0.301	0.117
Catchment 107	0.397	0.544	0.948	0.371
Total Flow Rate	0.935	2.172	8.805	3.839

5.2 Post-Development Conditions

For post-development analysis purposes, the site was modelled as eleven (11) drainage catchments. The proposed stormwater management facility is designed to have three (3) inlets. Inlet 1 is located at the south portion of the facility accepting flows from Bielby Street, Inlet 2 is located midway though the facility accepting flows from Street B and Inlet 3 is located at the northerly end of the facility accepting flows from Street C. The post-development drainage catchments are shown on **Figure 4** and described below. The post-development MIDUSS computer modelling is attached in **Appendix D**.

Catchments discharging to Inlet 1 (Bielby Street):

Catchment 204 (2.09 hectares, 10% impervious) represents the proposed park block fronting Street A. Runoff generated from Catchment 204 will be directed to the proposed stormwater management facility Inlet 1.

Catchment 205 (4.32 hectares, 65% impervious) represents the southwestern portion of the proposed development. Runoff generated from Catchment 205 will be directed to the proposed stormwater management facility Inlet 1.

Catchment 301 (2.60 hectares, 55% impervious) represents the existing development on Scott Street. The storm sewer on Scott Street discharges to the proposed stormwater management facility at the Inlet 1 location.

Catchment 303 (3.55 hectares, 55% impervious) represents the existing drainage from the lots on Crozier Street and Baker Court. Runoff will be conveyed through the site to the proposed stormwater management facility Inlet 1.

Catchments discharging to Inlet 2 (Street B):

Catchment 203 (2.76 hectares, 60% impervious) represents the southeastern portion of the proposed development. Runoff generated from Catchment 203 will be directed to the proposed stormwater management facility Inlet 2.

Catchments discharging to Inlet 3 (Street C):

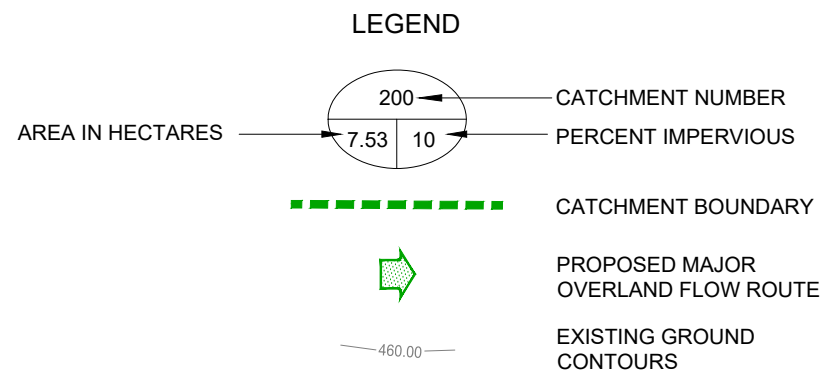
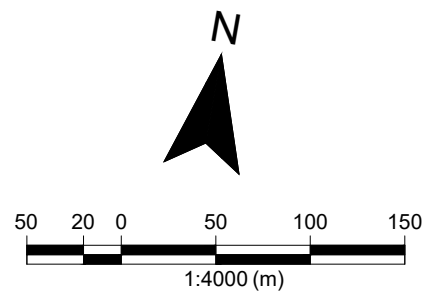
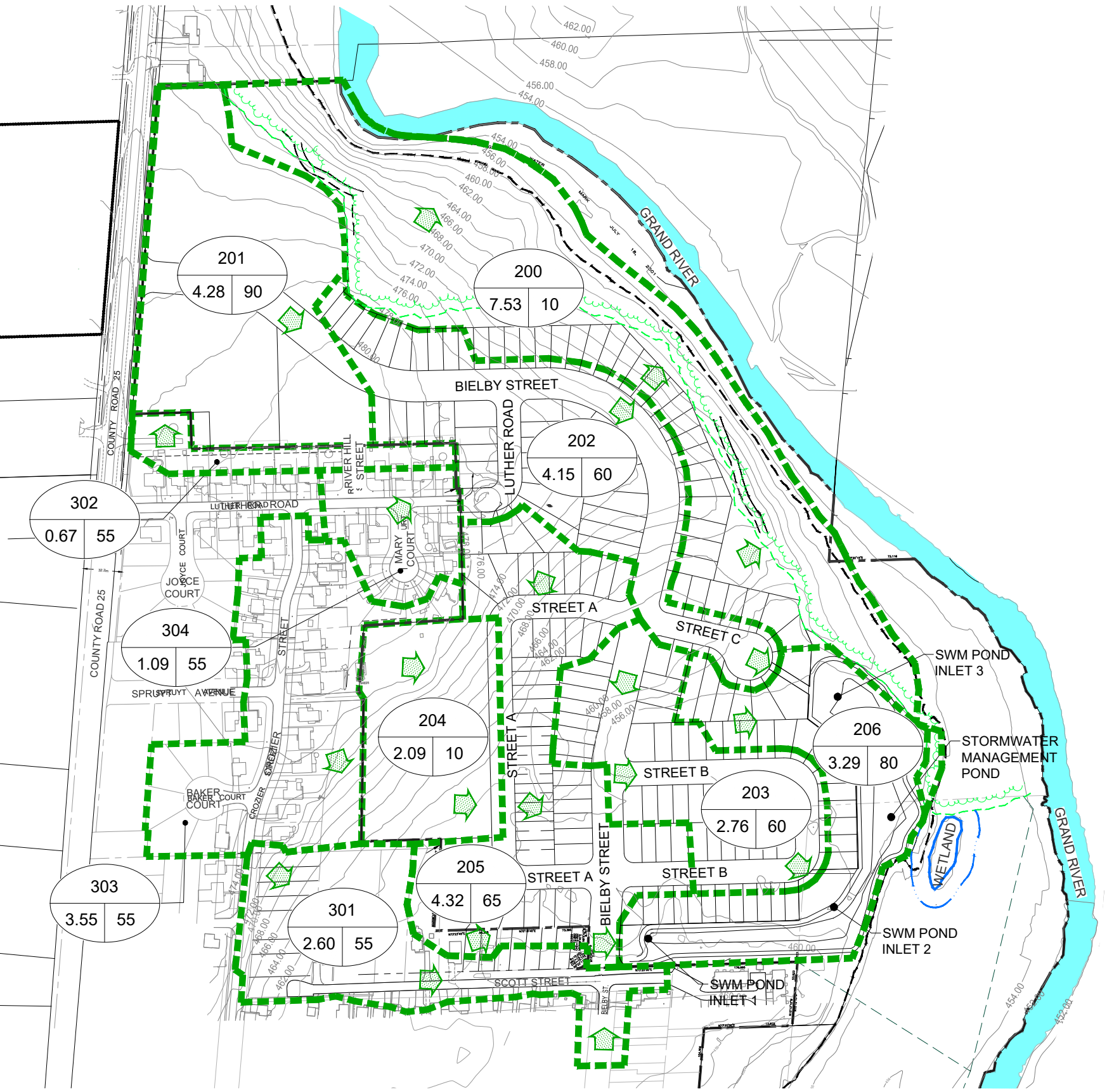
Catchment 201 (4.28 hectares, 90% impervious) represents the townhome and apartment blocks located at northwestern portion of the proposed development. Runoff generated from Catchment 201 will be directed to the proposed stormwater management facility Inlet 3.

Catchment 202 (4.15 hectares, 60% impervious) represents the northeast portion of the proposed development. Runoff generated from Catchment 202 will be directed to the proposed stormwater management facility Inlet 3.

Catchment 302 (0.67 hectares, 55% impervious) represents rear yard areas from the existing residential lots located along the north side of Luther Road and the existing water tower site. Runoff generated from Catchment 202 will sheetflow to the apartment block followed from which it will be directed to the proposed stormwater management facility Inlet 3 via the proposed storm sewer system and roadway network.

Catchment 304 (1.09 hectares, 55% impervious) represents existing Mary Court and parts of Luther Road. The existing storm sewer on Luther Road will be connected to the proposed storm sewer system on site. Runoff generated from Catchment 304 will be directed to the proposed stormwater management facility Inlet 3.

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POST DEVELOPMENT
 DRAINAGE PLAN

Figure No. 4



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Catchments discharging directly to the Grand River:

Catchment 200 (7.53 hectares, 10% impervious) represents the proposed open space block and rear yard areas of the lots fronting onto Bielby Street. Runoff generated from Catchment 200 will be sheetflow to the Grand River. Runoff generated from this catchment is considered clean water as the catchment area consists of building rooftops and vegetated areas.

Catchment 206 (3.29 hectares, 80% impervious) represents the proposed stormwater management facility and rear yard areas from Streets B and C. The proposed stormwater management facility is designed to provide the required Enhanced level of protection prior to discharging to the Grand River. The stormwater management facility will outlet to a linear dispersion trench which will spread the flows over a wide area, preventing a point source discharge, and will direct a portion of the flow to the existing wetland while directing the remaining flows to the Grand River.

Table 4 lists the post-development uncontrolled flow rates for each catchment under the 2, 5 and 100-year design storms.

Table 4: Post-Development Uncontrolled Flow Rates

	2-Year	5-Year	100-Year	Regional Storm
Catchment 200	0.162 m ³ /s	0.437 m ³ /s	1.746 m ³ /s	0.809 m ³ /s
Catchment 201	0.783 m ³ /s	1.019 m ³ /s	1.688 m ³ /s	0.512 m ³ /s
Catchment 202	0.511 m ³ /s	0.695 m ³ /s	1.195 m ³ /s	0.447 m ³ /s
Catchment 203	0.323 m ³ /s	0.462 m ³ /s	0.979 m ³ /s	0.325 m ³ /s
Catchment 204	0.043 m ³ /s	0.103 m ³ /s	0.433 m ³ /s	0.225 m ³ /s
Catchment 205	0.587 m ³ /s	0.788 m ³ /s	1.353 m ³ /s	0.485 m ³ /s
Catchment 206	0.550 m ³ /s	0.732 m ³ /s	1.198 m ³ /s	0.389 m ³ /s
Catchment 301	0.300 m ³ /s	0.405 m ³ /s	0.730 m ³ /s	0.282 m ³ /s
Catchment 302	0.072 m ³ /s	0.104 m ³ /s	0.233 m ³ /s	0.078 m ³ /s
Catchment 303	0.397 m ³ /s	0.544 m ³ /s	0.948 m ³ /s	0.371 m ³ /s
Catchment 304	0.125 m ³ /s	0.169 m ³ /s	0.301 m ³ /s	0.117 m ³ /s
Total Flow Rate	3.852 m³/s	5.256 m³/s	9.788 m³/s	3.853 m³/s

5.3 Stormwater Management Overview

In line with current practices and guidelines, the stormwater management approach for the River's Edge Subdivision is designed as a "treatment train" to remove sediments and any absorbed contaminants prior to the discharge of runoff from the development to the receiving outlets. The "treatment train" approach will include a combination of lot level, conveyance and end-of-pipe best management practices and is proposed to filter and remove sediments from stormwater runoff prior to discharging to the Grand River and existing wetland.

Due to the proximity of the Grand River, the stormwater management facility is designed to route runoff volumes that exceed the water quality volumes through the pond as quickly as possible. This will ensure the peak flows from the development are discharged and routed along the Grand River prior to peak flows from upstream lands.

Lot Level Controls

Stormwater management practices recommended for providing lot level controls on this site are as follows:

a) Roof Drainage to Ground Surface

The lots will drain to the street. The roof runoff will be filtered across the grassed surface. The runoff for any event large enough to generate flow to the swale system will be adequately filtered by the grass.

b) Rear Yard Swales

The grading of the lots will be to current Town of Grand Valley standards. Where practical, the length of the rear lot swales between catch basins will be increased to extend the contact time with the grassed surfaces.

To promote more infiltration on the lots and in the swales, it is recommended that the average depth of graded topsoil be 300 mm.

c) Foundation Drainage

Gravity connections from the foundation drainage to the storm sewer system are recommended wherever possible per Town of Grand Valley standards. Inlet restrictions will ensure that during the full range of design events, flows in the storm sewer system will not surcharge to the elevation of the foundation drains. Sump pumps are proposed for lots where a gravity foundation connection is not possible – the sump pump will pump the foundation drainage to the storm sewer lateral. Lots requiring sump pumps will be identified on the preliminary grading and servicing drawings.

Conveyance Controls

Conveyance controls will be achieved through municipal maintenance of the storm sewer system. The regular cleanout of the manholes, catch basins and oil/grit structures will remove the heavier sediments deposited from the runoff during storm events.

Oil/grit separators have been designed and sized to pre-treat the runoff from the proposed development prior to discharging to the stormwater management facility. Oil/grit separators will be installed within the proposed development at each of the three pond inlets (Inlet 1 – Bielby Street, Inlet 2 – Street B and Inlet 3 – Street C). The pre-treatment of stormwater runoff will remove larger particles and reduce the frequency with which the facility requires remediation due to sediment build-up.

End of Pipe Facilities

The end-of-pipe component consists of the proposed stormwater management facility located near the south-easterly edge of the property adjacent to the Grand River. The stormwater management facility will outlet to a linear dispersion trench which will spread the flows over a wide area preventing a point source discharge and will direct a portion of the flow to the existing wetland while directing the remaining flows to the Grand River. The proposed stormwater management facility has been designed as wetland type facility complete with three forebays and a 0.3m deep permanent pool to provide the required water quality controls. The proposed outlet structure is designed to provide extended detention of the required water quality storage volumes as per the Ministry of Environment 2003 Stormwater Management Planning and Design Manual.

5.4 Proposed Stormwater Management Facility

Water Quality

The proposed stormwater management facility has been designed to function as a wetland. From Table 3.2, Stormwater Management Planning and Design Manual, 2003, in order to provide Enhanced water quality treatment, a wetland facility requires 115 m³/ha of storage volume for a contributing drainage area that is 65% impervious. 40 m³/ha of the required storage volume is extended detention volume, while the remaining 75 m³/ha is permanent pool.

The quality control drainage area was sized based on the development areas provided in the Draft Plan of Subdivision. The table below summarizes the development areas which contribute flows to the proposed stormwater management facility.

Table 5: Development Areas Contributing Flow to the Stormwater Management Facility

Land Use	Area (ha)	% Impervious
Single Detached Lots	9.271	60.0%
Semi Detached	0.373	70.0%
Townhomes	2.125	90.0%
Apartment	1.268	90.0%
Park	2.532	10.0%
Stormwater Management	2.321	50.0%
To be added to Water Tower	0.047	50.0%
Roads	4.425	80.0%
Total	22.362	62.0%

Please note that Open Space Block 187 and Future Development Block 189 are not included in the above calculation as the open space area drains directly to the Grand River and the future development block is located downstream of the stormwater management facility and is assumed to have it's own stormwater management system at the time of development.

Based on a contributing site area of 22.362 hectares and a requirement for 75m³/ha of permanent pool storage volume to achieve Enhanced treatment, the required permanent pool storage is 1,677m³.

The external drainage areas that discharge to the site such as existing developments on Luther Road, Mary Court, Baker Court, Crozier Street and Scott Street are not included in the water quality calculations for the proposed stormwater management facility and will be conveyed through the facility to the Grand River.

The proposed stormwater management facility has been designed with a 0.3 metre deep permanent pool, which provides 1,788m³ of permanent pool volume. Additional permanent pool volume of 962m³ will be provided in the three (3) sediment forebays created at each pond inlet for a total permanent pool volume of 2,750m³.

Oil/grit separators located upstream of the pond inlets have been sized to pre-treat runoff prior to entering the proposed stormwater management facility. A Stormceptor EF08 will pre-treat runoff prior to reaching Inlet 1, providing 64% Total Suspended Solid removal. For Inlet 2, a Stormceptor EF04 will pre-treat runoff providing 61% Total Suspended Solid reduction. AT Inlet 3, a Stormceptor EF08 is proposed to pre-treat runoff providing 60% Total Suspended Solid reduction. The pre-treatment of the stormwater runoff from the development will reduce the frequency in which the stormwater management facility needs to be remediated due to sediment buildup. Pre-treatment will remove larger sediments before they are deposited within the stormwater management facility. Oil/grit separator sizing is included in **Appendix E**.

Extended Detention

Extended detention volume is calculated based on the runoff volume generated by the 4-hour 25mm design storm event. The outlet structure has been designed to provide a 24.7-hour detention of the required extended detention volume, which corresponds to a release rate of 0.060m³/s.

From the design of the stormwater management pond, a storage volume of 3,749m³ corresponds to a ponding depth of approximately 0.50 metres. The outlet structure has been designed with a 215mm diameter orifice, which will control the extended detention volume for the 24.7 hour detention time.

Pond Routing

Table 6 compares the routing results through the stormwater management facility with the available stage/storage/ discharge capacities.

Table 6: Stormwater Management Facility – Stage/Storage/Discharge Comparison

Control	Available Capacity			Actual Capacity Used			Drawdown Time hr.
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	
Bottom of Pond	0	0	456.00	--	--	--	--
25 mm Storm	--	--	--	0.060	3,749	456.50	24.7
DICB Lip	0.060	3,760	456.50	--	--	--	--
2 Year Storm	--	--	--	0.264	4,552	456.59	25.1
5 Year Storm	--	--	--	1.235	6,160	456.77	25.5
Weir (3 weirs)	2.490	8,552	457.00	--	--	--	--
Regional Storm	--	--	--	2.808	8,684	457.01	26.0
100 Year Storm	--	--	--	5.771	9,570	457.09	25.8
Top of Pond	31.960	13,276	457.40	--	--	--	--
Overflow	44.189	14,711	457.50	--	--	--	--

The pond outlet will comprise of three (3) ditch inlet structures per OPSD 702.050, each connected to a 600mm diameter outlet pipe at a slope of 0.50%, outleting to a linear dispersion trench located at the east side of the pond block at an elevation of 455.36m, above the river's 100-year storm flooding elevation of 455.13m. The linear dispersion trench will spread the flows over a wide area, preventing a point source discharge. Additional details for the linear dispersion trench are provided in **Section 5.6**.

Three (3) major overland flow weirs are proposed at an elevation of 457.00m, above the Grand River Regional flooding elevation of 456.84m. The intent of the three weirs is to provide an unrestricted outlet for the major flow events so that the peak flows from the development are discharged and routed along the Grand River prior to peak flows from upstream lands.

The flooding elevations for the Grand River have been provided by the Grand River Conservation Authority (GRCA) in Canadian Geodetic Vertical Datum 1928 (CGVD28) format.

Sediment Forebay Design

The proposed stormwater management facility has been designed with three (3) sediment forebays at the main pipe inlet locations. The sediment forebays are 1.0 m deep and have been designed per the Ministry of Environment Stormwater Management Planning and Design Manual guidelines. The full sediment forebay sizing information has been included in **Appendix D**.

Table No. 7 summarizes the required and provided parameters within the sediment forebay design.

Table No. 7: Sediment Forebay Design Details

		Forebay at Inlet 1	Forebay at Inlet 2	Forebay at Inlet 3
Required	Dispersion Length (m)	28.9	7.4	31.8
	Settling Length (m)	24.9	25.1	23
	Flow Velocity (m/s)	0.15	0.15	0.15
	Length to Width Ratio	2:1	2:1	2:1
	Settling Velocity (m/s)	0.0003	0.0003	0.0003
Provided	Forebay Length (m)	34	33	38.5
	Flow Velocity (m/s)	0.15	0.04	0.12
	Length to Width Ratio	3.1:1	3.1:1	2.7:1

Therefore, the sediment forebays have been designed to provide the required dispersion and flow lengths.

Sediment Loading and Cleanout Frequency

Table No. 8 illustrates sediment loading to the oil/grit separator systems and sediment forebays as well as the subsequent cleanout frequency required to maintain these systems. The oil/grit separator structures are designed to filter out large sediments before reaching the stormwater management facility. The oil/grit separators are proposed to be flushed out and cleaned on a yearly basis.

Table No. 8: Sediment Loading and Cleanout Frequency – Sediment Forebays

System Component	Treatment Train Component	Catchment Area	Imp. (%)	Annual Sediment Loading	TSS Removal	Annual TSS Reduction	Storage Volume	Cleanout Frequency
Forebay at Pond Inlet 1	OGS (Stormceptor EFO8)	9.96 ha	50%	15.9m ³ (based on 1.6m ³ /ha)	64%	10.1 m ³	8.8 m ³	~ 10 months
	Forebay				80%	4.6 m ³		
	Forebay (Scott St Pipe Outlet)	2.60 ha	55%	4.9m ³ (based on 1.9m ³ /ha)	80%	4.0 m ³	91 m ³ (1/3 of forebay)	~ 10 years
				Total Inlet 1 = 8.6m³				
Forebay at Pond Inlet 2	OGS (Stormceptor EFO4)	2.76 ha	60%	6.1m ³ (based on 2.2m ³ /ha)	61%	3.7 m ³	1.2 m ³	~4 months
	Forebay				80%	1.9 m ³	82 m ³ (1/3 of forebay)	~ 43 years
Forebay at Pond Inlet 3	OGS (Stormceptor EFO8)	10.19 ha	72%	31.6m ³ (based on 3.1m ³ /ha)	60%	19.0 m ³	8.8 m ³	~6 months
	Forebay				80%	10.1 m ³	148 m ³ (1/3 of forebay)	~ 14 years

Sediment loadings obtained from Table 6.3 of the MOE Stormwater Management Planning and Design Manual, 2003 and prorated based on catchment percent impervious.

The above noted Stormceptor storage volumes are manufacturer specified minimums. As part of the detail design stage, the Stormceptor sump can be increased to provide additional volume to accommodate the annual TSS reduction volume and require a cleanout frequency of one (1) year.

5.6 Stormwater Management Facility Outlet / Linear Dispersion Trench

The stormwater management facility outlet pipes will discharge to the proposed linear dispersion trench located at the east side of the pond between the existing woodlot and wetland.

The 15m long, 8m wide, 0.5m deep linear dispersion trench will spread the flows over a wide area, preventing a point source discharge and will be constructed flat to allow the trench to fill-up gradually then spill evenly over the sides. The flows spilling from the south side of the linear dispersion trench will be directed to the existing wetland by the natural topography while the remaining flows will follow the existing topography in the easterly direction to the Grand River.

Table 9 compares the routing results through the linear dispersion trench with the available stage/storage/discharge capacities.

Table 9: Linear Dispersion Trench – Stage/Storage/Discharge Comparison

Control	Available Capacity			Actual Capacity Used		
	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m	Peak Flow m ³ /s	Storage Volume m ³	Storage Elevation m
Bottom of Trench	0	0	454.86	--	--	--
Top of Trench / Weir	0	20	455.36	--	--	--
25 mm Storm	--	--	--	0.060	20	455.36
2 Year Storm	--	--	--	0.264	20	455.38
5 Year Storm	--	--	--	1.235	20	455.43
Regional Storm	--	--	--	2.458	21	455.48
100 Year Storm	--	--	--	3.056	21	455.50

The linear dispersion trench will act in a similar fashion to a weir. The north, south and east portions of the trench will all spill at the same elevation causing the flows to follow the natural topography to the existing wetland and Grand River.

Table 10 interpolates the flow distribution from the linear dispersion trench to the existing wetland and Grand River which is used in the MIDUSS Model. It is assumed that the linear dispersion trench is acting as a weir with three (3) sides, north, east and south. The north and east sides of the linear dispersion trench will outlet to the Grand River via the existing topography along the bank of the river while flows exiting the south side of the trench will follow the natural topography and will outlet to the existing wetland.

The total flow length along the three sides of the linear dispersion trench is 38m (15m north portion + 8m east portion + 15m south portion). Flows directed to the wetland are split on a 39.5% split from the total flow exiting the linear dispersion trench (15m / 38m).

Table 10: Flow Diversion to the Grand River and Existing Wetland

Design Storm	Level (m)	Flow (m ³ /s)	Existing Wetland (m ³ /s)	Grand River (m ³ /s)
2-Year	455.38	0.264	0.104	0.160
5-Year	455.43	1.235	0.488	0.747
100-Year	455.50	3.056	1.207	1.849
Regional	455.48	2.458	0.971	1.487

The MIDUSS model was used to divert the flows to the Grand River and existing wetland based on the flow rates in Table 10 above. Table 11 below compares the pre and post development flow rates and volumes to the existing wetland.

Table 11: Comparison of Pre and Post-Development Flows and Volumes to the Existing Wetland

	2 Year		5 Year		100 Year		Regional Storm	
	Flow Rate (m ³ /s)	Volume (m ³)	Flow Rate (m ³ /s)	Volume (m ³)	Flow Rate (m ³ /s)	Volume (m ³)	Flow Rate (m ³ /s)	Volume (m ³)
Pre-Development	0.147	428	0.497	1,869	2.116	6,413	1.056	14,804
Post-Development	0.104	2,289	0.488	4,097	1.207	8,163	0.971	24,098

Therefore, during post-development conditions, the flow rates to the existing wetland are generally in close proximity to existing flows with the exception of the 100-year design storm event where the flow rate is almost reduced in half. The runoff volumes to the existing wetland are generally higher in post-development conditions.

5.7 Comparison to Pre-Development Flows

Table 12 summarizes the comparison of pre and post-development flows from the entire development.

Table 12: Comparison of Pre and Post-Development Flows

	2 Year	5 Year	100 Year	Regional Storm
	Flow Rate (m ³ /s)	Flow Rate (m ³ /s)	Flow Rate (m ³ /s)	Flow Rate (m ³ /s)
Pre-Development	0.935	2.172	8.805	3.839
Post-Development	0.305	1.616	10.573	6.075

The post-development peak flow rates are generally lower than the pre-development peak flow rates from the site under the minor storm events. For the major storm events, the post development peak flow rates exceed the pre-development peak flow rates as both the 100-year design storm event and Regional storm event will flow over the weir structures in the stormwater management facility.

The intent is to route flows to the Grand River as quickly as possible as discussed in the sections above in order to ensure that the peak flows from the development are discharged and routed along the Grand River prior to peak flows from upstream lands. However, the stormwater management facility does provide some natural attenuation under the minor storm events as the quality storm extended detention drawdown time requirement of 24 hours limits the ability to quickly discharge minor storm events.

6.0 SEDIMENT AND EROSION CONTROL PLAN

Primary sediment control will be achieved with the installation of Type 2 sediment fence around the property boundary. The silt fence will eliminate the opportunity for water borne sediments to be transported from the site.

Temporary rock check dams will be installed in rear and side yard swales after the initial grading has been completed to slow the flow rates and promote the settlement of water borne sediments before they reach the silt fences and ponds.

Upon completion of the grading, any area not subject to active construction within 30 days will be topsoiled and seeded as per OPSS 572.

The stormwater management facility will be graded and shaped at the start of any construction or pre-grading activity. A silt fence will be placed around the outlet structures to restrict the movement of sediment.

Once catch basins have been installed, the grates will be wrapped in filter cloth. This will be maintained until all building and landscaping has been completed.

Inspection and maintenance of all silt fencing and the sediment pond will start after installation is complete. These features will be inspected on a weekly basis or after a rainfall event of 13 mm or greater. Maintenance will be carried out, within 48 hours, on any part of the facility found to need repair.

Monthly reports on the condition of the sediment and erosion control measures will be submitted to the Town of Grand Valley and the Grand River Conservation Authority.

Once construction has been substantially completed, the silt fence will be removed from within the pond, any accumulated sediment will be removed and the landscaping and planting of the ponds will be completed.

After construction of the complete development, erosion will not occur and sediment transport will be minimal. The stormwater management facility will provide all sediment removal.

7.0 CONCLUSIONS

In summary, the features of the design for the proposed development are as follows:

1. Gravity sanitary sewer for the River's Edge Subdivision will be provided by connecting to the existing municipal sanitary sewer on Bielby Street. It is noted that some improvements could be required downstream of the subject lands to support the ultimate Town buildout as discussed in Section 4.4 of this report.
2. Water servicing will be provided by connection to the existing municipal watermains on Bielby Street, Luther Road and proposed watermain extension on Highway 25. It is noted that some improvements could be required to the Town's water storage and supply system as discussed in Section 4.3 of this report.
3. Storm sewers will be sized at the detail design stage to convey the 5-year design storm event to the proposed stormwater management facility. It is noted that storm sewers on collector roads, i.e. Bielby Street will be required to convey the 10-year design storm event as per Town of Grand Valley Engineering Standards. Major overland flows will be directed towards the municipal right of ways to the proposed stormwater management facility.
4. The proposed stormwater management facility is designed to function as a wetland with 0.3m deep permanent pool and forebays to provide an Enhanced level of quality control (80% Total Suspended Solid removal).
5. Stormwater quality pre-treatment will be provided by oil/grit separators located upstream of the stormwater management facility.
6. Due to proximity to the Grand River, quantity control targets are not required for the proposed stormwater management facility. However, the facility does provide natural attenuation of runoff prior to outletting to the Grand River.
7. The proposed development will continue to discharge flows to the existing wetland located at the south easterly portion of the site. Under post-development conditions, the flow rates to the existing wetland are generally reduced while the runoff volumes are generally increased from pre-development conditions.
8. Flows discharged from the proposed stormwater management facility will be routed through a linear dispersion trench to spread the flows over a wide area, avoiding a point source discharge.
9. The stormwater management systems meet the current Provincial and Municipal guidelines.
10. The principles of "Stormwater Management Practices", the Ministry of Environment Stormwater Management Planning and Design Manual 2003 have been used in the design of the stormwater management system.

All of which is respectfully submitted.

GM Blue Plan Engineering LIMITED
Per:



Angela Kroetsch, P.Eng.

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APPENDIX A

Geotechnical Investigation, Peto MacCallum Ltd., 2009
Geotechnical Slope Assessment, Peto MacCallum Ltd., 2012
Geotechnical Investigation, JLP Services Inc., 2022





**GEOTECHNICAL INVESTIGATION
FIFE ROAD SUBDIVISION
FIFE ROAD
GRAND VALLEY, ONTARIO**

**for
GAMSBY AND MANNEROW LIMITED**

PETO MacCALLUM LTD.
16 FRANKLIN STREET SOUTH
KITCHENER, ONTARIO
N2C 1R4
PHONE: (519) 893-7500
FAX: (519) 893-0654
EMAIL: kitchener@petomacallum.com

Distribution:

4 cc: Gamsby and Mannerow Limited
1 cc: PML Kitchener
1 cc: PML Toronto

PML Ref.: 09KF021
Report: 1
June 3, 2009

June 3, 2009

PML Ref.: 09KF021
Report: 1

Ms. Sarah Austin, P.Eng.
Gamsby and Mannerow Limited
255 Woodlawn Road West
Suite 210
Guelph, Ontario
N1H 8J1

Dear Ms. Austin

Geotechnical Investigation
Fife Road Subdivision
Fife Road
Grand Valley, Ontario

Peto MacCallum Ltd. (PML) is pleased to report the findings of the geotechnical investigation recently completed for the above noted site. Authorization was received from Mr. Paul McLennan, P. Eng of Gamsby and Mannerow Limited, in a signed Engineering Services Agreement (ESA), dated April 1, 2009.

The project involves a planned 115 lot residential subdivision, located near the existing Fife Road Subdivision, in Grand Valley, Ontario. The development will include storm water management facilities at the northwest and southeast corners of the site. Once the design is finalized, PML should be retained to check that the recommendations in this report remain applicable.

The purpose of the geotechnical investigation was to explore the subsurface soil and groundwater conditions at the site. Based on the findings, geotechnical design and construction recommendations were to be provided. Specific considerations to be addressed included:

- Site preparation, including stripping, cut and fill grading, and requirements for placement of engineered fill
- Foundations, including bearing capacity and settlement estimates
- Excavating conditions, including safe side slopes, and groundwater control requirements



- Bedding and backfilling, including reusability of site materials
- Structural pavement designs
- Hydraulic conductivity estimates for storm water management retention ponds

An environmental assessment of the site was not included within the terms of reference for this assignment.

Investigation Procedure

The field work for the investigation was carried out on April 16 and 17, 2009, and consisted of nine conventional sampled boreholes at locations shown on the appended Borehole Location Plan, Drawing 1. The boreholes were advanced to depths of between 2.30 and 14.00 m below grade. Boreholes 3, 4 and 9 included 50 mm observation well installations for follow-up groundwater level monitoring.

The boreholes were advanced using a Dietrich-50, track mounted drillrig equipped with continuous flight solid stem and hollow stem augers, supplied and operated by a specialist drilling contractor.

Representative samples of the overburden were recovered at regular intervals throughout the depths explored. Standard penetration tests were carried out during sampling operations using conventional split spoon equipment. Groundwater observations were made in the boreholes and observation wells during and upon completion drilling.

The field work was supervised throughout by a member of our engineering staff who directed the drilling and sampling operation, prepared stratigraphic logs, monitored groundwater conditions, and processed the recovered samples.



The borehole locations and ground surface elevations were surveyed by Gamsby and Mannerow Limited, and provided on a site grading plan.

All samples secured in the field were returned to our laboratory for detailed visual examination to confirm field soil classifications. The laboratory testing program included moisture content determinations on all recovered samples together with three particle size distribution analysis carried out on soil samples encountered in the area of the storm water management pond.

It should be noted that decommissioning of the monitoring wells as required under O.Reg 903 was not included within PML's terms of reference for this assignment and future decommissioning will be required.

Summarized Site and Subsurface Conditions

The site consisted primarily of vacant land, with agricultural land comprising the northern portion of the site. A former gravel pit was located near the southern portion of the site, but was no longer in operation. The site is hilly with a total relief of approximately 25 m from a plateau near Fife Road to the low point, above a slope to the Grand River near the southern portion of the site. From the grading plan, it appears that many lots will contain units with walkout basements.

Reference is made to the appended Log of Borehole sheets for details of the drilling work including soil descriptions, inferred stratigraphy, standard penetration N values, groundwater observations during and upon completion of drilling, and natural moisture content determination test results.

In general, the subsurface stratigraphy contacted at the site comprised surficial topsoil overlying mixed till, sand and gravelly sand deposits.



The boreholes typically encountered between 600 and 800 mm of surficial topsoil. No topsoil was penetrated in Borehole 8 and in Boreholes 6 and 9, 200 and 100 mm of topsoil were encountered respectively. The surficial topsoil was typically about plastic limit (APL) and was typically described as firm to hard clayey silt with rootlets and occasional cobbles. Surficial boulders were also present around Boreholes 6, 7 and 9.

Mixed deposits of tills, sand and gravelly sand were underlying the surficial topsoil.

Clayey silt till was contacted immediately below the surficial topsoil in Boreholes 1 through 7 and was described as firm to stiff with standard penetration N values between 7 and 77 for 150 mm of penetration. Occasional to numerous cobbles and boulders were contacted in the till layer. Isolated seams of sand and gravelly sand were contained within the till deposit at various depths, which exhibited groundwater seepage in Boreholes 1, 3 and 4. The upper till layer extended to between 3.05 and 6.30 m below grade in Boreholes 1 through 7; extending to the borehole termination depth in Boreholes 2, 3 and 7. In Borehole 5, the clayey silt till was overlying a silt till layer and a second layer of clayey silt to a depth of 9.45 m below grade. Deeper till deposits were also encountered in Boreholes 1, 4 and 9; typically consisting of hard brown or grey clayey silt.

Reference is made to Figure 1, attached, for a particle size distribution chart for the clayey silt till encountered in Borehole 3.

Various sand and gravel, gravelly sand and sand deposits were encountered in Boreholes 1, 4, 5, 6, 8 and 9. These layers were described as saturated in Boreholes 1, 4, 5 and 9. Borehole 8 was terminated in a sand layer at a depth of 2.30 m below grade, due to auger refusal on boulders.

Particle size distribution charts for the medium sand, and the sand and gravel deposits encountered in Borehole 9 are presented in Figures 2 and 3 respectively.

Groundwater observations carried out during and upon completion of drilling and in observation wells installed in Boreholes 3, 4 and 9 are summarized on the appended Log of Borehole Sheets.



Groundwater seepage was encountered in Boreholes 1 and 4 in sand seams confined in the till deposits at 1.5 m below grade in both boreholes. The seepage in these boreholes was likely attributable to perched water conditions. The groundwater table was penetrated in Boreholes 1, 3, 4 and 9. Groundwater was encountered at various depths in the boreholes, between elevation 453.80 and 477.85 (metric, geodetic).

Initial water level readings were taken in the observations wells installed in Boreholes 4 and 9 and were at elevation 472.20 and 454.23 respectively. The observation well installed at Borehole 3 was dry at the time of the initial reading.

It should be noted that groundwater levels, particularly perched groundwater levels, can vary significantly in response to seasonal weather conditions and weather events.

Discussion and Recommendations

The project involves a planned 115 lot residential subdivision located near the existing Fife Road Subdivision. The development will include storm water management ponds at the northwest and southeast corners of the site. The storm water management ponds are expected to be detention type facilities. It is understood that the proposed development will generally involve site grading, servicing, road construction and building construction.

Site Grading

The site is hilly with a total relief of approximately 25 m across the site. The site plateaus north of the existing Fife Road at an elevation of approximately 481.0 (metric, geodetic) and gradually falls off to the north, to an elevation of about 469.0 at the top of a steep embankment slope to the Grand River. From the plateau, the site grade falls steeply to an elevation of about 455.0 toward the east and southeast of the site; the grades through this portion of the site are variable, and are hillier than to the north or northwest. A former gravel pit was located near the southeast portion of the site. The Grand River is situated along the eastern portion of the site, running from northwest to southeast.



The site grading plan shows that significant cuts and fills will be required prior to construction. Grade cuts will be necessary to the east and north of the existing Fife Road (around Boreholes 1, 4 and 5). The grade cuts will typically be between 1.0 and 2.0 m with as much as 4.0 m east of Borehole 4, and east of Borehole 1. The northern portion of the site (Boreholes 2 and 3) will typically require around 0.5 to 1.5 m of fill and as much as 2.0 to 3.0 m near the top of the slope to the Grand River. The south and southeastern portion of the site will also require fill (Boreholes 6 through 9). The area around Borehole 6 and 7 typically require less than 1.0 m of fill, however between 3.0 and 5.0 m of fill is needed around Borehole 8. Approximately 5.0 m of fill is typically expected for the area around Borehole 9, with a range between 2.0 and 7.0 m.

Removal of all surficial topsoil will be required prior to the grading operations. Surficial topsoil thicknesses across the site typically ranged between 100 and 800 mm. No topsoil was present at Borehole 8, and topsoil was sporadic around Borehole 9. In calculating the approximate quantity of topsoil to be stripped, we recommend that the topsoil thickness shown on the individual borehole logs be increased by 50 mm to account for variations and some stripping of the mineral soil below.

Engineered fill material used to raise grades should comprise approved inorganic material placed in maximum 300 mm thick lifts and compacted to at least 98% standard Proctor maximum dry density (SPMDD) below footings, and 95% SPMDD below floors and pavements. Further, generic recommendations for fill subgrade preparation and engineered fill construction are provided in Appendix A.

It is anticipated that excess materials generated from grade cuts will primarily comprise clayey silt, and silt materials which are highly susceptible to moisture content variations and not well suited for engineered fill construction. Compaction difficulties due to elevated moisture levels are expected. It is anticipated that the site material will not be suitable for compaction to 98% SPMDD. Select portions of the material may be used where compaction to 95% SPMDD is specified providing excessively wet soils are discarded (i.e material described as WTPL or wet / saturated on the borehole logs).



Settlement of on site soils due to the surcharge of additional properly compacted site grading fills is not expected to be of concern.

Building Construction

Based on the investigation findings, the native inorganic soils are suitable to support the proposed buildings on conventional footings. For preliminary design purposes, footings founded at least 0.50 m into the native deposits may be designed using the recommended values for factored resistance at ultimate limit state (ULS) and serviceability limit state (SLS) of 225 and 150 kPa, respectively.

Where grades are raised, the footings may be supported on engineered structural fill, placed in accordance with the generic recommendations for engineered fill construction provided in Appendix A. For engineered fill supporting footing loads, compaction to a minimum 98% of the materials SPMD, should be specified, as per recommendations outlined in the preceding 'Site Grading' section of this report and in Appendix A.

Footings supported on the structural fill may then be designed using the recommended values for factored resistance at ULS and SLS of 225 and 150 kPa, respectively.

Full time inspection of the structural fill placement by PML personnel is recommended to approve fill materials and to verify that the specified compaction levels are being achieved.

Total settlements of footings founded on the approved engineered fill or native overburden deposits, designed as outlined above are not expected to exceed 25 mm, with differential settlements between footings being no more than 50% of this value.

All exterior footings should be provided with a minimum 1.2 m of earth cover or the thermal insulation equivalent to provide adequate insulation against potential frost damage. A 25 mm thick layer of polystyrene insulation is thermally equivalent to 600 mm of soil cover.



Prior to concrete placement, all founding surfaces should be examined by PML personnel to verify the competency of the founding surfaces.

It is anticipated that the floor slabs for the proposed structures will be founded on approved structural fill or on the competent overburden deposits and conventional slab-on-grade construction is feasible. We recommend a minimum 150 mm cushion of well compacted Granular A be provided directly beneath the slab to provide uniform support. For conventional slabs, a polyethylene vapour barrier should be installed directly beneath the slab if moisture sensitive floor finishes are planned.

Following removal of any loose organic material and deleterious fill, the exposed floor slab subgrade must be proofrolled with a heavy compactor to achieve 98% SPMDD. Any additional soft areas encountered in the proofrolling process should be subexcavated and backfilled with approved granular material, compacted to 98% SPMDD.

Foundation drainage measures should be taken for units with basements. For discussion purposes, it is assumed that foundations for the proposed buildings will extend to depths of up to 2.0 m below final lot grades. Perforated drainage pipe should be laid around the outside edge of the footings, and connected to a frost free sump system. It is recommended that the drainage pipes be surrounded with a granular filter protected with filter fabric, or alternatively wrapped with filter cloth and surrounded by concrete sand.

A "free draining" granular material or an equivalent, approved drainage board product must be provided for the basement walls, in accordance with the Ontario Building Code. The onsite silty soils are not suitable for use as basement wall backfill unless a drainage board product is provided. The gravelly sand and sand and gravel materials encountered in several locations would be suitable for basement wall backfill. Backfilling should not take place until the ground floor has been constructed, in order to provide lateral support for the wall.



Excavation and Backfilling

Excavation for footings and service trenches will extend through a combination of surficial topsoil, clayey silt till and cohesionless soils. Seepage into the footing excavations should be expected due to perched groundwater within cohesionless soil layers, although this should be manageable by conventional sump pumping.

These soils are classified as Type 3 materials as defined in the Occupational Health and Safety Act (OHSA). Subject to inspection and providing adequate groundwater control is achieved, excavations within Type 3 soils that are to be entered by workers should be inclined from the base of the excavation at one horizontal to one vertical (1H:1V) or flatter.

No bearing problems are anticipated for pipes founded in the native mineral soils encountered at the site. On stable subgrade, a minimum 150 mm thick bedding course of Granular A material compacted to 95% SPMDD is recommended beneath the pipes. The Granular A material should extend around the pipe to at least 300 mm above the pipe obvert or as set out by Ontario Provincial Standards (OPS), or the local authority. Material that is too wet for compaction to a minimum of 95% SPMDD should be allocated for use in landscaped/non settlement sensitive locations, and compacted to at least 90% SPMDD.

Foundation backfill and backfill below pavements, floor slabs (interior backfill) and other settlement sensitive features, should be similarly compacted to 95% SPMDD. It should be noted that the clayey silt till and silt till site materials are frost susceptible and should not be used at locations where frost heave movement could have adverse effects such as at exterior doors and the like. In addition, it should be noted that the clayey silt till material will tend to retain a voided structure when placed as fill. It will be particularly important to ensure that sufficient compaction is applied to breakdown all lumps/clods within the fill matrix to achieve a non-voided condition. Significant post-construction settlement could otherwise result. It is anticipated that suitable site material may not be available in sufficient quantities for use as interior foundation backfill, particularly during wet weather construction periods. Imported sand and gravel will likely be required for this purpose.



For walk-out style houses, exterior grades should be maintained at least 150 mm below the finished lower floor slab level and sloped to promote drainage away from the building.

Pavement Design

Based on the proposed pavement usage, frost susceptibility, and strength of the expected subgrade soils, the following pavement component thicknesses are considered suitable for local residential and minor collector traffic categories.

PAVEMENT COMPONENT	THICKNESS (mm)
HL 3 Surface Course	40
HL 4 Binder Course	40
Granular A Base	150
Granular B Subbase	350

The flexible pavement designs provided above consider that construction will be carried out during the drier time of the year and the subgrade is stable, as determined by proofrolling inspected by PML personnel. If the subgrade is wet and unstable, additional granular subbase will be required.

The pavement materials should conform to current OPS specifications. The Granular A base and Granular B subbase courses should be placed in thin lifts and compacted to a minimum of 100% SPMDD, and asphaltic concrete should be placed and compacted to between 92% and 96.5% of the material's maximum relative density (MRD). Reference is made to OPS Specification 310, revised April 2008.



During construction, testing should be conducted to confirm the gradation and compactibility characteristics of the granular base and subbase materials and the mix design properties of the asphaltic concrete.

Proofrolling procedures and the placement and compaction of all the fill and granular materials and asphaltic concrete for the pavement construction and backfilling at the site should be inspected on a continuous basis by PML technicians.

Since relatively impermeable clayey silt till material will be present at shallow depth beneath the pavement structure in several areas, pavement subdrains should be provided to prevent water accumulation on the pavement subgrade surface. The subgrade should be graded so that water is directed to the catch basin structures or to the pavement edge. Subdrains should be discharged in to the catch basins. The subdrains may consist of 3 m long stubs of filter wrapped, 100 mm diameter perforated plastic pipe, set within the subbase layer at the subgrade surface. At the pavement edge, suitable pavement drainage should be provided by means of subdrains or side ditches.

Storm Water Management

Two storm water management (SWM) ponds are depicted on the Gamsby and Mannerow Limited site grading plan. The ponds are located at Block 165, near the northwest corner (Borehole 3) and Block 166, near the southeast corner (Borehole 9) of the site. It is understood that the SWM ponds will be detention ponds.

Minor grade cuts and fills (typically less than 2.0 m) are planned for the Block 165 SWM pond. The site material in the area of this pond consists primarily of clayey silt till material of very low permeability; typically less than 1×10^{-6} cm/sec. Therefore, a detention pond in this area is considered feasible. Reference is made to Figure 1 attached, for a typical particle size distribution chart of the clayey silt till.



Large cut and fill operations will be required (up to 3.5 m) for the Block 166 SWM pond. Based on the information from Borehole 9 and the grading plan, the subgrade soil for this pond will likely be variable. In the cut areas, the subgrade soil will likely comprise sand and gravel materials of high permeability; typically greater than 5.0×10^{-3} cm/sec. This material would not be considered suitable for a detention pond. It is therefore recommended that a minimum 1.0 m thick clay liner is constructed for the Block 166 SWM pond.

The liner may be constructed from excess clayey silt till material generated from cuts during site grading (near Boreholes 1, 4 and 5, and from the Block 165 SWM pond). The liner should be compacted to minimum 95% SPMDD during construction. In addition, it should be noted that the clayey silt till material will tend to retain a voided structure when placed as fill. It will be particularly important to ensure that sufficient compaction is applied to breakdown all lumps/clods within the fill matrix to achieve a non-voided condition. Significant post-construction settlement and leakage could otherwise result.

Both SWM ponds should be inspected by PML personnel during construction to verify the presence of clayey silt subgrade materials for the Block 165 SWM pond, and to inspect construction of the clay liner for the Block 166 SWM pond.



We trust this report has been completed within our terms of reference, and is sufficient for your immediate requirements. If you have any questions or require further information, please do not hesitate to contact our office.

Sincerely

Peto MacCallum Ltd.

A handwritten signature in black ink, appearing to read "Ken Hanes", is positioned above the typed name.

Ken Hanes, BASc
Project Supervisor



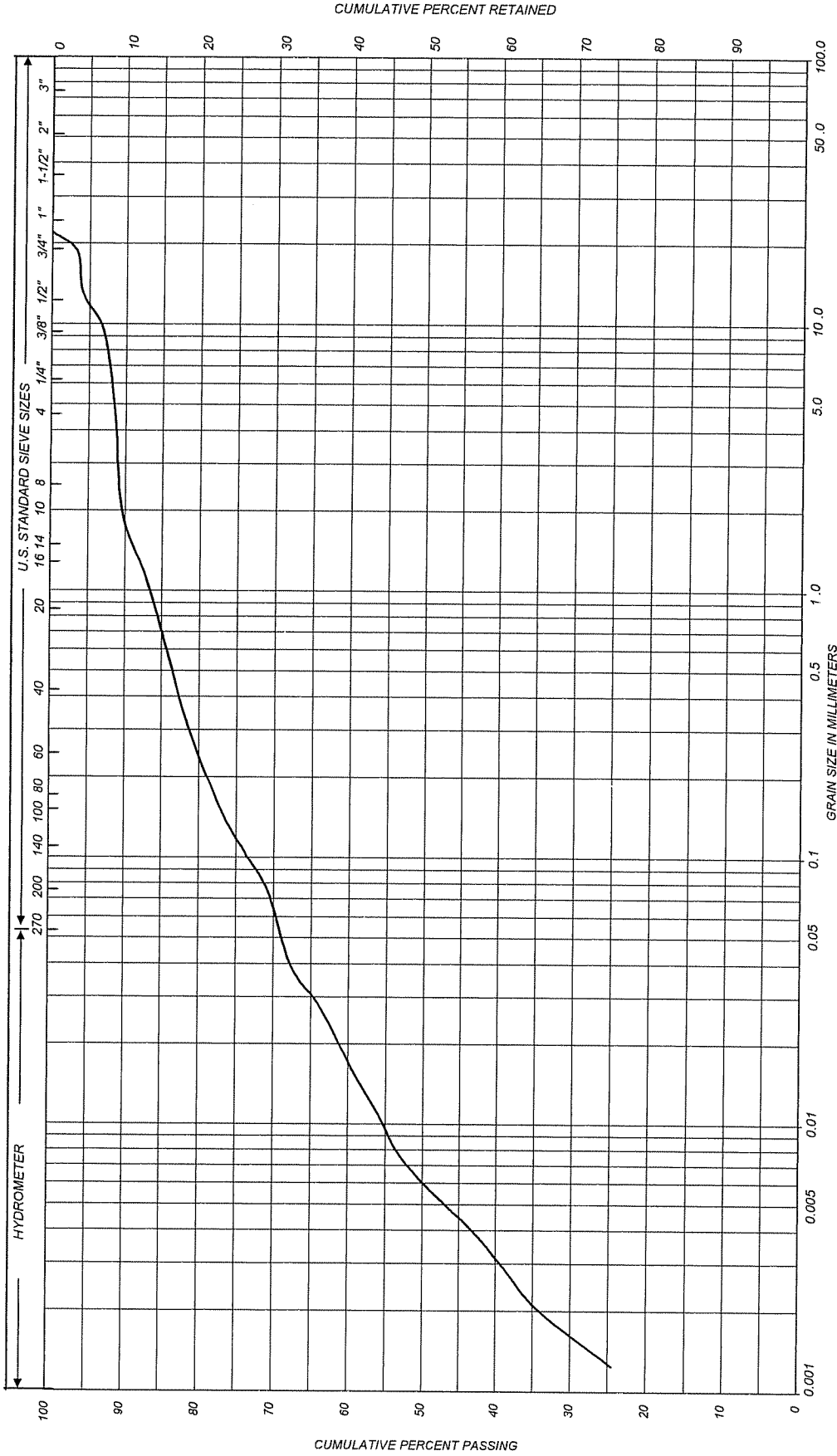
Marian S. Molodecki, P.Eng.
Senior Consultant
Geotechnical and Geoenvironmental Services

KH:sh

Enclosures:

Figures 1-3 – Particle Size Distribution Analysis
List of Abbreviations
Log of Borehole 1 to 9
Drawing 1 - Borehole Location Plan
Appendix A - Engineered Fill

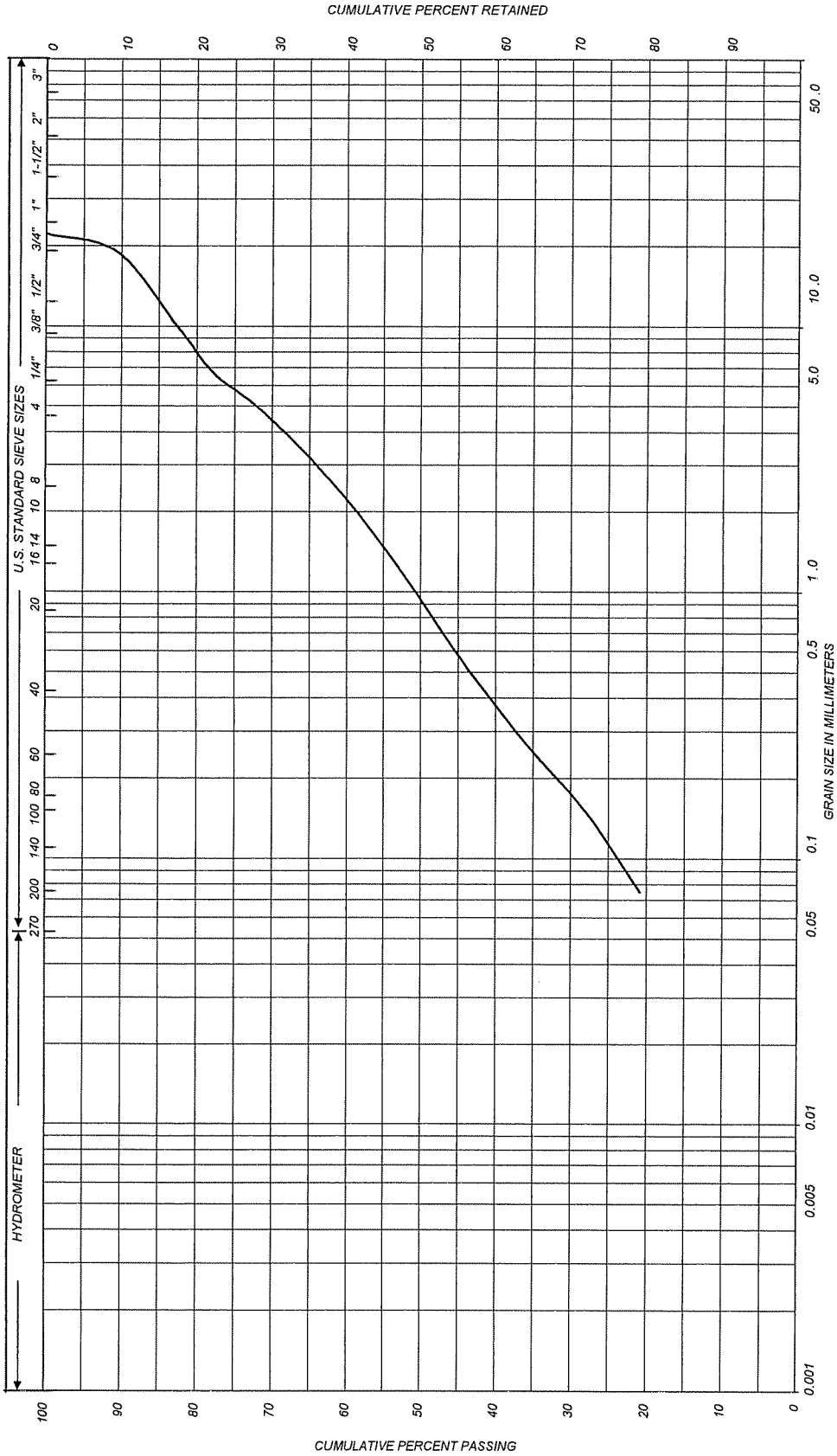
PARTICLE SIZE DISTRIBUTION CHART



CLAY	SILT & CLAY		FINE SAND		MEDIUM SAND		COARSE SAND		GRAVEL		UNIFIED	
	FINE	MEDIUM	FINE	MEDIUM	FINE	MEDIUM	COARSE	COARSE	GRAVEL	GRAVEL	COBBLES	M.I.T.
CLAY	SILT		VERY FINE	FINE	MEDIUM	COARSE	COARSE	COARSE	GRAVEL	GRAVEL	GRAVEL	U.S. BUREAU

REMARKS Borehole 3, Sample SS2 and SS3, Depth 0.60 to 2.00 m
CLAYEY SILT TILL, TRACE GRAVEL, TRACE SAND

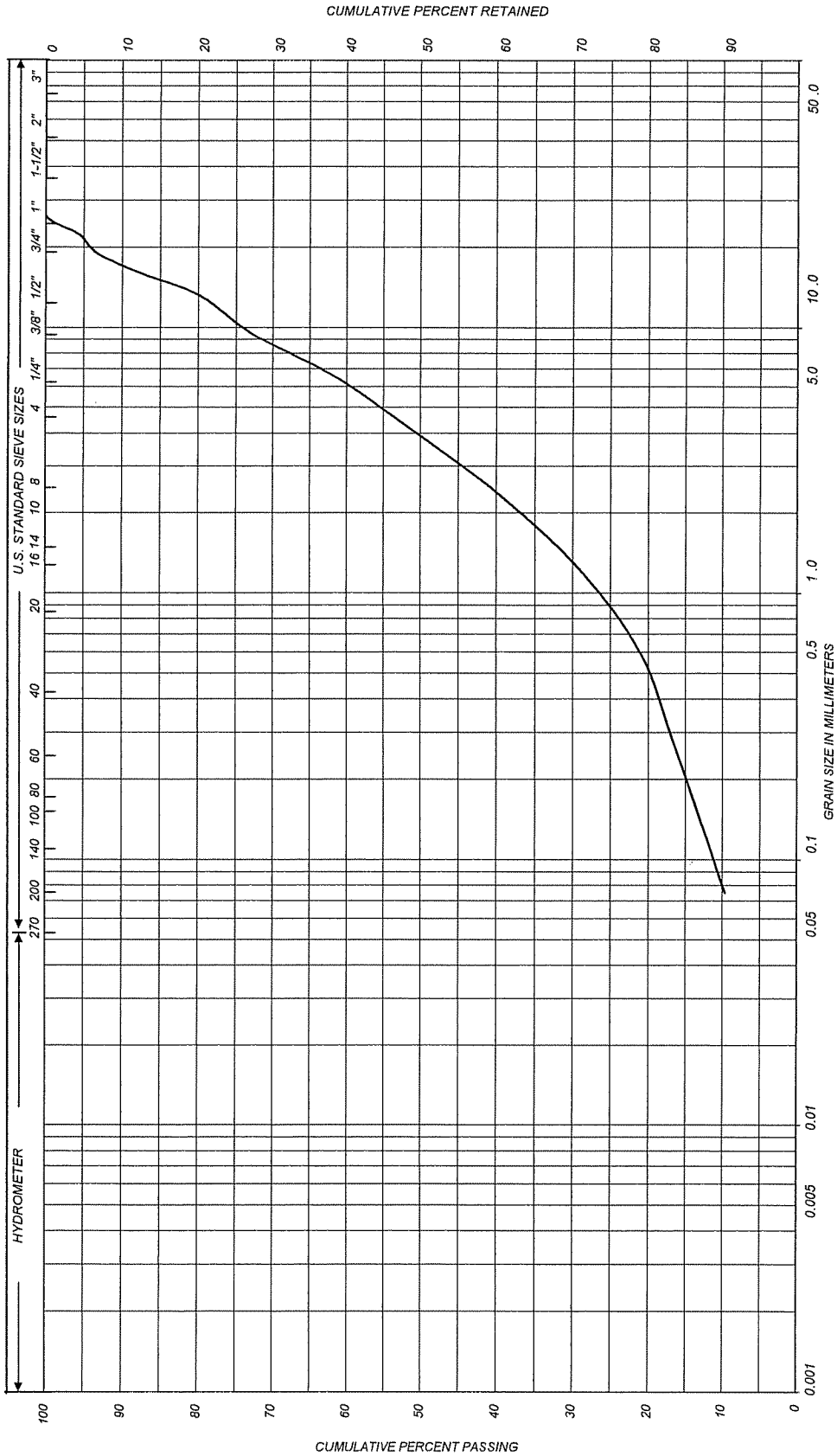
PARTICLE SIZE DISTRIBUTION CHART



CLAY	SILT & CLAY		SAND		GRAVEL		UNIFIED
	FINE	COARSE	FINE	COARSE	FINE	COARSE	CBU
							UBS
							COBBLES
							M.I.T.
							U.S. BUREAU

REMARKS Borehole 9, Sample SS2, Depth 0.10 to 1.50 m
MEDIUM SAND, SOME GRAVEL, SOME SILT

PARTICLE SIZE DISTRIBUTION CHART



HYDROMETER		U.S. STANDARD SIEVE SIZES										UNIFIED												
		270	200	140	100	80	60	40	20	16	14	10	8	4	1/4"	3/8"	1/2"	3/4"	1"	1-1/2"	2"	3"	COB	BLEES
		SILT & CLAY		FINE SAND		MEDIUM SAND		COARSE SAND		GRAVEL		GRAVEL		GRAVEL		GRAVEL		GRAVEL		GRAVEL		M.I.T.		
		CLAY		FINE		MEDIUM		COARSE		FINE		MEDIUM		COARSE		FINE		MEDIUM		COARSE		U.S. BUREAU		
		CLAY		VERY FINE		FINE		MEDIUM		COARSE		FINE		MEDIUM		COARSE		FINE		MEDIUM		U.S. BUREAU		

REMARKS Borehole 9, Sample SS3, Depth 1.50 to 2.75 m
SAND AND GRAVEL, TRACE SILT

LIST OF ABBREVIATIONS



PENETRATION RESISTANCE

Standard Penetration Resistance N: - The number of blows required to advance a standard split spoon sampler 0.3 m into the subsoil. Driven by means of a 63.5 kg hammer falling freely a distance of 0.76 m.

Dynamic Penetration Resistance: - The number of blows required to advance a 51 mm, 60 degree cone, fitted to the end of drill rods, 0.3 m into the subsoil. The driving energy being 475 J per blow.

DESCRIPTION OF SOIL

The consistency of cohesive soils and the relative density or denseness of cohesionless soils are described in the following terms:

<u>CONSISTENCY</u>	<u>N (blows/0.3 m)</u>	<u>c (kPa)</u>	<u>DENSENESS</u>	<u>N (blows/0.3 m)</u>
Very Soft	0 - 2	0 - 12	Very Loose	0 - 4
Soft	2 - 4	12 - 25	Loose	4 - 10
Firm	4 - 8	25 - 50	Compact	10 - 30
Stiff	8 - 15	50 - 100	Dense	30 - 50
Very Stiff	15 - 30	100 - 200	Very Dense	> 50
Hard	> 30	> 200		
WTPL	Wetter Than Plastic Limit			
APL	About Plastic Limit			
DTPL	Drier Than Plastic Limit			

TYPE OF SAMPLE

SS	Split Spoon	TW	Thinwall Open
WS	Washed Sample	TP	Thinwall Piston
SB	Scraper Bucket Sample	OS	Oesterberg Sample
AS	Auger Sample	FS	Foil Sample
CS	Chunk Sample	RC	Rock Core
ST	Slotted Tube Sample		
	PH	Sample Advanced Hydraulically	
	PM	Sample Advanced Manually	

SOIL TESTS

Qu	Unconfined Compression	LV	Laboratory Vane
Q	Undrained Triaxial	FV	Field Vane
Qcu	Consolidated Undrained Triaxial	C	Consolidation
Qd	Drained Triaxial		

LOG OF BOREHOLE NO. 1

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

OUR PROJECT NO. 09KF021
 BORING DATE 2009 04 16
 ENGINEER M. Molodecki
 TECHNICIAN K. Hanes

SOIL PROFILE				SAMPLES			SHEAR STRENGTH C_u ▲				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS
DEPTH In METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3m N - VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION ×				WATER CONTENT W			
							STANDARD PENETRATION TEST ●				WATER CONTENT %			
							BLOWS/0.3M				10 20 30			
	GROUND ELEVATION 479.35													
0.60	TOPSOIL: Dark brown clayey silt, trace gravel, occasional cobbles, rootlets, APL/WTPL		479	1	AS									
1.50	CLAYEY SILT TILL: Firm to stiff brown clayey silt, trace to some gravel, occasional cobbles, WTPL		478	2	SS	7								Sampler wet from 1.5 m.
2.30	numerous saturated gravelly sand seams becoming hard, DTPL		477	3	SS	16								
3.05			477	4	SS	66								
4.55	GRAVELLY SAND: Very dense brown gravelly sand, saturated		476	5	SS	50	100	mm						
5.05	CLAYEY SILT TILL: Hard grey clayey silt, trace gravel, DTPL		475	6	SS	50	50	mm						Upon completion of drilling, borehole wet caved at 1.05 m.
	BOREHOLE TERMINATED AT 5.05 m													

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LOG OF BOREHOLE NO. 2

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

OUR PROJECT NO. 09KF021
 ENGINEER M Molodecki
 TECHNICIAN K Hanes

SOIL PROFILE			SAMPLES			SHEAR STRENGTH C_u ▲				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS	
DEPTH in METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3m N - VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION × STANDARD PENETRATION TEST ●				WATER CONTENT W			
							BLOWS/0.3M				WATER CONTENT %			
							20	40	60	80	10	20	30	
	GROUND ELEVATION 476.11													
0.85	TOPSOIL: Firm dark brown clayey silt, trace gravel, occasional cobbles, rootlets, APL			1	SS	7	●					○		
1.50	CLAYEY SILT TILL: Stiff clayey silt, trace gravel, occasional cobbles and boulders, APL		475	2	SS	9	●	▲				○		
2.30	becoming very stiff		474	3	SS	22	●		▲			○		
3.0	becoming hard, DTPL		473	4	SS	50 fo	75 mm			▲		○		
4.5			472	5	SS	50 fo	100 mm			▲		○		
5.05	BOREHOLE TERMINATED AT 5.05 m			6	SS	50 fo	75 mm			▲		○	Upon completion of drilling, borehole open with no free water.	

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LOG OF BOREHOLE NO. 3

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

OUR PROJECT NO. 09KF021
 ENGINEER M. Molodecki
 TECHNICIAN K. Hanes

SOIL PROFILE				SAMPLES			SHEAR STRENGTH C_u				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS
DEPTH In METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3m N-VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION \times STANDARD PENETRATION TEST \bullet				WATER CONTENT W			
							BLOWS/0.3M				WATER CONTENT %			
							20	40	60	80	10	20	30	
	GROUND ELEVATION 471.02													
0.60	TOPSOIL: Firm to stiff dark brown clayey silt, trace gravel, occasional cobbles, rootlets, APL		470	1	SS	8								
1.50	CLAYEY SILT TILL: Very stiff to hard brown clayey silt, trace gravel, occasional cobbles and boulders, APL/DTPL		469	2	SS	22								
2.30	occasional wet medium sand seams no sand seams		468	3	SS	30								
3.0			468	4	SS	28								
4.05			467	5	SS	32								
4.5	becoming stiff, grey, APL		466	6	SS	13								
5.05	BOREHOLE TERMINATED AT 5.05 m													

Water Level Reading:
04/17/2009: No water

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LOG OF BOREHOLE NO. 4

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

BORING DATE 2009 04 17

OUR PROJECT NO. 09KF021
 ENGINEER M. Molodecki
 TECHNICIAN K. Hanes

SOIL PROFILE			SAMPLES			SHEAR STRENGTH C_v				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS	
DEPTH in METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3M N-VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION \times STANDARD PENETRATION TEST \bullet				WATER CONTENT W			
							BLOWS/0.3M				WATER CONTENT %			
							20	40	60	80	10	20	30	
	GROUND ELEVATION 477.00												1.0 m Stick up with J Plug and Steel Casing Concrete	
0.60	TOPSOIL: Stiff dark brown clayey silt, trace sand, trace gravel, occasional cobbles, rootlets, APL		476	1	SS	10								
1.50	CLAYEY SILT TILL: Very stiff brown clayey silt, trace gravel, numerous cobbles, occasional boulders, APL becoming hard with occasional saturated medium sand seams		475	2	SS	16							Groundwater seepage at 1.5 m.	
3.05	becoming DTPL, no sand seams		474	3	SS	40								
			473										50 mm PVC standpipe	
4.5			472	4	SS	44							Bentonite Seal	
6.0			471											
6.30	GRAVELLY SAND: Brown gravelly sand, numerous cobbles, occasional boulders, saturated		470	5	SS	50 for 100 mm							Groundwater seepage at 6.3 m.	
7.00	CLAYEY SILT TILL: Brownish grey clayey silt, trace gravel, occasional cobbles and boulders, APL		469	6	SS	50 for 100 mm								
7.5			468	7	AS									
9.0			467	8	AS								Groundwater below 9.5 m.	
9.50	SAND: Grey sand, some gravel, saturated		466										Filter Sand	
10.5			465											
12.0			464										Slotted Screen	
13.5			463											
14.00	BOREHOLE TERMINATED AT 14.00 m												Water Level Reading: 04/17/2009: 4.8 m	

NOTES

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LOG OF BOREHOLE NO. 5

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

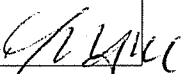
BORING DATE 2009 04 17

OUR PROJECT NO. 09KF021
 ENGINEER M. Molodecki
 TECHNICIAN K. Hanes

SOIL PROFILE			SAMPLES			SHEAR STRENGTH C_u ▲				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS	
DEPTH in METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3M N - VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION ×				WATER CONTENT W			
							STANDARD PENETRATION TEST ●				WATER CONTENT %			
							BLOWS/0.3M				WATER CONTENT %			
	GROUND ELEVATION 471.33													
0.60	TOPSOIL: Firm dark brown clayey silt, occasional cobbles, rootlets, APL		471	1	SS	7								
	CLAYEY SILT TILL: Firm to stiff clayey silt, trace gravel, occasional cobbles and boulders, APL		470	2	SS	10								
1.5			469	3	SS	6								
3.05	becoming hard, DTPL		468	4	SS	77								
4.55	SILT TILL: Very dense brown silt, trace fine sand, trace gravel, occasional cobbles and boulders, damp		467											
6.10		some fine sand, some gravel	466	5	SS	50 for 150 mm								
			465	6	SS	50 for 150 mm								
7.5			464	7	SS	52								
8.60	CLAYEY SILT: Hard clayey silt, trace gravel, DTPL		463											
9.45			462	8	SS	62								
10.10	SAND: Very dense light brown fine sand, trace silt, damp becoming wet/saturated		461											
11.15				9	SS	50 for 150 mm								
	BOREHOLE TERMINATED AT 11.15 m												Upon completion of drilling, borehole open with groundwater seepage below 10.1 m	

NOTES

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LOG OF BOREHOLE NO. 6

PROJECT FIFE ROAD SUBDIVISION
LOCATION Fife Road, Grand Valley, Ontario
BORING METHOD Continuous Flight Solid Stem Augers

BORING DATE 2009 04 16

OUR PROJECT NO. 09KF021
ENGINEER M Molodecki
TECHNICIAN K. Hanes

SOIL PROFILE			SAMPLES			SHEAR STRENGTH C_u ▲				LIQUID LIMIT W_L _____			GROUND WATER OBSERVATIONS AND REMARKS	
DEPTH in METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3M N-VALUES	50 100 150 200				PLASTIC LIMIT W_p _____			
							DYNAMIC CONE PENETRATION ×				WATER CONTENT W _____			
							BLOWS/0.3M				WATER CONTENT %			
							20	40	60	80	10	20	30	
	GROUND ELEVATION 467.13													
0.20	TOPSOIL: Soft dark brown clayey silt, trace gravel, high organics, APL			1	SS	3	●					○		
	CLAYEY SILT TILL: Stiff clayey silt, trace gravel, numerous cobbles, APL/WTPL		466	2	SS	12	●					○		
1.50	becoming APL with occasional dry brown sand seams		465	3	SS	8	●					○		
				4	SS	17	●					○		
3.05	becoming very stiff with occasional wet gravelly sand seams		464	5	SS	44	●			▲		○		
	becoming hard, DTPL													
4.00			463											
4.5	SILT: Very dense brown silt, trace fine sand, moist to wet			6	SS	50 for 150 mm						○		
6.15		461		7	SS	50 for 125 mm						○		
6.55	SAND: Very dense light brown fine sand, moist													
	BOREHOLE TERMINATED AT 6.55 m											Upon completion of drilling, borehole open with no free water.		

NOTES

CHECKED BY

LOG OF BOREHOLE NO. 9

PROJECT FIFE ROAD SUBDIVISION

OUR PROJECT NO. 09KF021

LOCATION Fife Road, Grand Valley, Ontario

BORING DATE 2009 04 16

ENGINEER M. Molodecki

BORING METHOD Continuous Flight Hollow Stem Augers

TECHNICIAN K Hanes

SOIL PROFILE				SAMPLES			SHEAR STRENGTH C_u				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS
DEPTH in METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3m N-VALUES	50 100 150 200 ▲				PLASTIC LIMIT W_p	WATER CONTENT W		
							DYNAMIC CONE PENETRATION ×				W_p	W	W_L	
							STANDARD PENETRATION TEST ●				WATER CONTENT %			
							BLOWS/0.3M				10 20 30			
	GROUND ELEVATION 455.33													
0.10	SILT TOPSOIL: Dark brown silt, trace gravel, occasional cobbles and boulders, moist		455	1	SS	50*								
1.50	SAND: Dark brown medium sand, some gravel, some silt, numerous cobbles, occasional boulders, damp		454	2	SS	50*								
2.75	SAND AND GRAVEL: Brown sand and gravel, trace silt, numerous cobbles and boulders, saturated		453	3	SS	50*								
3.0	CLAYEY SILT TILL: Hard brown clayey silt, trace gravel, occasional cobbles and boulders, APL		452	4	SS	37								
4.00	becoming grey, DTPL													
4.5			451											
5.05	BOREHOLE TERMINATED AT 5.05 m			5	SS	50 for 125 mm								

Water Level Reading:
04/16/2009: 1.1 m

NOTES

CHECKED BY



APPENDIX A

ENGINEERED FILL

The information presented in this appendix is intended for general guidance only. Site specific conditions and prevailing weather may require modification of compaction standards, backfill type or procedures. Each site must be discussed, and procedures agreed with Peto MacCallum Ltd. prior to the start of the earthworks and must be subject to ongoing review during construction. This appendix is not intended to apply to embankments. Steeply sloping ravine residential lots require special consideration.

For fill to be classified as engineered fill suitable for supporting structural loads, a number of conditions must be satisfied, including but not necessarily limited to the following:

1. Purpose

The site specific purpose of the engineered fill must be recognized. In advance of construction, all parties should discuss the project and its requirements and agree on an appropriate set of standards and procedures.

2. Minimum Extent

The engineered fill envelope must extend beyond the footprint of the structure to be supported. The minimum extent of the envelope should be defined from a geotechnical perspective by:

- at founding level, extend a minimum 1.0 m beyond the outer edge of the foundations, greater if adequate layout has not yet been completed as noted below; and
- extend downward and outward at a slope no greater than 45° to meet the subgrade

All fill within the envelope established above must meet the requirements of engineered fill in order to support the structure safely. Other considerations such as survey control, or construction methods may require an envelope that is larger, as noted in the following sections.

Once the minimum envelope has been established, structures must not be moved or extended without consultation with Peto MacCallum Ltd. Similarly, Peto MacCallum Ltd. should be consulted prior to any excavation within the minimum envelope.

3. Survey Control

Accurate survey control is essential to the success of an engineered fill project. The boundaries of the engineered fill must be laid out by a surveyor in consultation with engineering staff from Peto MacCallum Ltd. Careful consideration of the maximum building envelope is required.

During construction it is necessary to have a qualified surveyor provide total station control on the three dimensional extent of filling.

4. Subsurface Preparation

Prior to placement of fill, the subgrade must be prepared to the satisfaction of Peto MacCallum Ltd. All deleterious material must be removed and in some cases, excavation of native mineral soils may be required.

Particular attention must be paid to wet subgrades and possible additional measures required to achieve sufficient compaction. Where fill is placed against a slope, benching may be necessary and natural drainage paths must not be blocked.

5. Suitable Fill Materials

All material to be used as fill must be approved by Peto MacCallum Ltd. Such approval will be influenced by many factors and must be site and project specific. External fill sources must be sampled, tested and approved prior to material being hauled to site.

6. Test Section

In advance of the start of construction of the engineered fill pad, the Contractor should conduct a test section. The compaction criterion will be assessed in consultation with Peto MacCallum Ltd. for the various fill material types using different lift thicknesses and number of passes for the compaction equipment proposed by the Contractor.

Additional test sections may be required throughout the course of the project to reflect changes in fill sources, natural moisture content of the material and weather conditions.

The Contractor should be particularly aware of changes in the moisture content of fill material. Site review by Peto MacCallum Ltd. is required to ensure the desired lift thickness is maintained and that each lift is systematically compacted, tested and approved before a subsequent lift is commenced.

7. Inspection and Testing

Uniform, thorough compaction is crucial to the performance of the engineered fill and the supported structure. Hence, all subgrade preparation, filling and compacting must be carried out under the full time inspection by Peto MacCallum Ltd.

All founding surfaces for all buildings and residential dwellings or any part thereof (including but not limited to footings and floor slabs) on structural fill or native soils must be inspected and approved by PML engineering personnel prior to placement of the base/subbase granular material and/or concrete. The purpose of the inspection is to ensure the subgrade soils are capable of supporting the building/house foundation and floor slab loads and to confirm the building/house envelope does not extend beyond the limits of any structural fill pads.

8. Protection of Fill

Fill is generally more susceptible to the effects of weather than natural soil. Fill placed and approved to the level at which structural support is required must be protected from excessive wetting, drying, erosion or freezing. Where adequate protection has not been provided, it may be necessary to provide deeper footings or to strip and recompact some of the fill.

9. Construction Delay Time Considerations

The integrity of the fill pad can deteriorate due to the harsh effects of our Canadian weather. Hence, particular care must be taken if the fill pad is constructed over a long time period.

It is necessary therefore, that all fill sources are tested to ensure the material compactability prior to the soil arriving at site. When there has been a lengthy delay between construction periods of the fill pad, it is necessary to conduct subgrade proof rolling, test pits or boreholes to verify the adequacy of the exposed subgrade to accept new fill material.

When the fill pad will be constructed over a lengthy period of time, a field survey should be completed at the end of each construction season to verify the areal extent and the level at which the compacted fill has been brought up to, tested and approved.

In the following spring, subexcavation may be necessary if the fill pad has been softened attributable to ponded surface water or freeze/thaw cycles.

A new survey is required at the beginning of the next construction season to verify that random dumping and/or spreading of fill has not been carried out at the site.

10. Approved Fill Pad Surveillance

It should be appreciated that once the fill pad has been brought to final grade and documented by field survey, there must be ongoing surveillance to ensure that the integrity of the fill pad is not threatened.

Grading operations adjacent to fill pads can often take place several months or years after completion of the fill pad.

It is imperative that all site management and supervision staff, the staff of Contractors and earthwork operators be fully aware of the boundaries of all approved engineered fill pads.

Excavation into an approved engineered fill pad should never be contemplated without the full knowledge, approval and documentation by the geotechnical consultant.

If the fill pad is knowingly built several years in advance of ultimate construction, the areal limits of the fill pad should be substantially overbuilt laterally to allow for changes in possible structure location and elevation and other earthwork operations and competing interests on the site. The overbuilt distance required is project and/or site specified.

Iron bars should be placed at the corner/intermediate points of the fill pad as a permanent record of the approved limits of the work for record keeping purposes.

11. Unusual Working Conditions

Construction of fill pads may at times take place at night and/or during periods of freezing weather conditions because of the requirements of the project schedule. It should be appreciated therefore, that both situations present more difficult working conditions. The Owner, Contractor, Design Consultant and Geotechnical Engineer must be willing to work together to revise site construction procedures, enhance field testing and surveillance, and incorporate design modifications as necessary to suit site conditions.

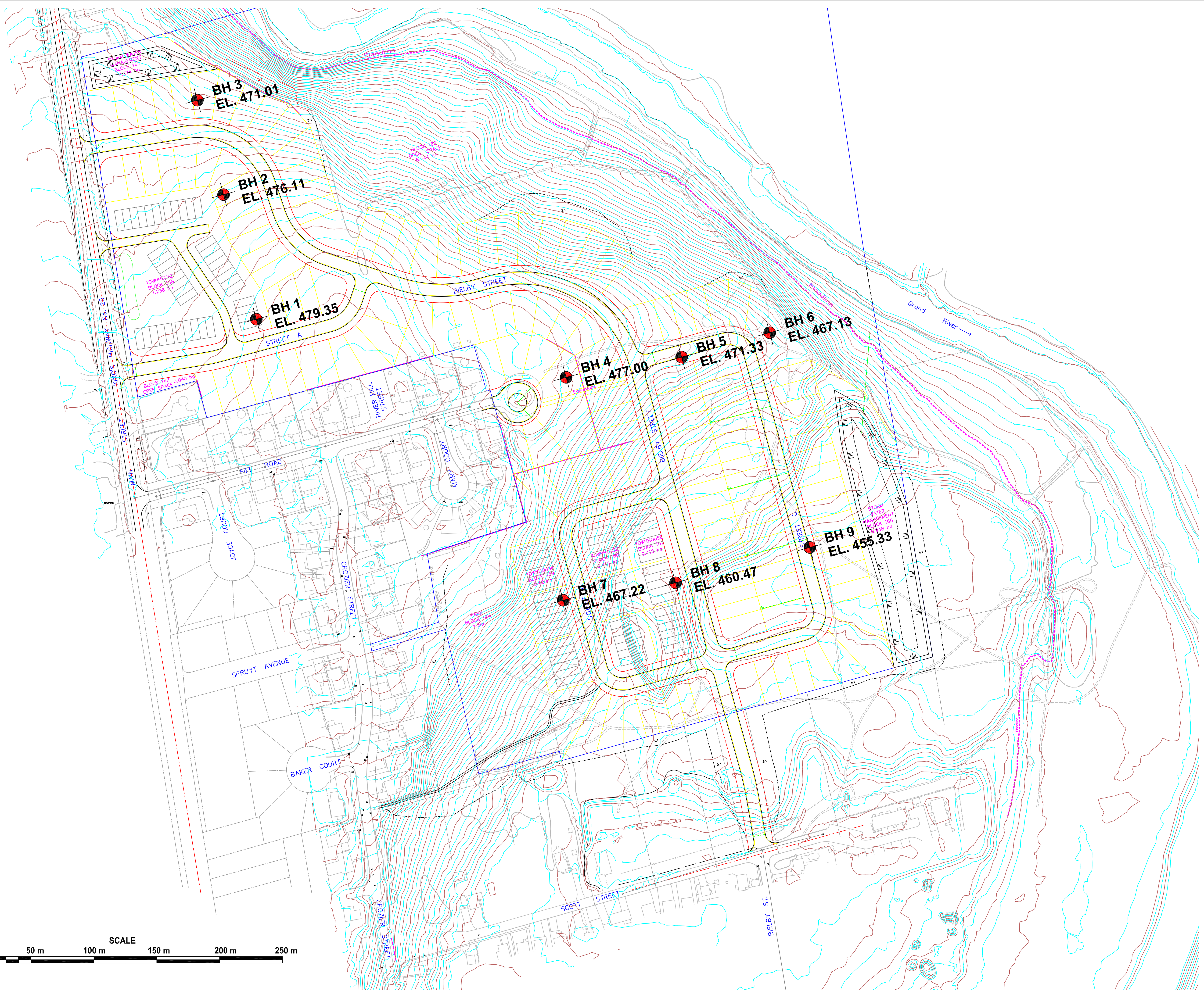
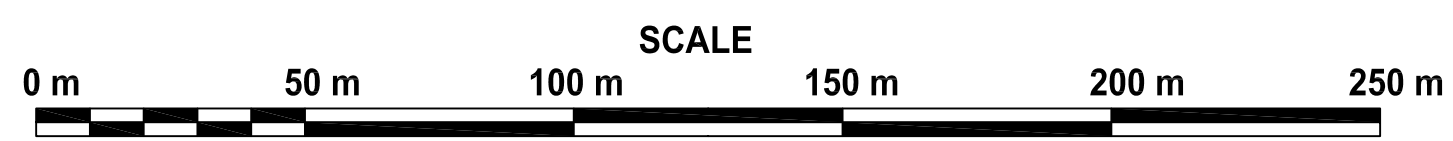
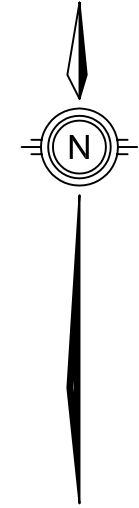
When working at night there must be sufficient artificial light to properly illuminate the fill pad and borrow areas.

Placement of material to form an engineered fill pad during winter and freezing temperatures has its own special conditions that must be addressed. It is imperative that each day prior to placement of new fill, the exposed subgrade must be inspected and any overnight snow or frozen material removed. Particular attention should be given to the borrow source inspection to ensure only nonfrozen fill is brought to the site.

The Contractor must continually assess the work program and have the necessary spreading and compacting equipment to ensure that densification of the fill material takes place in a minimum amount of time. Changes may be required to the spreading methods, lift thickness, and compaction techniques to ensure the desired compaction is achieved uniformly throughout each fill lift.

The Contractor should adequately protect the subgrade at the end of each shift to minimize frost penetration overnight. Since water cannot be added to the fill material to facilitate compaction, it is imperative that densification of the fill be achieved by additional compaction effort and an appropriate reduced lift thickness. Once the fill pad has been completed, it must be properly protected from freezing temperatures and ponding of water during the spring thaw period.

If the pad is unusually thick or if the fill thickness varies dramatically across the width or length of the fill pad, Peto MacCallum Ltd. should be consulted for additional recommendations. In this case, alternative special provisions may be recommended, such as providing a surcharge preload for a limited time or increase the degree of compaction of the fill.




NOTES
1. THE INFERRED STRATIGRAPHY REFERRED TO IN THIS REPORT IS BASED ON DATA FROM THESE BOREHOLES, SUPPLEMENTED BY GEOLOGICAL EVIDENCE. THE ACTUAL STRATIGRAPHY AT OTHER POINTS BETWEEN THE BOREHOLES MAY VARY FROM THAT SHOWN.
2. THIS DRAWING WAS REPRODUCED FROM A PLAN PROVIDED BY CLIENT.

GAMSBY AND MANNEROW LIMITED

FIFE ROAD SUBDIVISION
GRAND VALLEY
ONTARIO

BOREHOLE LOCATION PLAN

 **Peto MacCallum Ltd.**
CONSULTING ENGINEERS

DRAWN: KH	DATE:	SCALE:	JOB NO.:	DRAWING NO.:
CHECKED: MSM	JUNE 2009	AS SHOWN	09KF021	1
APPROVED: MSM				



**GEOTECHNICAL SLOPE ASSESSMENT
FIFE ROAD SUBDIVISION
GRAND VALLEY, ONTARIO**
for
GAMSBY AND MANNEROW LIMITED

PETO MacCALLUM LTD.
16 FRANKLIN STREET SOUTH
KITCHENER, ONTARIO
N2C 1R4
PHONE: (519) 893-7500
FAX: (519) 893-0654
EMAIL: kitchener@petomacallum.com

Distribution:
4 cc: Gamsby and Mannerow Limited
1 cc: PML Kitchener
1 cc: PML Toronto

PML Ref.: 09KF021A
Report: 1
February 9, 2012

February 9, 2012

PML Ref.: 09KF021A
Report: 1

Mr. Grant Campbell, BAsC, EIT
Gamsby and Mannerow Limited
650 Woodlawn Road West
Block C, Unit 2
Guelph, Ontario
N1K 1B8

Dear Mr. Campbell

Geotechnical Slope Hazard Assessment
Fife Road Subdivision
Grand Valley, Ontario

We are pleased to present our report on the geotechnical site assessment recently carried out at the above referenced site. This work was authorized by Mr. Grant Campbell in an email received January 18, 2012.

The project involves a planned 115 lot residential subdivision extending to the north and east of the existing Fife Road Subdivision in Grand Valley, Ontario. Peto MacCallum Ltd. has completed a prior geotechnical investigation for the development, with the findings presented in our Geotechnical Investigation, PML Ref.: 09KF021, Report 1, dated June 3, 2009.

The lot layout has now been completed. The purpose of the slope hazard assessment was to determine the safe set-back distance for lots backing onto the west bank of the Grand River, in accordance with Grand Valley Conservation Authority (GRCA) published policy document titled "Policies for the Administration of the Development, Interference with Wetlands and Alterations to Shorelines and Watercourses Regulation" dated November 30, 2007, made under O.Reg. 150/06 of the Conservation Authorities Act.



Methodology

The scope of work for this assignment included the following:

- A review of lot plans and site topography as shown on Gamsby and Mannerow 'Preliminary Grading Plan, Drawings A and B', for Fife Subdivision, dated June, 2009;
- A review of the geotechnical report including Borehole 3 and 6 specifically, where there was some potential that the safe building setback could encroach close to or into the proximal lots as identified from the topographic contours;
- A site visit completed on January 21, 2012, to conduct a visual slope assessment at the two potential hazard locations; and
- A review of the local geology and historical aerial photography and topographic mapping over a 41 year time period, as tabulated below:

TABLE I
AERIAL PHOTOGRAPHS, TOPOGRAPHIC MAPS, GEOLOGIC MAPS

1. Aerial Photographs		
TIME PERIOD	SCALE	SOURCE
2000	1:10,000	GRCA Website
2010	1:10,000	GRCA Website
2. Geologic Maps		
1971	1:50,000	Ontario Division of Mines, Preliminary Map P. 848, 'Quaternary Geology of the Orangeville Area' published 1973.
3. Topographic Maps		
1976	1:50,000	Department of Energy, Mine and Resources, Surveys and Mapping Branch, National Topographic System Map Index 40 P/16 'Orangeville' Edition 3
1980	1:50,000	Energy, Mine and Resources Canada, Surveys and Mapping Branch, National Topographic System Map Index 40 P/16 'Orangeville' Edition 4



Findings and Observations

Site Description and Topography

The overall subdivision development, including the lot layout and site topography at 0.5 m contour intervals, is shown on the appended Hazard Limits drawing. PML has identified two locations adjacent to the west bank of the Grand River, where the slope hazard limit line could potentially encroach into or close to the proximal building lots. The site reconnaissance and detailed research for this assignment was focused on these two areas. Both locations feature riverbank slopes with local inclinations approaching two horizontal to one vertical (2H:1V), as based on the topographic contours and site observations.

The cross sectional geometry was calculated at the steepest section of each area as illustrated on the appended drawing.

At Area 1, located near Borehole 3, the average slope inclination is 2.3H:1V. The slope height from toe to crest is about 15 to 15.5 m. The toe of the slope at the river's edge exhibits evidence of slow, ongoing toe erosion as evidenced by a 0.5 to 1.0 m high cut standing at a near vertical inclination. The eroded face at the toe consists of silt till-like material with a dense mat of exposed tree roots. The slope itself is densely wooded with mixed hardwoods and stands of cedar. Numerous cedar of up 200 mm diameter were inclined at about 45° within the bottom 1 m of the tree trunk, gradually curving to a near vertical posture at a height of about 0.5 to 1.0 m above grade. The tree trunk curvature is indicative of ongoing solifluction movement of the slope surface.

Area 2 in the vicinity of Borehole 6 exhibits a 13 m high riverbank slope with an average toe to crest inclination of 3.9H:1V, and with a maximum inclination of 2.4H:1V near the top 4 m of the slope face. The toe of slope at the water's edge exhibited similar erosional features as Area 1, with an eroded face of up to 1.5 m in height. The overall slope face was fully wooded with cedar tree vegetation, with no overt evidence of solifluction movement.



There were no signs found of slope failure, active erosional gullies, springs or seepage in either area examined.

Subsurface Conditions

The soil stratigraphy in Borehole 3 (Area 1) comprises very stiff to hard clayey silt till to a depth of 4.05 m, underlain by stiff clayey silt till to the borehole termination depth of 5.05 m.

Borehole 6 near Area 2, disclosed stiff to hard clayey silt till to a depth of 4.00 m, underlain by very dense silt to a depth of 6.15 m, underlain by dense sand to the borehole termination at a depth of 6.55 m.

Groundwater was not contacted within the depth explored at either borehole.

The remaining boreholes within the subdivision disclosed similar high strength soils to a lowest explored elevation of 450.28.

The site geology shown on Ontario Division of Mines, Preliminary Map P. 848, 'Quaternary Geology of the Orangeville Area', indicates that the riverbank at both locations is comprised of silt till of the Georgian Bay Lobe.

A copy of each log for Boreholes 3 and 6 is appended for ease of reference.

Discussions and Recommendations

The angle of internal friction of the predominant clayey silt till deposit is estimated to be between 35 and 38°. The factor of safety against deep seated failure of the slope is estimated at 1.6 to 1.8.

The long term regression of these slopes will be dictated by solifluction movement being mobilized by ongoing erosion at the toe.

The riverbank slope at both locations is classified as a confined system with active toe erosion. The regulated development limit for this setting is established based on a toe erosion allowance, a stable slope angle, plus a 15 m allowance beyond the Erosion Hazard Limit. This is illustrated in the sketch below, excerpted from the GRCA policy document.

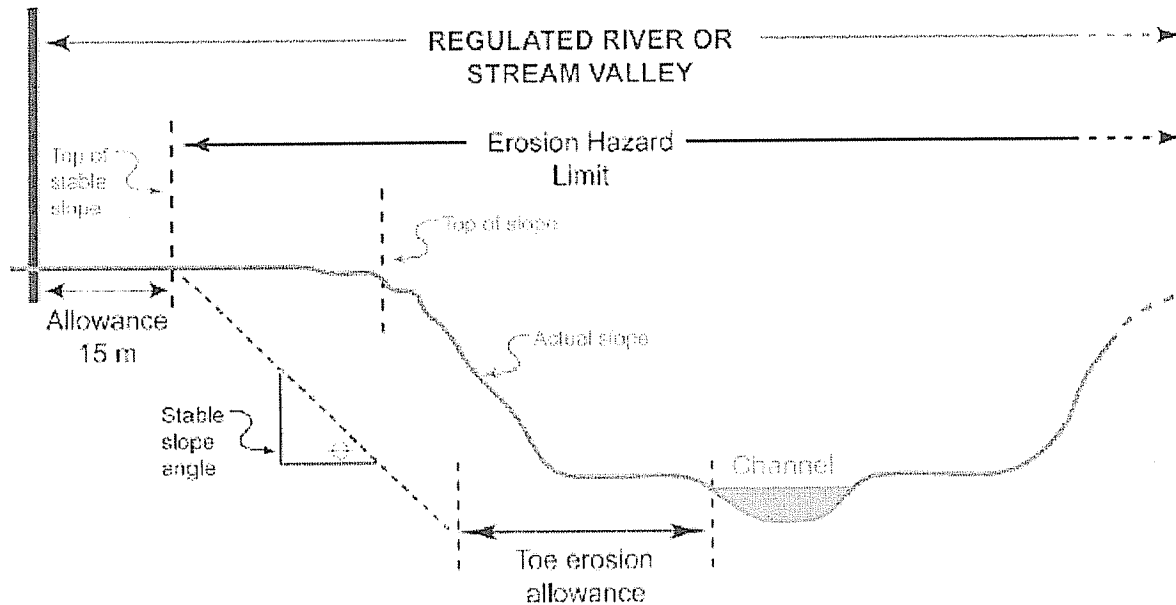


Figure 6. Riverine Erosion Hazard – Regulated Area for Apparent Oversteepened Valleys with Active Toe Erosion

Area 1 is located on the outside of a local meander loop, positioned about 70 m from the project path of a straight river alignment. Assuming that 70 m of movement has occurred over a 10,000 year time period since the recession of the Wisconsin glaciation, an average erosion rate of 0.7 m per 100 years is inferred. The typical toe erosion allowance in dense, silt till subsoil setting is in the order of 1 m per 100 years. Although there was no erosion evident on the mapping and aerial photography researched, a Toe Erosion Allowance of 2 m per 100 years is estimated based on the observed scour at the toe and evidence of solifluction movement. The stable slope angle is conservatively estimated at 2.3H:1V, equivalent to the current average inclination at the steepest cross section in Area 1. For purposes of graphing the Hazard Limit line, the eroded toe of slope was taken as Elevation 454.0. The Hazard Limit thus established is illustrated on Drawing 1, appended.



Where a geotechnical study has been completed, Section 8.2.3 of the GRCA policy generally permits building construction to within the Erosion Hazard Allowance, providing the lots are set back a minimum of 6 m from the Hazard Limit. The permissible property line limit thus established for the lots is also illustrated on Drawing 1. Beyond the two areas studied in detail, the lot limit may be taken as 6 m from the crest of the slope, which is generally defined by the existing tree line.

It is noted that a storm water management pond will be located near the crest of the slope in Area 1. In accordance with Section 8.1.20 of the GRCA policy document, this will be permissible providing it is a dug pond and it does not encroach within the Hazard Limit. However, a portion of the perimeter fill berm along the north side of the pond encroaches beyond the Hazard Limit, which contravenes GRCA policy with regard to fill placement on slopes. The pond layout and grading plans will need to be revised accordingly.

Significant storm runoff discharged over the slope in unprotected channels will result in erosional problems. We therefore recommend that any storm discharge over the slope should generally be in controlled channels.

Along the crest of the existing slope, the existing ground surface should be maintained, as is, and vegetation cover should be maintained. During construction, fill and/or construction materials should be kept away from this area. As well, material should not be dumped over the crest of the slopes.

In addition, erosion control measures should be observed for the entire slope. Typically this would require maintenance of a healthy vegetation cover.



We trust this report has been completed within our terms of reference, and is sufficient for your immediate requirements. If you have any questions or require further information, please do not hesitate to contact our office.

Sincerely

Peto MacCallum Ltd.



Marian S. Molodecki, P.Eng.
Senior Consultant,
Geotechnical and Geoenvironmental Services



Gerry Mitchell, MEng, P.Eng.
Director
Branch Manager

MSM:sh

Enclosures:

List of Abbreviations
Log of Boreholes 3 and 6 from 09KF021
Drawing 1 – Hazard Limits

LIST OF ABBREVIATIONS



PENETRATION RESISTANCE

Standard Penetration Resistance N: - The number of blows required to advance a standard split spoon sampler 0.3 m into the subsoil. - Driven by means of a 63.5 kg hammer falling freely a distance of 0.76 m.

Dynamic Penetration Resistance: The number of blows required to advance a 51 mm, 60 degree cone, fitted to the end of drill rods, 0.3 m into the subsoil. The driving energy being 475 J per blow.

DESCRIPTION OF SOIL

The consistency of cohesive soils and the relative density or denseness of cohesionless soils are described in the following terms:

<u>CONSISTENCY</u>	<u>N (blows/0.3 m)</u>	<u>c (kPa)</u>	<u>DENSENESS</u>	<u>N (blows/0.3 m)</u>
Very Soft	0 - 2	0 - 12	Very Loose	0 - 4
Soft	2 - 4	12 - 25	Loose	4 - 10
Firm	4 - 8	25 - 50	Compact	10 - 30
Stiff	8 - 15	50 - 100	Dense	30 - 50
Very Stiff	15 - 30	100 - 200	Very Dense	> 50
Hard	> 30	> 200		
WTPL	Wetter Than Plastic Limit			
APL	About Plastic Limit			
DTPL	Drier Than Plastic Limit			

TYPE OF SAMPLE

SS	Split Spoon	TW	Thinwall Open
WS	Washed Sample	TP	Thinwall Piston
SB	Scraper Bucket Sample	OS	Oesterberg Sample
AS	Auger Sample	FS	Foil Sample
CS	Chunk Sample	RC	Rock Core
ST	Slotted Tube Sample	USS	Undisturbed Shear Strength
PH	Sample Advanced Hydraulically	RSS	Remoulded Shear Strength
PM	Sample Advanced Manually		

SOIL TESTS

Qu	Unconfined Compression	LV	Laboratory Vane
Q	Undrained Triaxial	FV	Field Vane
Qcu	Consolidated Undrained Triaxial	C	Consolidation
Qd	Drained Triaxial		

LOG OF BOREHOLE NO. 3

PROJECT FIFE ROAD SUBDIVISION
LOCATION Fife Road, Grand Valley, Ontario
BORING METHOD Continuous Flight Solid Stem Augers

BORING DATE 2009 04 17

OUR PROJECT NO. 09KF021
ENGINEER M Molodecki
TECHNICIAN K Hanes

SOIL PROFILE			SAMPLES		SHEAR STRENGTH C_u				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS	
DEPTH In METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3M N - VALUES	50	100	150	200	PLASTIC LIMIT W_p		WATER CONTENT W
							DYNAMIC CONE PENETRATION x STANDARD PENETRATION TEST o				WATER CONTENT %		
							BLOWS/0.3M						
							20	40	60	80	10	20	30
	GROUND ELEVATION 471.02												
0.60	TOPSOIL: Firm to stiff dark brown clayey silt, trace gravel, occasional cobbles, rootlets, APL		470	1	SS	8							
1.50	CLAYEY SILT TILL: Very stiff to hard brown clayey silt, trace gravel, occasional cobbles and boulders, APL/DTPL		469	2	SS	22							
2.30	occasional wet medium sand seams no sand seams		468	3	SS	30							
			467	4	SS	28							
3.0			466	5	SS	32							
4.05	becoming stiff, grey, APL		465	6	SS	13							
5.05	BOREHOLE TERMINATED AT 5.05 m												Water Level Reading: 04/17/2009: No water

NOTES

CHECKED BY

LOG OF BOREHOLE NO. 6

PROJECT FIFE ROAD SUBDIVISION
 LOCATION Fife Road, Grand Valley, Ontario
 BORING METHOD Continuous Flight Solid Stem Augers

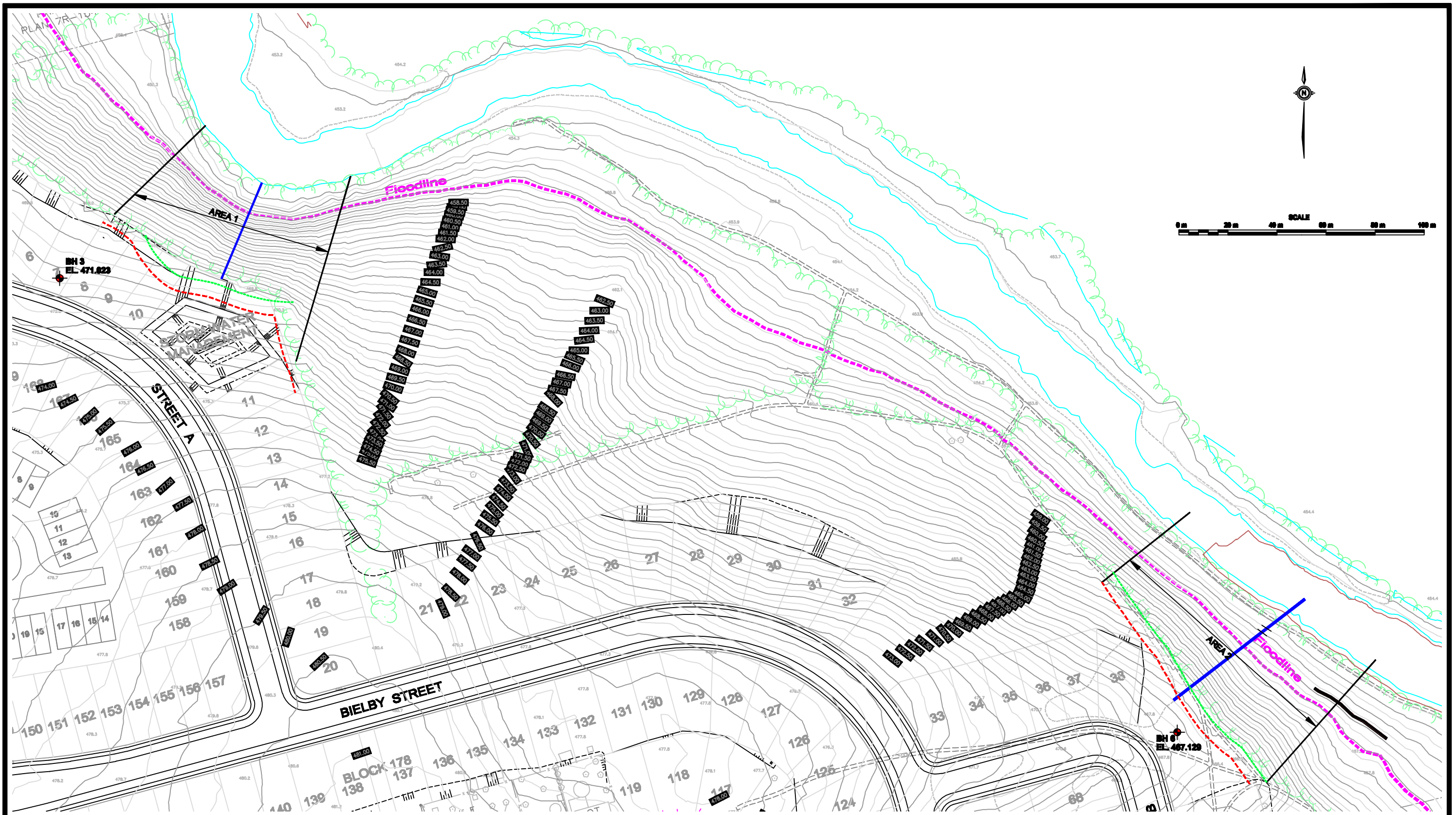
BORING DATE 2009 04 16

OUR PROJECT NO. 09KF021
 ENGINEER M Molodecki
 TECHNICIAN K Hanes

SOIL PROFILE			SAMPLES		SHEAR STRENGTH C_u				LIQUID LIMIT W_L			GROUND WATER OBSERVATIONS AND REMARKS		
DEPTH In METRES	DESCRIPTION	LEGEND	ELEVATION	NUMBER	TYPE	BLOWS/0.3m N-VALUES	50 100 150 200				PLASTIC LIMIT W_p			
							DYNAMIC CONE PENETRATION x STANDARD PENETRATION TEST				WATER CONTENT W			
							BLOWS/0.3M				WATER CONTENT %			
							20	40	60	80	10	20	30	
	GROUND ELEVATION 467.13													
0.20	TOPSOIL: Soft dark brown clayey silt, trace gravel, high organics, APL			1	SS	3								
	CLAYEY SILT TILL: Stiff clayey silt, trace gravel, numerous cobbles, occasional boulders, APL/WTPL		466	2	SS	12								
1.50	becoming APL with occasional dry brown sand seams		465	3	SS	8								
			464	4	SS	17								
2.75			463	5	SS	44								
3.05	becoming very stiff with occasional wet gravelly sand seams		462	6	SS	50 for 150 mm								
4.00	becoming hard, DTPL		461	7	SS	50 for 125 mm								
4.5	SILT: Very dense brown silt, trace fine sand, moist to wet													
6.0														
6.15	SAND: Very dense light brown fine sand, moist													
6.55	BOREHOLE TERMINATED AT 6.55 m												Upon completion of drilling, borehole open with no free water	

NOTES

CHECKED BY *[Signature]*



REFERENCE:
BOREHOLE LOCATION PLAN REPRODUCED FROM DRAWING SUPPLIED BY CLIENT.

NOTE:
THE INFERRED STRATIGRAPHY REFERRED TO IN THE REPORT IS BASED ON THE DATA FROM THESE BOREHOLES SUPPLEMENTED BY GEOLOGICAL EVIDENCE. THE ACTUAL STRATIGRAPHY BETWEEN THE BOREHOLES MAY VARY.

- LEGEND:**
- BOREHOLE FROM PREVIOUS INVESTIGATION (PML REF NO. 09KF021, REPORT 1)
 - ANALYSED PROFILE
 - HAZARD LIMIT
 - PROPERTY LINE LIMITS FOR LOTS
 - TREE LINE

GAMSBY AND MANNEROW LIMITED
GEOTECHNICAL SLOPE HAZARD ASSESSMENT
 FIFE ROAD SUBDIVISION
 GRAND VALLEY, ONTARIO

HAZARD LIMITS

Peto MacCallum Ltd.		CONSULTING ENGINEERS			
DRAWN	K. HANES	DATE	FEBRUARY 2012	SCALE	1:1500
CHECKED	M. MOLODECKI	PML REF.	09KF021A	DWG. NO.	1
APPROVED	M. MOLODECKI				

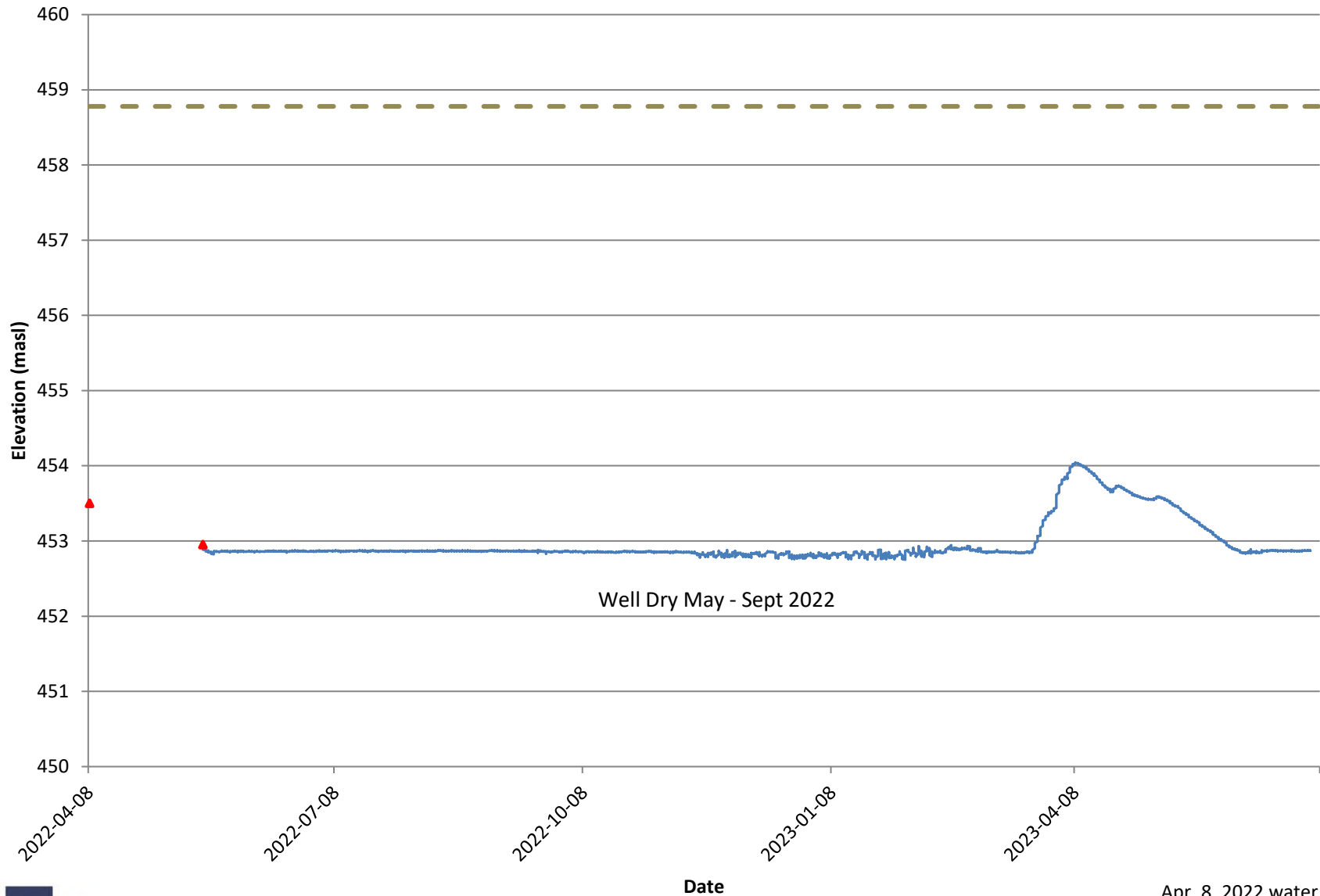


APPENDIX B

Groundwater Measurements by GM BluePlan Engineering
Limited



MW1



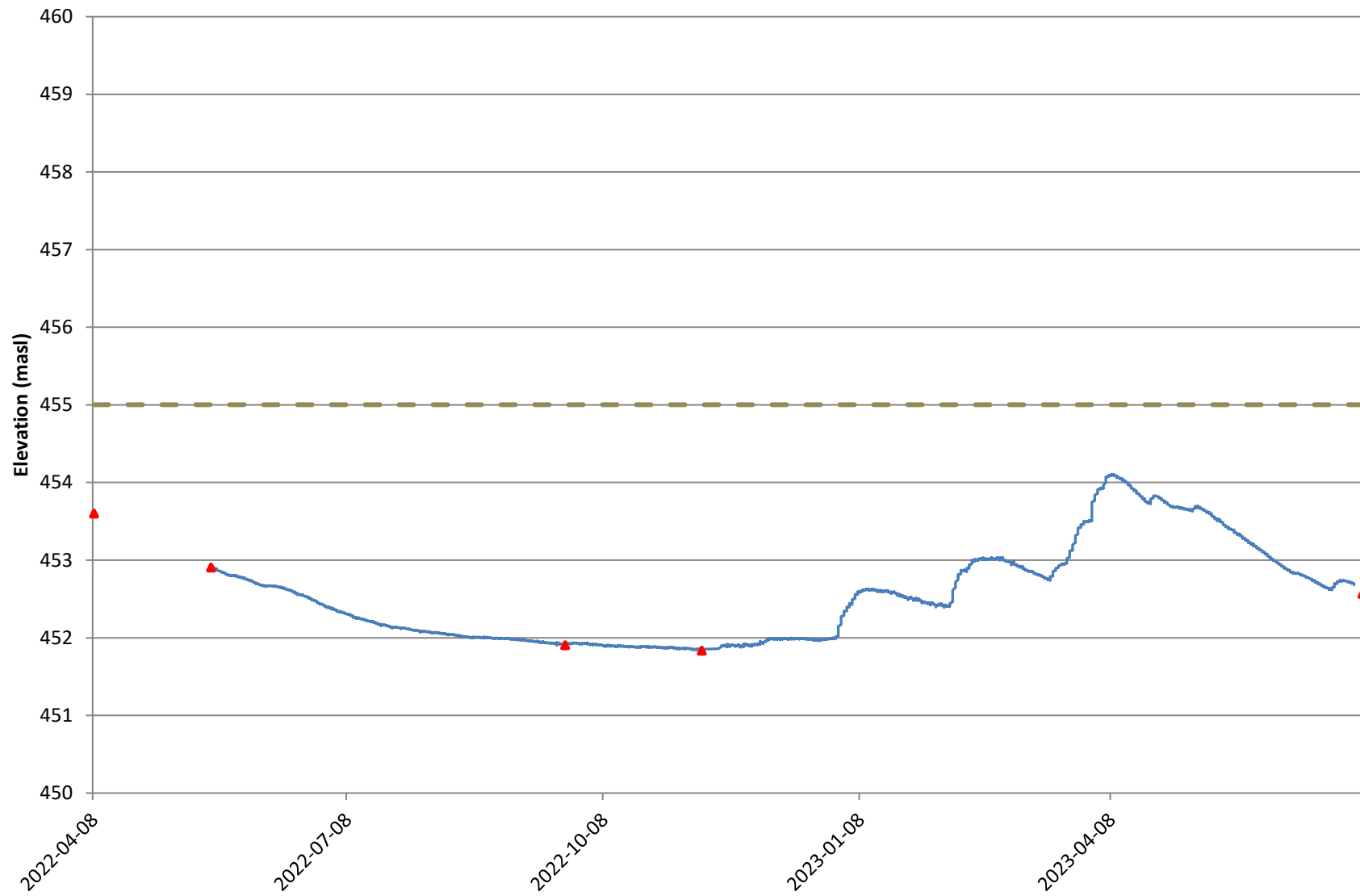
— Groundwater Level

▲ Manual Readings

— Ground Surface

Apr. 8, 2022 water level
by JLP Services Inc.

MW2



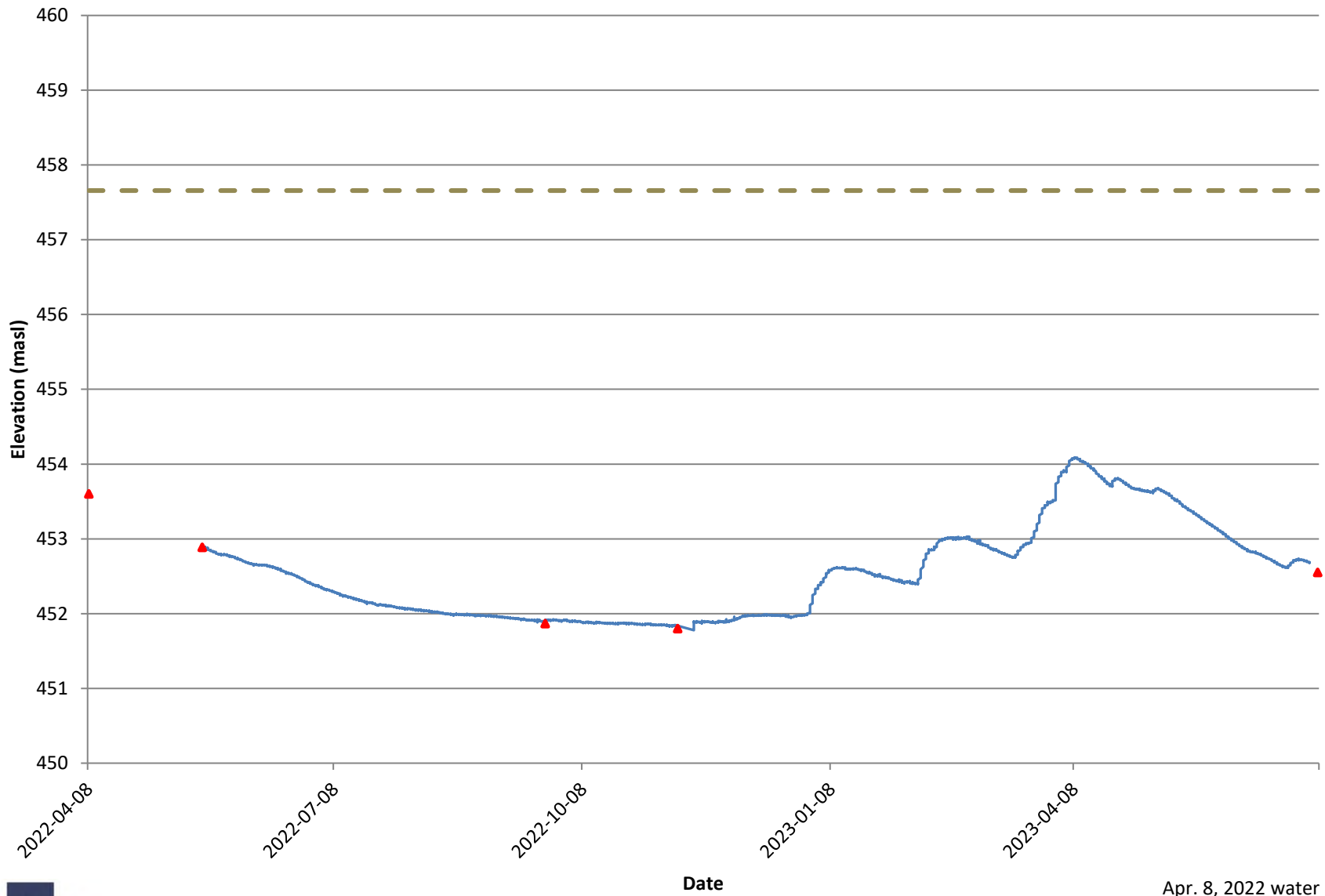
— Groundwater Level

▲ Manual Readings

- - - Ground Surface

Apr. 8, 2022 water level
by JLP Services Inc.

MW3



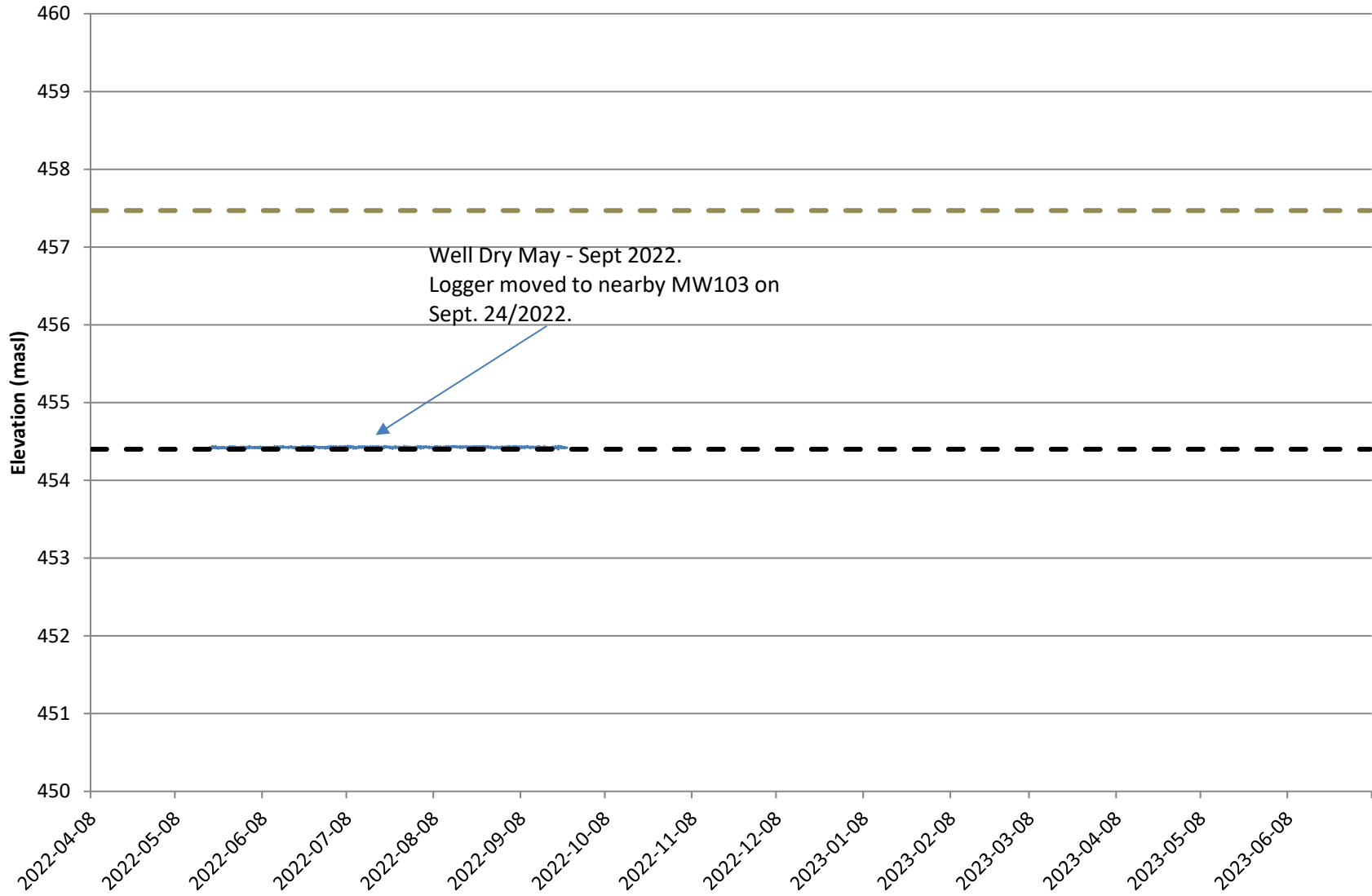
— Groundwater Level

▲ Manual Readings

- - - Ground Surface

Apr. 8, 2022 water level
by JLP Services Inc.

MW4



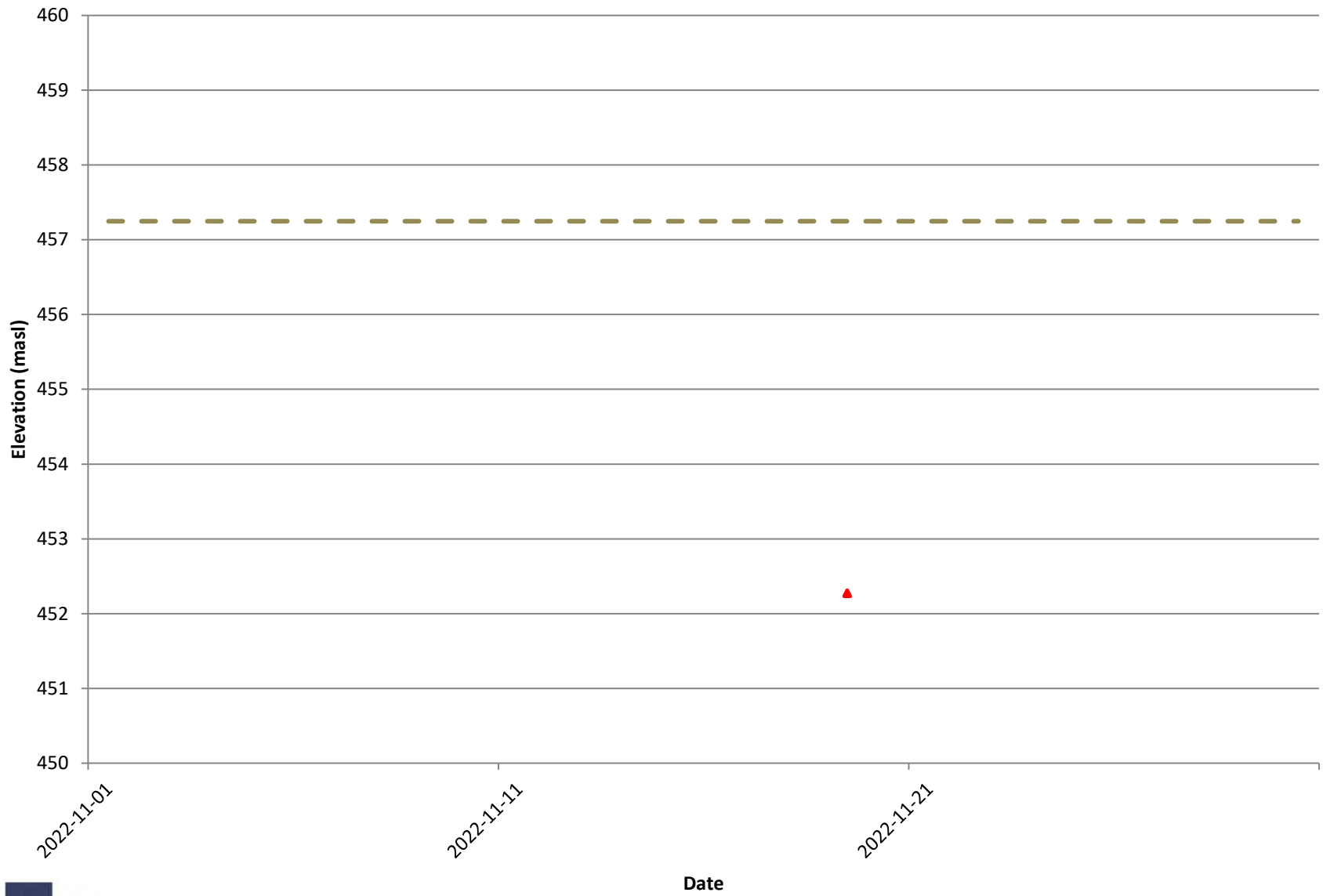
Date

— Groundwater Level ▲ Manual Readings

— Ground Surface — Well bottom

Apr. 8, 2022 water level
by JLP Services Inc. - well
dry

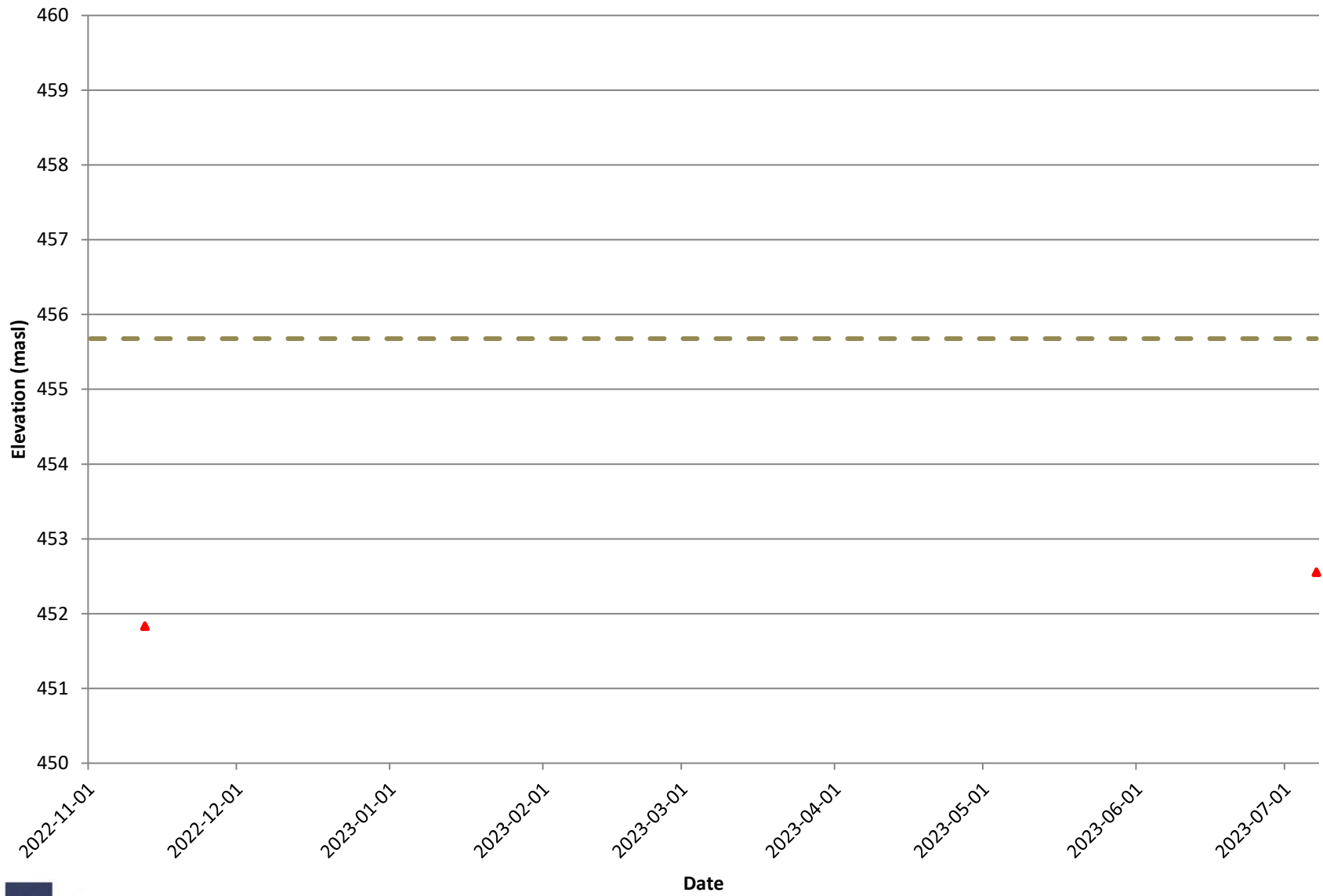
MW101



▲ Manual Readings

— Ground Surface

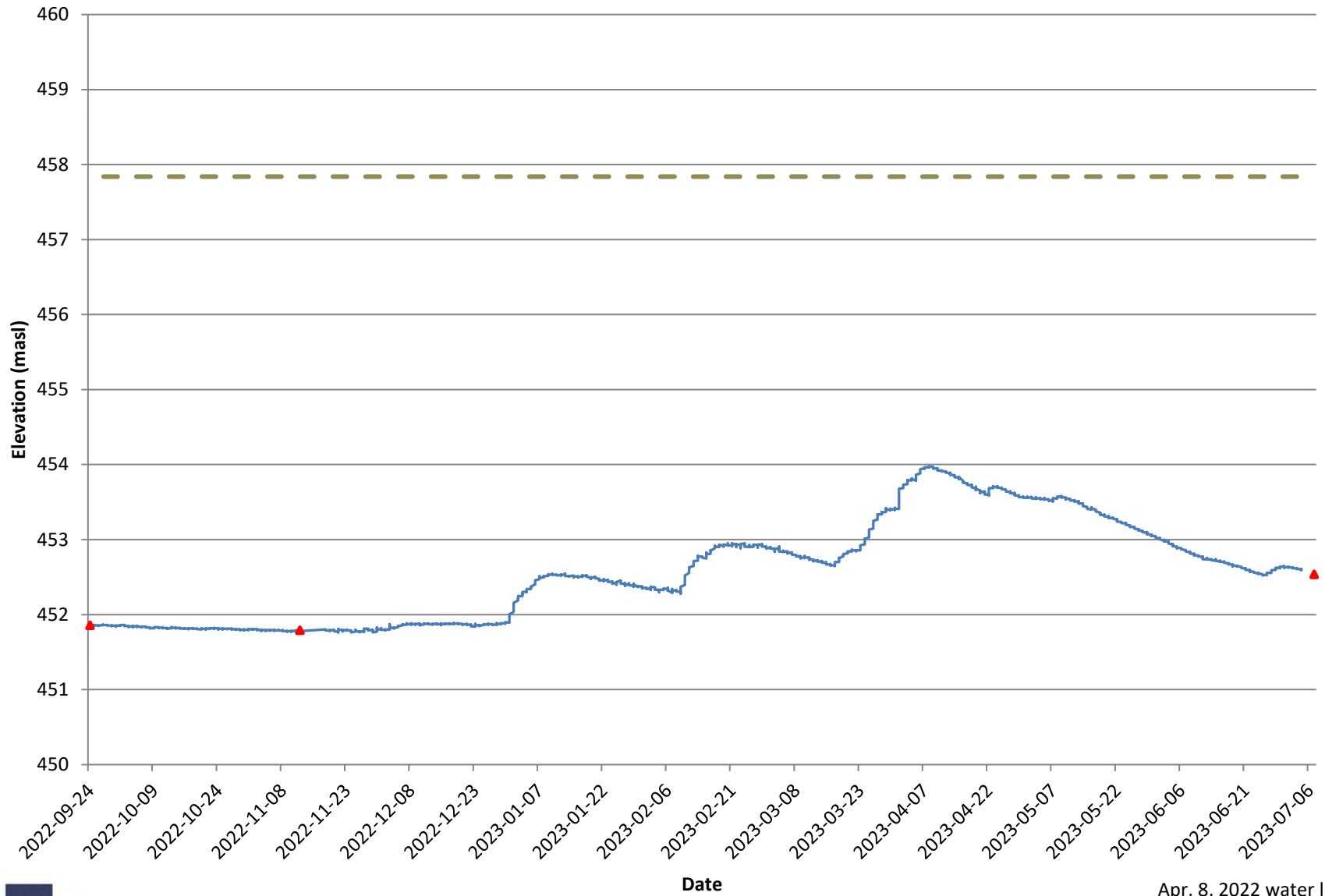
MW102



▲ Manual Readings

— Ground Surface

MW103



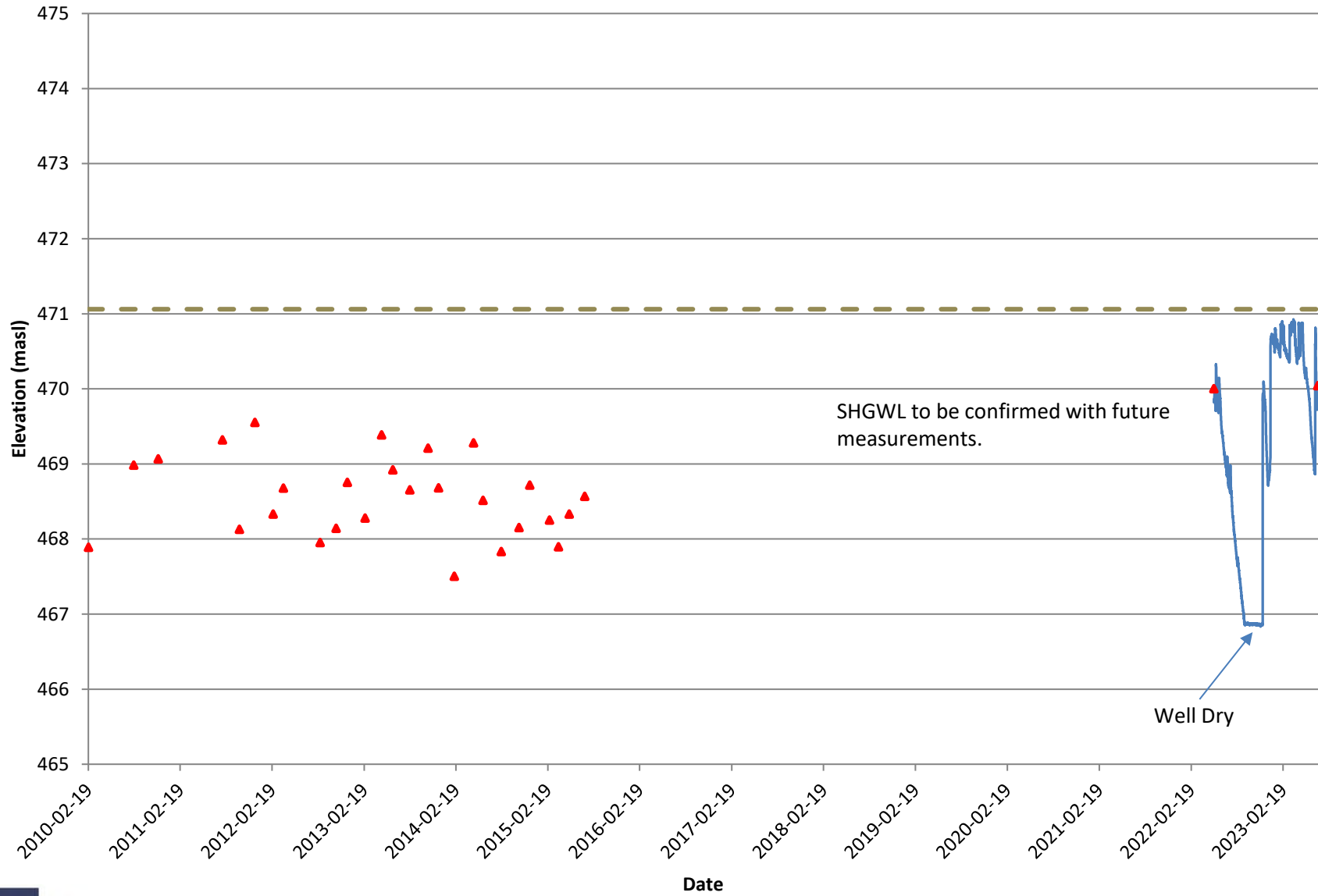
— Groundwater Level

▲ Manual Readings

— Ground Surface

Apr. 8, 2022 water level
by JLP Services Inc.

BH3



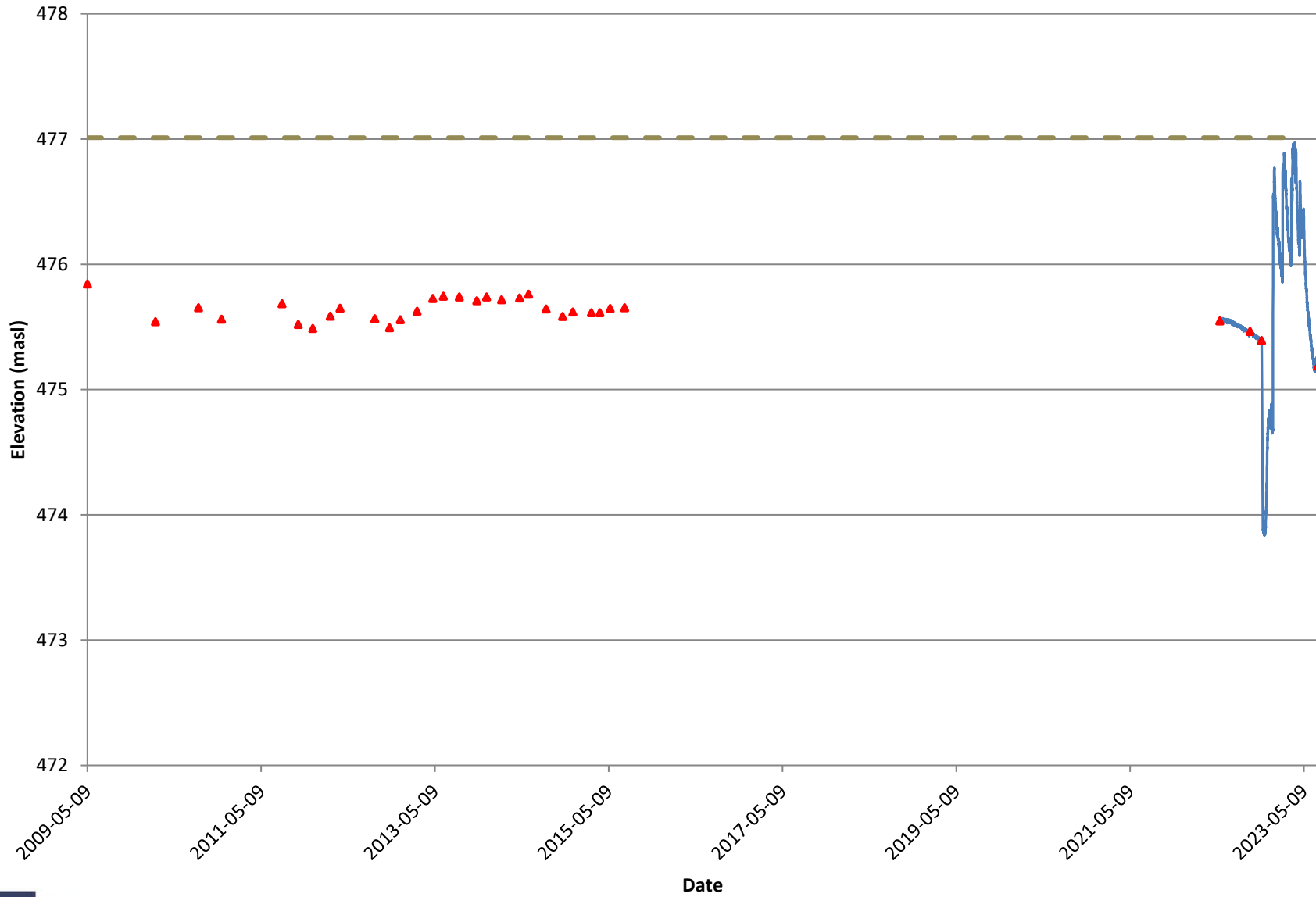
SHGWL to be confirmed with future measurements.

Well Dry



— Groundwater Level ▲ Manual Readings - - - Ground Surface

BH4

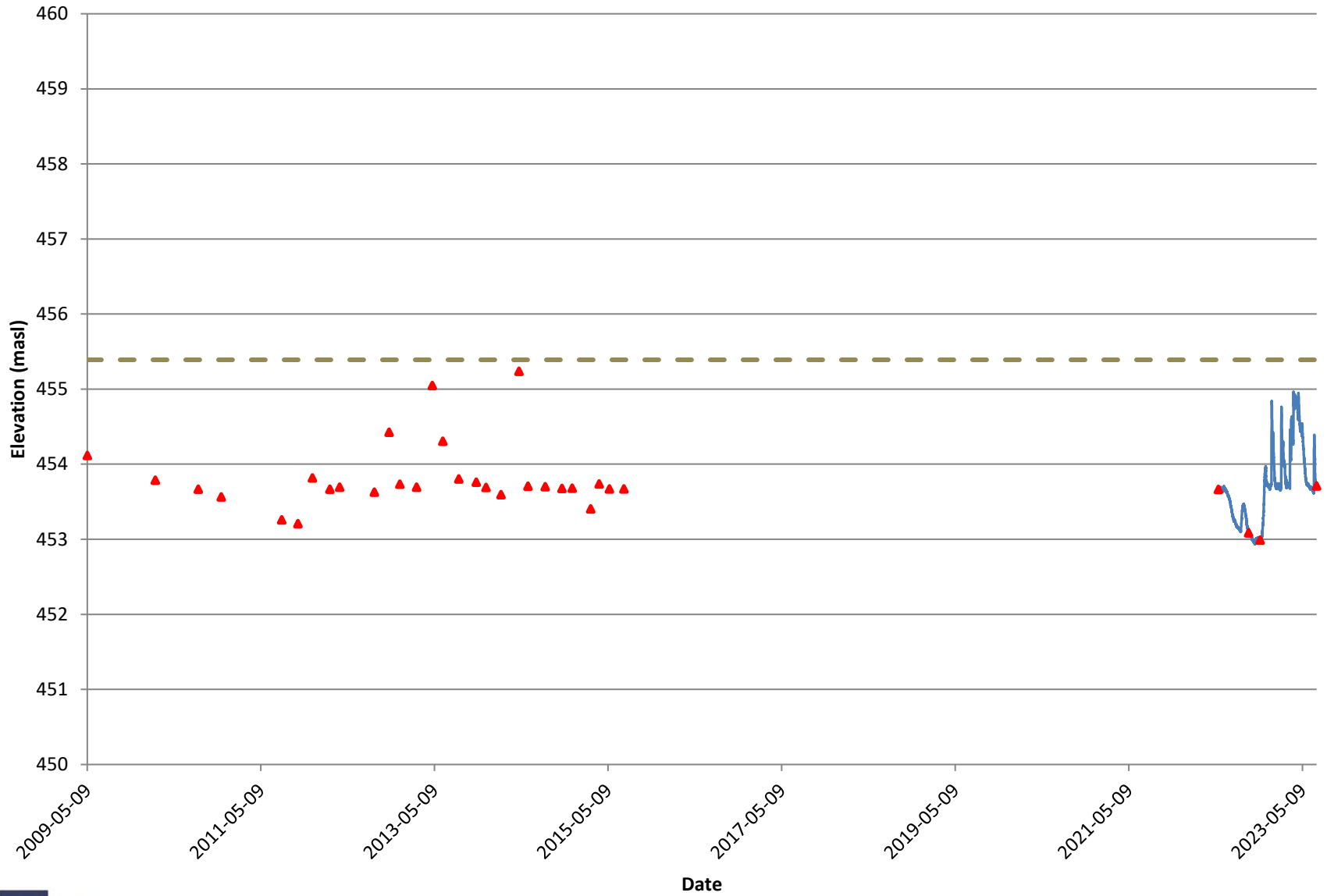


— Groundwater Level

▲ Manual Readings

— Ground Surface

BH9



— Groundwater Level ▲ Manual Readings - - - Ground Surface



APPENDIX C

Pre-Development Conditions
Stormwater Management Analysis



```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                     104-104 existing 002 year.out"
"          Licensee name:                       gmbp"
"          Company                              "
"          Date & Time last used:               2/21/2023 at 1:49:38 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.050 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                          33.014  mm"
"          6  002hyd Hydrograph extension used in this file"
" 33      CATCHMENT 105"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          105 Catchment 105"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.072  0.000  0.000  0.000 c.m/sec"

```

	Catchment 105	Pervious	Impervious	Total Area	"
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	22.062	2.281	3.430	minutes"
"	Time to Centroid	96.472	86.938	87.492	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	99.54	121.66	221.19	c.m"
"	Rainfall losses	30.676	2.000	14.904	mm"
"	Runoff depth	2.338	31.014	18.110	mm"
"	Runoff volume	7.05	114.29	121.33	c.m"
"	Runoff coefficient	0.071	0.939	0.549	"
"	Maximum flow	0.004	0.072	0.072	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.072 0.072 0.000 0.000"

" 33 CATCHMENT 100"

" 1 Triangular SCS"

" 1 Equal length"

" 2 Horton equation"

" 100 Catchment 100"

" 0.000 % Impervious"

" 4.920 Total Area"

" 175.000 Flow length"

" 8.000 Overland Slope"

" 4.920 Pervious Area"

" 175.000 Pervious length"

" 8.000 Pervious slope"

" 0.000 Impervious Area"

" 175.000 Impervious length"

" 8.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 75.000 Pervious Max.infiltration"

" 5.000 Pervious Min.infiltration"

" 0.250 Pervious Lag constant (hours)"

" 5.000 Pervious Depression storage"

" 0.015 Impervious Manning 'n'"

" 0.000 Impervious Max.infiltration"

" 0.000 Impervious Min.infiltration"

" 0.050 Impervious Lag constant (hours)"

" 1.500 Impervious Depression storage"

" 0.042 0.072 0.000 0.000 c.m/sec"

	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	4.920	0.000	4.920	hectare"
"	Time of concentration	41.936	4.335	41.935	minutes"
"	Time to Centroid	114.284	89.957	114.283	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	1624.29	0.00	1624.29	c.m"
"	Rainfall losses	30.670	1.796	30.669	mm"
"	Runoff depth	2.344	31.218	2.344	mm"
"	Runoff volume	115.35	0.00	115.35	c.m"
"	Runoff coefficient	0.071	0.000	0.071	"

"		Maximum flow	0.042	0.000	0.042	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.042	0.076	0.000	0.000"
" 33		CATCHMENT 101"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	101	Catchment 101"				
"	0.000	% Impervious"				
"	8.490	Total Area"				
"	130.000	Flow length"				
"	10.000	Overland Slope"				
"	8.490	Pervious Area"				
"	130.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	130.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.093	0.076	0.000	0.000 c.m/sec"
"		Catchment 101	Pervious	Impervious	Total Area	"
"		Surface Area	8.490	0.000	8.490	hectare"
"		Time of concentration	32.814	3.392	32.813	minutes"
"		Time to Centroid	106.157	88.635	106.157	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	2802.88	0.00	2802.89	c.m"
"		Rainfall losses	30.669	2.144	30.669	mm"
"		Runoff depth	2.345	30.870	2.345	mm"
"		Runoff volume	199.06	0.00	199.06	c.m"
"		Runoff coefficient	0.071	0.000	0.071	"
"		Maximum flow	0.093	0.000	0.093	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.093	0.144	0.000	0.000"
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"			0.093	0.144	0.144	0.000"
" 40		HYDROGRAPH Combine 1000"				
"	6	Combine "				
"	1000	Node #"				

"	Grand River"				
"	Maximum flow	0.144		c.m/sec"	
"	Hydrograph volume	435.741		c.m"	
"		0.093	0.144	0.144	0.144"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		0.093	0.000	0.144	0.144"
" 33	CATCHMENT 106"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	106 Catchment 106"				
"	55.000 % Impervious"				
"	1.090 Total Area"				
"	110.000 Flow length"				
"	2.000 Overland Slope"				
"	0.491 Pervious Area"				
"	110.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.600 Impervious Area"				
"	110.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.125	0.000	0.144	0.144 c.m/sec"
"	Catchment 106	Pervious	Impervious	Total Area	"
"	Surface Area	0.491	0.600	1.090	hectare"
"	Time of concentration	48.108	4.973	7.458	minutes"
"	Time to Centroid	119.816	90.843	92.512	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	161.93	197.92	359.85	c.m"
"	Rainfall losses	30.669	1.629	14.697	mm"
"	Runoff depth	2.345	31.385	18.317	mm"
"	Runoff volume	11.50	188.15	199.65	c.m"
"	Runoff coefficient	0.071	0.951	0.555	"
"	Maximum flow	0.004	0.124	0.125	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.125	0.125	0.144	0.144"
" 33	CATCHMENT 102"				
"	1 Triangular SCS"				
"	1 Equal length"				

```

"          2 Horton equation"
"         102 Catchment 102"
"         1.000 % Impervious"
"         8.680 Total Area"
"        220.000 Flow length"
"         6.000 Overland Slope"
"         8.593 Pervious Area"
"        220.000 Pervious length"
"         6.000 Pervious slope"
"         0.087 Impervious Area"
"        220.000 Impervious length"
"         6.000 Impervious slope"
"         0.250 Pervious Manning 'n'"
"        75.000 Pervious Max.infiltration"
"         5.000 Pervious Min.infiltration"
"         0.250 Pervious Lag constant (hours)"
"         5.000 Pervious Depression storage"
"         0.015 Impervious Manning 'n'"
"         0.000 Impervious Max.infiltration"
"         0.000 Impervious Min.infiltration"
"         0.050 Impervious Lag constant (hours)"
"         1.500 Impervious Depression storage"
"                0.061      0.125      0.144      0.144 c.m/sec"
"        Catchment 102      Pervious      Impervious Total Area "
"        Surface Area      8.593      0.087      8.680      hectare"
"        Time of concentration 52.444      5.422      46.855      minutes"
"        Time to Centroid 123.693      91.446      119.860      minutes"
"        Rainfall depth 33.014      33.014      33.014      mm"
"        Rainfall volume 2836.96      28.66      2865.61      c.m"
"        Rainfall losses 30.669      1.692      30.379      mm"
"        Runoff depth 2.345      31.322      2.635      mm"
"        Runoff volume 201.54      27.19      228.73      c.m"
"        Runoff coefficient 0.071      0.949      0.080      "
"        Maximum flow 0.059      0.018      0.061      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"                0.061      0.147      0.144      0.144"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"                0.061      0.147      0.147      0.144"
" 40 HYDROGRAPH Combine 2000"
"      6 Combine "
"      2000 Node #"
"            Wetland"
"            Maximum flow      0.147      c.m/sec"
"            Hydrograph volume 428.383      c.m"
"                0.061      0.147      0.147      0.147"
" 40 HYDROGRAPH Confluence 2000"
"      7 Confluence "
"      2000 Node #"

```

"	Wetland"				
"	Maximum flow		0.147		c.m/sec"
"	Hydrograph volume		428.383		c.m"
"	0.061	0.147	0.147		0.000"
" 33	CATCHMENT 107"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	107 Catchment 107"				
"	55.000 % Impervious"				
"	3.550 Total Area"				
"	140.000 Flow length"				
"	2.000 Overland Slope"				
"	1.597 Pervious Area"				
"	140.000 Pervious length"				
"	2.000 Pervious slope"				
"	1.952 Impervious Area"				
"	140.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.397	0.147	0.147		0.000 c.m/sec"
"	Catchment 107	Pervious	Impervious	Total Area	"
"	Surface Area	1.597	1.952	3.550	hectare"
"	Time of concentration	55.598	5.748	8.629	minutes"
"	Time to Centroid	126.518	91.941	93.939	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	527.40	644.60	1172.00	c.m"
"	Rainfall losses	30.669	1.730	14.753	mm"
"	Runoff depth	2.345	31.284	18.261	mm"
"	Runoff volume	37.47	610.81	648.28	c.m"
"	Runoff coefficient	0.071	0.948	0.553	"
"	Maximum flow	0.011	0.397	0.397	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.397	0.544	0.147		0.000"
" 33	CATCHMENT 103"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	103 Catchment 103"				
"	0.000 % Impervious"				

```

"      6.330  Total Area"
"    160.000  Flow length"
"      9.000  Overland Slope"
"      6.330  Pervious Area"
"    160.000  Pervious length"
"      9.000  Pervious slope"
"      0.000  Impervious Area"
"    160.000  Impervious length"
"      9.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.059    0.544    0.147    0.000 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area  "
"      Surface Area      6.330      0.000      6.330      hectare"
"      Time of concentration  38.361      3.966      38.360      minutes"
"      Time to Centroid      111.070      89.459      111.070      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      2089.78      0.00      2089.78      c.m"
"      Rainfall losses      30.671      1.977      30.671      mm"
"      Runoff depth      2.343      31.037      2.343      mm"
"      Runoff volume      148.30      0.00      148.30      c.m"
"      Runoff coefficient      0.071      0.000      0.071      "
"      Maximum flow      0.059      0.000      0.059      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.059    0.550    0.147    0.000"
" 33      CATCHMENT 104"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      104      Scott Street External"
"    55.000  % Impervious"
"      2.600  Total Area"
"    95.000  Flow length"
"      2.000  Overland Slope"
"      1.170  Pervious Area"
"    95.000  Pervious length"
"      2.000  Pervious slope"
"      1.430  Impervious Area"
"    95.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

```

```

"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.300    0.550    0.147    0.000 c.m/sec"
"      Catchment 104      Pervious  Impervious Total Area  "
"      Surface Area      1.170    1.430    2.600    hectare"
"      Time of concentration 44.057    4.555    6.834    minutes"
"      Time to Centroid    116.160  90.239    91.735    minutes"
"      Rainfall depth      33.014    33.014    33.014    mm"
"      Rainfall volume      386.26    472.10    858.36    c.m"
"      Rainfall losses      30.671    1.703    14.738    mm"
"      Runoff depth         2.343    31.311    18.276    mm"
"      Runoff volume        27.41    447.75    475.16    c.m"
"      Runoff coefficient    0.071    0.948    0.554    "
"      Maximum flow         0.009    0.299    0.300    c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4  Add Runoff  "
"          0.300    0.850    0.147    0.000"
" 64      SHOW TABLE"
"      2  Flow hydrograph"
"      4  Inflow Hydrograph"
"      Maximum flow          0.850    c.m/sec"
"      Hydrograph volume      1700.128  c.m"
" 40      HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.300    0.850    0.850    0.000"
" 40      HYDROGRAPH Combine 1000"
"      6  Combine  "
"      1000 Node #"
"      Grand River"
"      Maximum flow          0.935    c.m/sec"
"      Hydrograph volume      2135.870  c.m"
"          0.300    0.850    0.850    0.935"
" 40      HYDROGRAPH Confluence 1000"
"      7  Confluence  "
"      1000 Node #"
"      Grand River"
"      Maximum flow          0.935    c.m/sec"
"      Hydrograph volume      2135.870  c.m"
"          0.300    0.935    0.850    0.000"
" 38      START/RE-START TOTALS 1000"
"      3  Runoff Totals on EXIT"
"      Total Catchment area      36.330    hectare"
"      Total Impervious area      4.437    hectare"

```

"
" 19

Total % impervious
EXIT"

12.214"

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 existing 005 year.out"
"          Licensee name:                      gmbp"
"          Company                             "
"          Date & Time last used:              2/21/2023 at 1:50:18 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          180.000  Max. Storm length"
"          3600.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          1459.072  Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    113.586  mm/hr"
"          Total depth                          49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 105"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          105  Catchment 105"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.104  0.000  0.000  0.000 c.m/sec"

```


	Catchment 105	Pervious	Impervious	Total Area	
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	14.420	2.108	4.886	minutes"
"	Time to Centroid	100.204	85.724	88.991	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	150.12	183.48	333.60	c.m"
"	Rainfall losses	32.852	2.224	16.007	mm"
"	Runoff depth	16.939	47.568	33.785	mm"
"	Runoff volume	51.07	175.29	226.36	c.m"
"	Runoff coefficient	0.340	0.955	0.679	"
"	Maximum flow	0.028	0.095	0.104	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

	4	Add Runoff "			
"	0.104	0.104	0.000	0.000"	

" 33 CATCHMENT 100"

	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	100	Catchment 100"			
"	0.000	% Impervious"			
"	4.920	Total Area"			
"	175.000	Flow length"			
"	8.000	Overland Slope"			
"	4.920	Pervious Area"			
"	175.000	Pervious length"			
"	8.000	Pervious slope"			
"	0.000	Impervious Area"			
"	175.000	Impervious length"			
"	8.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"	0.294	0.104	0.000	0.000 c.m/sec"	

	Catchment 100	Pervious	Impervious	Total Area	
"	Surface Area	4.920	0.000	4.920	hectare"
"	Time of concentration	27.409	4.007	27.409	minutes"
"	Time to Centroid	115.162	88.415	115.162	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	2449.75	0.00	2449.75	c.m"
"	Rainfall losses	32.839	2.133	32.839	mm"
"	Runoff depth	16.953	47.659	16.953	mm"
"	Runoff volume	834.06	0.00	834.07	c.m"
"	Runoff coefficient	0.340	0.000	0.340	"

"		Maximum flow	0.294	0.000	0.294	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.294	0.334	0.000	0.000"	
" 33		CATCHMENT 101"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	101	Catchment 101"				
"	0.000	% Impervious"				
"	8.490	Total Area"				
"	130.000	Flow length"				
"	10.000	Overland Slope"				
"	8.490	Pervious Area"				
"	130.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	130.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.610	0.334	0.000	0.000	c.m/sec"
"		Catchment 101	Pervious	Impervious	Total Area	"
"		Surface Area	8.490	0.000	8.490	hectare"
"		Time of concentration	21.447	3.135	21.447	minutes"
"		Time to Centroid	108.288	87.231	108.288	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	4227.31	0.00	4227.31	c.m"
"		Rainfall losses	32.850	2.512	32.850	mm"
"		Runoff depth	16.942	47.279	16.942	mm"
"		Runoff volume	1438.37	0.00	1438.38	c.m"
"		Runoff coefficient	0.340	0.000	0.340	"
"		Maximum flow	0.610	0.000	0.610	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.610	0.943	0.000	0.000"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.610	0.943	0.943	0.000"	
" 40		HYDROGRAPH Combine 1000"				
"	6	Combine "				
"	1000	Node #"				

"		Grand River"				
"		Maximum flow	0.943		c.m/sec"	
"		Hydrograph volume	2498.804		c.m"	
"		0.610	0.943	0.943	0.943"	
" 40		HYDROGRAPH Start - New Tributary"				
"	2	Start - New Tributary"				
"		0.610	0.000	0.943	0.943"	
" 33		CATCHMENT 106"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	106	Catchment 106"				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.169	0.000	0.943	0.943 c.m/sec"	
"		Catchment 106	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	31.443	4.597	10.620	minutes"
"		Time to Centroid	119.805	89.191	96.060	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	244.23	298.50	542.73	c.m"
"		Rainfall losses	32.853	1.876	15.816	mm"
"		Runoff depth	16.939	47.916	33.976	mm"
"		Runoff volume	83.09	287.25	370.34	c.m"
"		Runoff coefficient	0.340	0.962	0.682	"
"		Maximum flow	0.027	0.165	0.169	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.169	0.169	0.943	0.943"	
" 33		CATCHMENT 102"				
"	1	Triangular SCS"				
"	1	Equal length"				

```

"          2 Horton equation"
"          102 Catchment 102"
"          1.000 % Impervious"
"          8.680 Total Area"
"         220.000 Flow length"
"          6.000 Overland Slope"
"          8.593 Pervious Area"
"         220.000 Pervious length"
"          6.000 Pervious slope"
"          0.087 Impervious Area"
"         220.000 Impervious length"
"          6.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"                0.439      0.169      0.943      0.943 c.m/sec"
"          Catchment 102      Pervious      Impervious Total Area "
"          Surface Area      8.593      0.087      8.680      hectare"
"          Time of concentration 34.277      5.011      33.463      minutes"
"          Time to Centroid 123.076      89.738      122.149      minutes"
"          Rainfall depth 49.792      49.792      49.792      mm"
"          Rainfall volume 4278.70      43.22      4321.92      c.m"
"          Rainfall losses 32.841      1.787      32.531      mm"
"          Runoff depth 16.950      48.005      17.261      mm"
"          Runoff volume 1456.58      41.67      1498.25      c.m"
"          Runoff coefficient 0.340      0.964      0.347      "
"          Maximum flow 0.434      0.024      0.439      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"                0.439      0.497      0.943      0.943"
" 40 HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"                0.439      0.497      0.497      0.943"
" 40 HYDROGRAPH Combine 2000"
"          6 Combine "
"         2000 Node #"
"          Wetland"
"          Maximum flow      0.497      c.m/sec"
"          Hydrograph volume 1868.590      c.m"
"                0.439      0.497      0.497      0.497"
" 40 HYDROGRAPH Confluence 2000"
"          7 Confluence "
"         2000 Node #"

```

"	Wetland"				
"	Maximum flow		0.497		c.m/sec"
"	Hydrograph volume		1868.590		c.m"
"		0.439	0.497	0.497	0.000"
" 33	CATCHMENT 107"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	107 Catchment 107"				
"	55.000 % Impervious"				
"	3.550 Total Area"				
"	140.000 Flow length"				
"	2.000 Overland Slope"				
"	1.597 Pervious Area"				
"	140.000 Pervious length"				
"	2.000 Pervious slope"				
"	1.952 Impervious Area"				
"	140.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.544	0.497	0.497	0.000 c.m/sec"
"	Catchment 107		Pervious	Impervious	Total Area "
"	Surface Area		1.597	1.952	3.550 hectare"
"	Time of concentration		36.338	5.313	12.264 minutes"
"	Time to Centroid		125.451	90.132	98.045 minutes"
"	Rainfall depth		49.792	49.792	49.792 mm"
"	Rainfall volume		795.42	972.18	1767.60 c.m"
"	Rainfall losses		32.839	1.757	15.744 mm"
"	Runoff depth		16.953	48.035	34.048 mm"
"	Runoff volume		270.82	937.88	1208.71 c.m"
"	Runoff coefficient		0.340	0.965	0.684 "
"	Maximum flow		0.078	0.534	0.544 c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.544	0.796	0.497	0.000"
" 33	CATCHMENT 103"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	103 Catchment 103"				
"	0.000 % Impervious"				

```

"      6.330  Total Area"
"    160.000  Flow length"
"      9.000  Overland Slope"
"      6.330  Pervious Area"
"    160.000  Pervious length"
"      9.000  Pervious slope"
"      0.000  Impervious Area"
"    160.000  Impervious length"
"      9.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.404    0.796    0.497    0.000 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area  "
"      Surface Area      6.330      0.000      6.330      hectare"
"      Time of concentration  25.072    3.666    25.072    minutes"
"      Time to Centroid      112.469  87.991    112.469  minutes"
"      Rainfall depth      49.792    49.792    49.792    mm"
"      Rainfall volume      3151.81   0.00     3151.81   c.m"
"      Rainfall losses      32.853    2.512    32.853    mm"
"      Runoff depth         16.939    47.280    16.939    mm"
"      Runoff volume        1072.25   0.00     1072.25   c.m"
"      Runoff coefficient    0.340     0.000    0.340     "
"      Maximum flow         0.404     0.000    0.404     c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.404    1.092    0.497    0.000"
" 33  CATCHMENT 104"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      104  Scott Street External"
"    55.000  % Impervious"
"      2.600  Total Area"
"    95.000  Flow length"
"      2.000  Overland Slope"
"      1.170  Pervious Area"
"    95.000  Pervious length"
"      2.000  Pervious slope"
"      1.430  Impervious Area"
"    95.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

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"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.405      1.092      0.497      0.000 c.m/sec"
"      Catchment 104      Pervious      Impervious Total Area "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration 28.795      4.210      9.735      minutes"
"      Time to Centroid 116.746      88.669      94.978      minutes"
"      Rainfall depth 49.792      49.792      49.792      mm"
"      Rainfall volume 582.56      712.02      1294.58      c.m"
"      Rainfall losses 32.859      1.996      15.884      mm"
"      Runoff depth 16.933      47.796      33.908      mm"
"      Runoff volume 198.12      683.48      881.60      c.m"
"      Runoff coefficient 0.340      0.960      0.681      "
"      Maximum flow 0.068      0.395      0.405      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.405      1.276      0.497      0.000"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow      1.276      c.m/sec"
"      Hydrograph volume 5031.145      c.m"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.405      1.276      1.276      0.000"
" 40      HYDROGRAPH Combine 1000"
"      6      Combine "
"      1000      Node #"
"      Grand River"
"      Maximum flow      2.172      c.m/sec"
"      Hydrograph volume 7529.947      c.m"
"          0.405      1.276      1.276      2.172"
" 40      HYDROGRAPH Confluence 1000"
"      7      Confluence "
"      1000      Node #"
"      Grand River"
"      Maximum flow      2.172      c.m/sec"
"      Hydrograph volume 7529.947      c.m"
"          0.405      2.172      1.276      0.000"
" 38      START/RE-START TOTALS 1000"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      36.330      hectare"
"      Total Impervious area      4.437      hectare"

```

"
" 19

Total % impervious
EXIT"

12.214"


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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 existing 100 year.out"
"          Licensee name:                      gmbp"
"          Company                             "
"          Date & Time last used:              2/21/2023 at 1:50:53 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.020 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    168.777  mm/hr"
"          Total depth                          97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 105"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          105  Catchment 105"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.233  0.000  0.000  0.000 c.m/sec"

```

"	Catchment 105	Pervious	Impervious	Total Area	"
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	10.076	1.799	4.711	minutes"
"	Time to Centroid	97.792	84.352	89.081	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	295.23	360.84	656.07	c.m"
"	Rainfall losses	34.803	2.791	17.197	mm"
"	Runoff depth	63.119	95.130	80.725	mm"
"	Runoff volume	190.30	350.55	540.86	c.m"
"	Runoff coefficient	0.645	0.971	0.824	"
"	Maximum flow	0.099	0.153	0.233	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.233	0.233	0.000	0.000"	
" 33	CATCHMENT 100"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	100 Catchment 100"				
"	0.000 % Impervious"				
"	4.920 Total Area"				
"	175.000 Flow length"				
"	8.000 Overland Slope"				
"	4.920 Pervious Area"				
"	175.000 Pervious length"				
"	8.000 Pervious slope"				
"	0.000 Impervious Area"				
"	175.000 Impervious length"				
"	8.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	1.192	0.233	0.000	0.000	c.m/sec"
"	Catchment 100	Pervious	Impervious	Total Area	"
"	Surface Area	4.920	0.000	4.920	hectare"
"	Time of concentration	19.152	3.420	19.152	minutes"
"	Time to Centroid	108.129	86.591	108.129	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	4817.72	0.00	4817.73	c.m"
"	Rainfall losses	34.540	3.750	34.540	mm"
"	Runoff depth	63.381	94.172	63.381	mm"
"	Runoff volume	3118.34	0.00	3118.35	c.m"
"	Runoff coefficient	0.647	0.000	0.647	"

"		Maximum flow	1.192	0.000	1.192	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		1.192	1.364	0.000	0.000"	
" 33		CATCHMENT 101"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	101	Catchment 101"				
"	0.000	% Impervious"				
"	8.490	Total Area"				
"	130.000	Flow length"				
"	10.000	Overland Slope"				
"	8.490	Pervious Area"				
"	130.000	Pervious length"				
"	10.000	Pervious slope"				
"	0.000	Impervious Area"				
"	130.000	Impervious length"				
"	10.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		2.442	1.364	0.000	0.000	c.m/sec"
"		Catchment 101	Pervious	Impervious	Total Area	"
"		Surface Area	8.490	0.000	8.490	hectare"
"		Time of concentration	14.986	2.676	14.986	minutes"
"		Time to Centroid	103.392	85.567	103.392	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	8313.51	0.01	8313.52	c.m"
"		Rainfall losses	34.574	3.577	34.574	mm"
"		Runoff depth	63.347	94.344	63.347	mm"
"		Runoff volume	5378.16	0.01	5378.17	c.m"
"		Runoff coefficient	0.647	0.000	0.647	"
"		Maximum flow	2.442	0.000	2.442	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		2.442	3.805	0.000	0.000"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		2.442	3.805	3.805	0.000"	
" 40		HYDROGRAPH Combine 1000"				
"	6	Combine "				
"	1000	Node #"				

"	Grand River"				
"	Maximum flow		3.805	c.m/sec"	
"	Hydrograph volume		9037.379	c.m"	
"		2.442	3.805	3.805	3.805"
" 40	HYDROGRAPH Start - New Tributary"				
"	2 Start - New Tributary"				
"		2.442	0.000	3.805	3.805"
" 33	CATCHMENT 106"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	106 Catchment 106"				
"	55.000 % Impervious"				
"	1.090 Total Area"				
"	110.000 Flow length"				
"	2.000 Overland Slope"				
"	0.491 Pervious Area"				
"	110.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.600 Impervious Area"				
"	110.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.301	0.000	3.805	3.805 c.m/sec"
"	Catchment 106	Pervious	Impervious	Total Area	"
"	Surface Area	0.491	0.600	1.090	hectare"
"	Time of concentration	21.971	3.923	10.297	minutes"
"	Time to Centroid	111.333	87.214	95.731	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	480.30	587.04	1067.34	c.m"
"	Rainfall losses	34.507	2.882	17.113	mm"
"	Runoff depth	63.414	95.039	80.808	mm"
"	Runoff volume	311.04	569.76	880.81	c.m"
"	Runoff coefficient	0.648	0.971	0.825	"
"	Maximum flow	0.114	0.260	0.301	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.301	0.301	3.805	3.805"
" 33	CATCHMENT 102"				
"	1 Triangular SCS"				
"	1 Equal length"				

```

"          2 Horton equation"
"         102 Catchment 102"
"         1.000 % Impervious"
"         8.680 Total Area"
"        220.000 Flow length"
"         6.000 Overland Slope"
"         8.593 Pervious Area"
"        220.000 Pervious length"
"         6.000 Pervious slope"
"         0.087 Impervious Area"
"        220.000 Impervious length"
"         6.000 Impervious slope"
"         0.250 Pervious Manning 'n'"
"        75.000 Pervious Max.infiltration"
"         5.000 Pervious Min.infiltration"
"         0.250 Pervious Lag constant (hours)"
"         5.000 Pervious Depression storage"
"         0.015 Impervious Manning 'n'"
"         0.000 Impervious Max.infiltration"
"         0.000 Impervious Min.infiltration"
"         0.050 Impervious Lag constant (hours)"
"         1.500 Impervious Depression storage"
"           1.891      0.301      3.805      3.805 c.m/sec"
"         Catchment 102      Pervious      Impervious Total Area "
"         Surface Area      8.593      0.087      8.680      hectare"
"         Time of concentration 23.951      4.277      23.656      minutes"
"         Time to Centroid 113.589      87.628      113.200      minutes"
"         Rainfall depth 97.921      97.921      97.921      mm"
"         Rainfall volume 8414.58      85.00      8499.57      c.m"
"         Rainfall losses 34.508      2.400      34.187      mm"
"         Runoff depth 63.413      95.522      63.734      mm"
"         Runoff volume 5449.21      82.91      5532.12      c.m"
"         Runoff coefficient 0.648      0.975      0.651      "
"         Maximum flow 1.874      0.038      1.891      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"     4 Add Runoff "
"           1.891      2.116      3.805      3.805"
" 40 HYDROGRAPH Copy to Outflow"
"     8 Copy to Outflow"
"           1.891      2.116      2.116      3.805"
" 40 HYDROGRAPH Combine 2000"
"     6 Combine "
"    2000 Node #"
"         Wetland"
"         Maximum flow      2.116      c.m/sec"
"         Hydrograph volume 6412.921      c.m"
"           1.891      2.116      2.116      2.116"
" 40 HYDROGRAPH Confluence 2000"
"     7 Confluence "
"    2000 Node #"

```

"	Wetland"				
"	Maximum flow		2.116	c.m/sec"	
"	Hydrograph volume		6412.922	c.m"	
"	1.891	2.116	2.116	0.000"	
" 33	CATCHMENT 107"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	107 Catchment 107"				
"	55.000 % Impervious"				
"	3.550 Total Area"				
"	140.000 Flow length"				
"	2.000 Overland Slope"				
"	1.597 Pervious Area"				
"	140.000 Pervious length"				
"	2.000 Pervious slope"				
"	1.952 Impervious Area"				
"	140.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.948	2.116	2.116	0.000 c.m/sec"	
"	Catchment 107	Pervious	Impervious	Total Area	"
"	Surface Area	1.597	1.952	3.550	hectare"
"	Time of concentration	25.391	4.534	11.868	minutes"
"	Time to Centroid	115.220	87.952	97.540	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	1564.29	1911.91	3476.21	c.m"
"	Rainfall losses	34.527	2.269	16.785	mm"
"	Runoff depth	63.395	95.653	81.137	mm"
"	Runoff volume	1012.73	1867.62	2880.35	c.m"
"	Runoff coefficient	0.647	0.977	0.829	"
"	Maximum flow	0.333	0.843	0.948	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.948	2.824	2.116	0.000"	
" 33	CATCHMENT 103"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	103 Catchment 103"				
"	0.000 % Impervious"				

```

"      6.330  Total Area"
"    160.000  Flow length"
"      9.000  Overland Slope"
"      6.330  Pervious Area"
"    160.000  Pervious length"
"      9.000  Pervious slope"
"      0.000  Impervious Area"
"    160.000  Impervious length"
"      9.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          1.633    2.824    2.116    0.000 c.m/sec"
"      Catchment 103      Pervious      Impervious      Total Area  "
"      Surface Area      6.330      0.000      6.330      hectare"
"      Time of concentration  17.519    3.129    17.519    minutes"
"      Time to Centroid      106.265  86.212    106.265    minutes"
"      Rainfall depth      97.921    97.921    97.921    mm"
"      Rainfall volume      6198.41   0.01      6198.42    c.m"
"      Rainfall losses      34.499    3.700     34.499    mm"
"      Runoff depth         63.423    94.221    63.423    mm"
"      Runoff volume        4014.64   0.01      4014.65    c.m"
"      Runoff coefficient    0.648     0.000     0.648     "
"      Maximum flow         1.633     0.000     1.633     c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          1.633    4.411    2.116    0.000"
" 33  CATCHMENT 104"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      104  Scott Street External"
"    55.000  % Impervious"
"      2.600  Total Area"
"    95.000  Flow length"
"      2.000  Overland Slope"
"      1.170  Pervious Area"
"    95.000  Pervious length"
"      2.000  Pervious slope"
"      1.430  Impervious Area"
"    95.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

```

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"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.730      4.411      2.116      0.000 c.m/sec"
"      Catchment 104      Pervious      Impervious Total Area "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration 20.121      3.593      9.455      minutes"
"      Time to Centroid 109.215      86.819      94.762      minutes"
"      Rainfall depth 97.921      97.921      97.921      mm"
"      Rainfall volume 1145.68      1400.27      2545.95      c.m"
"      Rainfall losses 34.520      3.529      17.475      mm"
"      Runoff depth 63.401      94.392      80.446      mm"
"      Runoff volume 741.80      1349.81      2091.61      c.m"
"      Runoff coefficient 0.647      0.964      0.822      "
"      Maximum flow 0.277      0.616      0.730      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.730      4.999      2.116      0.000"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"          Maximum flow      4.999      c.m/sec"
"          Hydrograph volume      15399.529      c.m"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.730      4.999      4.999      0.000"
" 40      HYDROGRAPH Combine 1000"
"      6      Combine "
"      1000      Node #"
"          Grand River"
"          Maximum flow      8.805      c.m/sec"
"          Hydrograph volume      24436.910      c.m"
"          0.730      4.999      4.999      8.805"
" 40      HYDROGRAPH Confluence 1000"
"      7      Confluence "
"      1000      Node #"
"          Grand River"
"          Maximum flow      8.805      c.m/sec"
"          Hydrograph volume      24436.910      c.m"
"          0.730      8.805      4.999      0.000"
" 38      START/RE-START TOTALS 1000"
"      3      Runoff Totals on EXIT"
"          Total Catchment area      36.330      hectare"
"          Total Impervious area      4.437      hectare"

```


"
" 19

Total % impervious
EXIT"

12.214"

```

"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 existing Reg.out"
"          Licensee name:                      gmbp"
"          Company                             "
"          Date & Time last used:              2/21/2023 at 1:51:38 PM"
" 31          TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          3600.000  Max. Hydrograph"
" 32          STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.028    2.028    2.028    2.028"
"                   2.028    2.026    2.026    2.026    2.028"
"                   2.026    6.000    4.000    6.000    13.000"
"                   17.000    13.000    23.000    13.000    13.000"
"                   53.000    38.000    13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
"          7  9999hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 105"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          105  Catchment 105"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"

```

```

"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.078      0.000      0.000      0.000 c.m/sec"
"      Catchment 105      Pervious      Impervious      Total Area  "
"      Surface Area      0.302      0.368      0.670      hectare"
"      Time of concentration 16.093      2.860      7.181      minutes"
"      Time to Centroid      2720.207      2251.465      2404.533      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      859.28      1050.22      1909.50      c.m"
"      Rainfall losses      139.563      39.596      84.581      mm"
"      Runoff depth      145.437      245.404      200.419      mm"
"      Runoff volume      438.49      904.31      1342.81      c.m"
"      Runoff coefficient      0.510      0.861      0.703      "
"      Maximum flow      0.031      0.047      0.078      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.078      0.078      0.000      0.000"
" 33      CATCHMENT 100"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      100      Catchment 100"
"      0.000      % Impervious"
"      4.920      Total Area"
"      175.000      Flow length"
"      8.000      Overland Slope"
"      4.920      Pervious Area"
"      175.000      Pervious length"
"      8.000      Pervious slope"
"      0.000      Impervious Area"
"      175.000      Impervious length"
"      8.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"
"      0.015      Impervious Manning 'n'"
"      0.000      Impervious Max.infiltration"
"      0.000      Impervious Min.infiltration"
"      0.050      Impervious Lag constant (hours)"
"      1.500      Impervious Depression storage"
"              0.538      0.078      0.000      0.000 c.m/sec"
"      Catchment 100      Pervious      Impervious      Total Area  "
"      Surface Area      4.920      0.000      4.920      hectare"
"      Time of concentration 30.590      5.436      30.590      minutes"
"      Time to Centroid      2736.207      2231.294      2736.206      minutes"

```

"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	1.4022	0.0000	1.4022	ha-m"
"	Rainfall losses	139.351	31.050	139.351	mm"
"	Runoff depth	145.649	253.950	145.649	mm"
"	Runoff volume	7165.91	0.01	7165.92	c.m"
"	Runoff coefficient	0.511	0.000	0.511	"
"	Maximum flow	0.538	0.000	0.538	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.538	0.601	0.000	0.000"	
" 33	CATCHMENT 101"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	101 Catchment 101"				
"	0.000 % Impervious"				
"	8.490 Total Area"				
"	130.000 Flow length"				
"	10.000 Overland Slope"				
"	8.490 Pervious Area"				
"	130.000 Pervious length"				
"	10.000 Pervious slope"				
"	0.000 Impervious Area"				
"	130.000 Impervious length"				
"	10.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.896	0.601	0.000	0.000 c.m/sec"	
"	Catchment 101	Pervious	Impervious	Total Area	"
"	Surface Area	8.490	0.000	8.490	hectare"
"	Time of concentration	23.936	4.253	23.936	minutes"
"	Time to Centroid	2730.043	2232.872	2730.043	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	2.4196	0.0000	2.4196	ha-m"
"	Rainfall losses	139.107	35.924	139.107	mm"
"	Runoff depth	145.893	249.076	145.893	mm"
"	Runoff volume	1.2386	0.0000	1.2386	ha-m"
"	Runoff coefficient	0.512	0.000	0.512	"
"	Maximum flow	0.896	0.000	0.896	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.896	1.497	0.000	0.000"	

```

" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.896      1.497      1.497      0.000"
" 40      HYDROGRAPH  Combine      1000"
"          6  Combine "
"      1000  Node #"
"              Grand River"
"              Maximum flow              1.497      c.m/sec"
"              Hydrograph volume          20895.053      c.m"
"              0.896      1.497      1.497      1.497"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.896      0.000      1.497      1.497"
" 33      CATCHMENT 106"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"      106  Catchment 106"
"      55.000  % Impervious"
"      1.090  Total Area"
"      110.000  Flow length"
"      2.000  Overland Slope"
"      0.491  Pervious Area"
"      110.000  Pervious length"
"      2.000  Pervious slope"
"      0.600  Impervious Area"
"      110.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.117      0.000      1.497      1.497 c.m/sec"
"          Catchment 106      Pervious      Impervious Total Area "
"          Surface Area      0.491      0.600      1.090      hectare"
"          Time of concentration  35.092      6.236      15.325      minutes"
"          Time to Centroid      2739.025      2235.505      2394.105      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      1397.93      1708.57      3106.50      c.m"
"          Rainfall losses      140.362      27.637      78.363      mm"
"          Runoff depth      144.638      257.363      206.637      mm"
"          Runoff volume      709.45      1542.89      2252.34      c.m"
"          Runoff coefficient      0.508      0.903      0.725      "
"          Maximum flow      0.054      0.076      0.117      c.m/sec"

```

```

" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.117      0.117      1.497      1.497"
" 33      CATCHMENT 102"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          102 Catchment 102"
"          1.000 % Impervious"
"          8.680 Total Area"
"          220.000 Flow length"
"          6.000 Overland Slope"
"          8.593 Pervious Area"
"          220.000 Pervious length"
"          6.000 Pervious slope"
"          0.087 Impervious Area"
"          220.000 Impervious length"
"          6.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.950      0.117      1.497      1.497 c.m/sec"
"          Catchment 102      Pervious      Impervious Total Area "
"          Surface Area      8.593      0.087      8.680      hectare"
"          Time of concentration 38.255      6.798      37.689      minutes"
"          Time to Centroid 2741.159      2241.450      2732.164      minutes"
"          Rainfall depth 285.000      285.000      285.000      mm"
"          Rainfall volume 2.4491      0.0247      2.4738      ha-m"
"          Rainfall losses 141.565      24.677      140.396      mm"
"          Runoff depth 143.435      260.323      144.604      mm"
"          Runoff volume 1.2326      0.0226      1.2552      ha-m"
"          Runoff coefficient 0.503      0.913      0.507      "
"          Maximum flow 0.943      0.011      0.950      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.950      1.056      1.497      1.497"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.950      1.056      1.056      1.497"
" 40      HYDROGRAPH Combine 2000"
"          6  Combine "
"          2000 Node #"
"          Wetland"

```

"		Maximum flow	1.056	c.m/sec"
"		Hydrograph volume	14803.967	c.m"
"		0.950 1.056 1.056	1.056	1.056"
" 40		HYDROGRAPH Confluence	2000"	
"	7	Confluence "		
"	2000	Node #"		
"		Wetland"		
"		Maximum flow	1.056	c.m/sec"
"		Hydrograph volume	14803.966	c.m"
"		0.950 1.056 1.056	1.056	0.000"
" 33		CATCHMENT 107"		
"	1	Triangular SCS"		
"	1	Equal length"		
"	2	Horton equation"		
"	107	Catchment 107"		
"	55.000	% Impervious"		
"	3.550	Total Area"		
"	140.000	Flow length"		
"	2.000	Overland Slope"		
"	1.597	Pervious Area"		
"	140.000	Pervious length"		
"	2.000	Pervious slope"		
"	1.952	Impervious Area"		
"	140.000	Impervious length"		
"	2.000	Impervious slope"		
"	0.250	Pervious Manning 'n'"		
"	75.000	Pervious Max.infiltration"		
"	5.000	Pervious Min.infiltration"		
"	0.250	Pervious Lag constant (hours)"		
"	5.000	Pervious Depression storage"		
"	0.015	Impervious Manning 'n'"		
"	0.000	Impervious Max.infiltration"		
"	0.000	Impervious Min.infiltration"		
"	0.050	Impervious Lag constant (hours)"		
"	1.500	Impervious Depression storage"		
"		0.371 1.056 1.056	0.000	c.m/sec"
"		Catchment 107	Pervious	Impervious Total Area "
"		Surface Area	1.597	1.952 3.550 hectare"
"		Time of concentration	40.556	7.207 17.519 minutes"
"		Time to Centroid	2744.076	2245.793 2399.878 minutes"
"		Rainfall depth	285.000	285.000 285.000 mm"
"		Rainfall volume	0.4553	0.5565 1.0117 ha-m"
"		Rainfall losses	141.525	22.773 76.211 mm"
"		Runoff depth	143.475	262.227 208.789 mm"
"		Runoff volume	2292.02	5119.98 7412.00 c.m"
"		Runoff coefficient	0.503	0.920 0.733 "
"		Maximum flow	0.175	0.247 0.371 c.m/sec"
" 40		HYDROGRAPH Add Runoff "		
"	4	Add Runoff "		
"		0.371 1.405 1.056	0.000"	

```

" 33      CATCHMENT 103"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          103 Catchment 103"
"          0.000 % Impervious"
"          6.330 Total Area"
"         160.000 Flow length"
"          9.000 Overland Slope"
"          6.330 Pervious Area"
"         160.000 Pervious length"
"          9.000 Pervious slope"
"          0.000 Impervious Area"
"         160.000 Impervious length"
"          9.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"                0.686      1.405      1.056      0.000 c.m/sec"
"          Catchment 103      Pervious      Impervious Total Area "
"          Surface Area      6.330      0.000      6.330      hectare"
"          Time of concentration 27.982      4.972      27.982      minutes"
"          Time to Centroid 2734.145      2231.264      2734.144      minutes"
"          Rainfall depth 285.000      285.000      285.000      mm"
"          Rainfall volume 1.8040      0.0000      1.8041      ha-m"
"          Rainfall losses 138.998      32.769      138.998      mm"
"          Runoff depth 146.002      252.231      146.002      mm"
"          Runoff volume 9241.89      0.02      9241.91      c.m"
"          Runoff coefficient 0.512      0.000      0.512      "
"          Maximum flow 0.686      0.000      0.686      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"                0.686      2.091      1.056      0.000"
" 33      CATCHMENT 104"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          104 Scott Street External"
"         55.000 % Impervious"
"          2.600 Total Area"
"         95.000 Flow length"
"          2.000 Overland Slope"
"          1.170 Pervious Area"

```



```

"      95.000 Pervious length"
"      2.000 Pervious slope"
"      1.430 Impervious Area"
"      95.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          0.282      2.091      1.056      0.000 c.m/sec"
"      Catchment 104      Pervious      Impervious Total Area "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration 32.137      5.711      14.115      minutes"
"      Time to Centroid 2737.280      2232.538      2393.050      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      3334.50      4075.50      7410.00      c.m"
"      Rainfall losses      139.558      29.801      79.192      mm"
"      Runoff depth      145.442      255.199      205.808      mm"
"      Runoff volume      1701.67      3649.34      5351.01      c.m"
"      Runoff coefficient      0.510      0.895      0.722      "
"      Maximum flow      0.128      0.181      0.282      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.282      2.343      1.056      0.000"
" 64      SHOW TABLE"
"      2      Flow hydrograph"
"      4      Inflow Hydrograph"
"      Maximum flow      2.343      c.m/sec"
"      Hydrograph volume      36808.887      c.m"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.282      2.343      2.343      0.000"
" 40      HYDROGRAPH Combine 1000"
"      6      Combine "
"      1000      Node #"
"      Grand River"
"      Maximum flow      3.839      c.m/sec"
"      Hydrograph volume      57703.930      c.m"
"          0.282      2.343      2.343      3.839"
" 40      HYDROGRAPH Confluence 1000"
"      7      Confluence "
"      1000      Node #"
"      Grand River"
"      Maximum flow      3.839      c.m/sec"

```

"	Hydrograph volume	57703.930	c.m"
"	0.282 3.839 2.343	0.000"	
" 38	START/RE-START TOTALS 1000"		
"	3 Runoff Totals on EXIT"		
"	Total Catchment area	36.330	hectare"
"	Total Impervious area	4.437	hectare"
"	Total % impervious	12.214"	
" 19	EXIT"		



APPENDIX D

Post-Development Conditions
Stormwater Management Analysis



**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Proposed Stormwater Management Pond**

**Active Storage Volume Calculations
Wetland SWM Facility**

Elevation (m)	Depth (m)	Pond Surface Area (m ²)	Perm. Pool (P.P) Volume (m ³)	Accum. P.P. Volume (m ³)	Pond Active Volume (m ³)	Accum. Active Volume (m ³)	
455.70	0.00	4,716.0	0.0	0.0			P.P. Bottom
455.80	0.10	5,931.0	532.4	532.4			
455.90	0.20	6,277.0	610.4	1,142.7			
456.00	0.00	6,626.0	645.2	1,787.9	0.0	0.0	P.P. Top/Bottom of Storage
456.10	0.10	6,979.0			680.3	680.3	
456.20	0.20	7,335.0			715.7	1,395.9	
456.30	0.30	7,696.0			751.6	2,147.5	
456.40	0.40	8,063.0			787.9	2,935.4	
456.48	0.48	8,361.0			657.0	3,592.4	
456.50	0.50	8,436.0			168.0	3,760.4	DICB Lip
456.60	0.60	8,904.0			867.0	4,627.4	
456.70	0.70	9,344.0			912.4	5,539.8	
456.80	0.80	9,798.0			957.1	6,496.9	
456.90	0.90	10,270.0			1,003.4	7,500.3	
457.00	1.00	10,756.0			1,051.3	8,551.6	Weir Control
457.10	1.10	11,253.0			1,100.5	9,652.0	
457.20	1.20	11,769.0			1,151.1	10,803.1	
457.30	1.30	12,298.0			1,203.4	12,006.5	
457.40	1.40	13,083.0			1,269.0	13,275.5	Top of Pond
457.50	1.50	15,629.0			1,435.6	14,711.1	Overflow

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Proposed Stormwater Management Pond**

Knockout			600 mm Diameter Orifice		
INV	456.00	m	INV	455.48	m
Q =	0.114	m ³ /s	Q =	0.986	m ³ /s
Cd =	0.600		Cd =	0.600	
H =	1.3925	m	H =	1.72	m
2g =	19.620		2g =	19.620	
A =	0.036	m ²	A =	0.283	m ²
D =	0.215	m	D =	0.600	m

3-600mm dia. pipes @ 0.50% will discharge flows from the stormwater management pond. Orifice flows will be multiplied by 3.

1200 mm x 1200 mm Ditch Inlet Outlet Structure Weir Flow OPSD 702.050

Elev m	d1 m	h m	H m	2g	L m	Qfront m³/s	Qback m³/s	Qsides m³/s	Qtotal m³/s	Q for 3 Structures m³/s
456.50	0.50	0.50	0.00	19.62	1.20	0.000	0.000	0.000	0.000	0.000
456.60	0.60	0.50	0.10	19.62	1.20	0.054	0.000	0.018	0.072	0.217
456.70	0.70	0.50	0.20	19.62	1.20	0.157	0.000	0.072	0.230	0.689
456.80	0.80	0.50	0.30	19.62	1.20	0.297	0.000	0.174	0.470	1.411
456.90	0.90	0.50	0.40	19.62	1.20	0.467	0.053	0.367	0.834	2.501
457.00	1.00	0.50	0.50	19.62	1.20	0.664	0.153	0.639	1.304	3.911

These are the flows over the DICB Lip - once flows are equal to or exceed orifice flow, the SSD table switches to orifice flow.

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Proposed Stormwater Management Pond**

Overflow Weir No. 1

Elev m	d1 m	h m	H m	2g	L m	Q m³/s
457.00	1.00	1.00	0.00	19.62	47.50	0.00
457.10	1.10	1.00	0.10	19.62	47.50	2.09
457.20	1.20	1.00	0.20	19.62	47.50	6.02
457.30	1.30	1.00	0.30	19.62	47.50	11.25
457.40	1.40	1.00	0.40	19.62	47.50	17.60
457.50	1.50	1.00	0.50	19.62	47.50	24.95

Overflow Weir No. 2

Elev m	d1 m	h m	H m	2g	L m	Q m³/s
457.00	1.00	1.00	0.00	19.62	19.00	0.00
457.10	1.10	1.00	0.10	19.62	19.00	0.83
457.20	1.20	1.00	0.20	19.62	19.00	2.41
457.30	1.30	1.00	0.30	19.62	19.00	4.50
457.40	1.40	1.00	0.40	19.62	19.00	7.04
457.50	1.50	1.00	0.50	19.62	19.00	9.98

Overflow Weir No. 3

Elev m	d1 m	h m	H m	2g	L m	Q m³/s
457.00	1.00	1.00	0.00	19.62	12.00	0.00
457.10	1.10	1.00	0.10	19.62	12.00	0.53
457.20	1.20	1.00	0.20	19.62	12.00	1.52
457.30	1.30	1.00	0.30	19.62	12.00	2.84
457.40	1.40	1.00	0.40	19.62	12.00	4.45
457.50	1.50	1.00	0.50	19.62	12.00	6.30

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Proposed Stormwater Management Pond**

Stage-Storage-Discharge Table											
Elevation	Stage	Storage	Knockout Control 210mm	1200 x 1200 Ditch Inlet 3-Total	600mm Orifice Control 3-Total	600mm Pipe Discharge 3-Total	Overflow Weir No. 1	Overflow Weir No. 2	Overflow Weir No. 3	Actual Discharge	
(m)	(m)	(m ³)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	(m ³ /s)	
456.00	0.00	0.0	0.000		0.000	0.000	0.000	0.000	0.000	0.000	Bottom of Storage
456.10	0.10	680.3	0.015		0.000	0.000	0.000	0.000	0.000	0.015	
456.20	0.20	1,395.9	0.029		0.000	0.000	0.000	0.000	0.000	0.029	
456.30	0.30	2,147.5	0.042		0.000	0.000	0.000	0.000	0.000	0.042	
456.40	0.40	2,935.4	0.052		0.000	0.000	0.000	0.000	0.000	0.052	
456.48	0.48	3,592.4	0.059		0.000	0.000	0.000	0.000	0.000	0.059	
456.50	0.50	3,760.4	0.060	0.000	0.000	0.000	0.000	0.000	0.000	0.060	DICB Lip
456.60	0.60	4,627.4	0.068	0.217	2.041	2.978	0.000	0.000	0.000	0.284	
456.70	0.70	5,539.8	0.074	0.689	2.162	3.201	0.000	0.000	0.000	0.763	
456.80	0.80	6,496.9	0.080	1.411	2.277	3.410	0.000	0.000	0.000	1.491	
456.90	0.90	7,500.3	0.086	2.501	2.386	3.607	0.000	0.000	0.000	2.386	
457.00	1.00	8,551.6	0.091	3.911	2.490	3.793	0.000	0.000	0.000	2.490	Weir
457.10	1.10	9,652.0	0.096		2.590	3.971	2.085	0.834	0.527	6.036	
457.20	1.20	10,803.1	0.101		2.686	4.141	6.018	2.407	1.520	12.632	
457.30	1.30	12,006.5	0.105		2.779	4.304	11.254	4.502	2.843	21.379	
457.40	1.40	13,275.5	0.110		2.869	4.461	17.603	7.041	4.447	31.960	Top of Pond
457.50	1.50	14,711.1	0.114		2.957	4.613	24.949	9.980	6.303	44.189	Overflow

Notes:

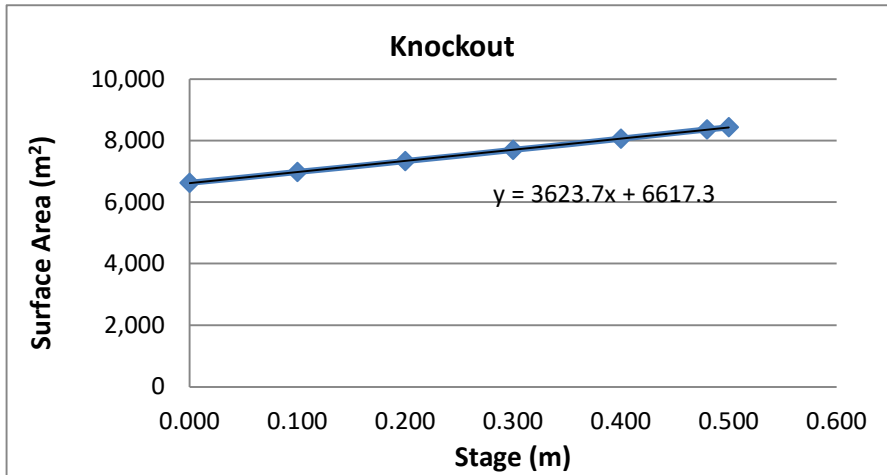
1. The minimum of ditch inlet + knockout, orifice control and pipe discharge flow rate is used in the actual discharge calculation

Interpolate weir flows away from Linear Dispersion Trench

Design Storm	Level (m)	Flow	Weir No. 1 Flow (m ³ /s)	Weir No. 2 Flow (m ³ /s)	Total Weir Flow 1 and 2 (m ³ /s)	Remaining Flow to Linear Dispersion Trench (m ³ /s)
100-Year	457.093	5.771	1.939	0.776	2.715	3.056
Regional	457.012	2.808	0.250	0.100	0.350	2.458

River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Stormwater Management Pond Drawdown Calculations

Elevation	Stage	Surface area	
(m)	(m)	(m²)	
456.00	0.000	6,626	Knockout Invert
456.10	0.100	6,979	
456.20	0.200	7,335	
456.30	0.300	7,696	
456.40	0.400	8,063	
456.48	0.480	8,361	
456.50	0.500	8,436	DICB Lip
456.60	0.600	8,904	
456.70	0.700	9,344	
456.80	0.800	9,798	
456.90	0.900	10,270	
457.00	1.000	10,756	Weir Control
457.10	1.100	11,253	
457.20	1.200	11,769	
457.30	1.300	12,298	
457.40	1.400	13,083	Top of Pond
457.50	1.500	15,629	Overflow

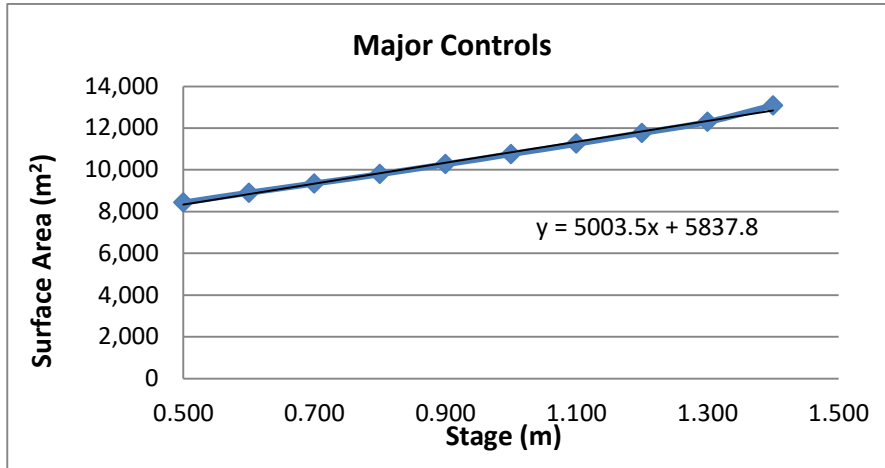


River's Edge Subdivision

Town of Grand Valley

File No. 104-104

Stormwater Management Pond Drawdown Calculations



$$t = \frac{0.66C_2h^{1.5} + 2C_3h^{0.5}}{2.75A_o}$$

Eq. 4.11 (MOE, 2003)

Knockout

Given: d = 0.215 m
 A_o = 0.036 m
 C₂ = 3624
 C₃ = 6617

Major Controls

Given: d = 0.600 m x 3
 A_o = 0.848 m
 C₂ = 5004
 C₃ = 5838

Storm	Ponding Elevation (m)	Knockout h (m)	Knockout Drawdown (hr)	Major Controls h (m)	Major Controls Drawdown (hr)	Total Drawdown (hr)
25mm	456.50	0.39	24.7	0.00	0.0	24.7
2-Year	456.59	0.39	24.7	0.09	0.4	25.1
5-Year	456.77	0.39	24.7	0.26	0.8	25.5
100-Year	457.09	0.39	24.7	0.59	1.3	26.0
Regional	457.01	0.39	24.7	0.51	1.1	25.8

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104**

Stormwater Management Facility

Forebay at Inlet 1 Sizing

Forebay Length =	34.0 m	(Dist)
Forebay Top Width =	11.0 m	
Active Forebay Depth =	1.0 m	(d)
Active Forebay Bottom Width =	5.0 m	
Approximate Permanent Forebay Pool Volume =	272 m ³	
Length Width Ratio =	3.1 :1	(r)
25 mm Storm Peak Flowrate =	0.060 m ³ /s	(Q25mm)
5 Year Storm Inflow Rate =	1.806 m ³ /s	(Q5)
Desired Forebay Velocity =	0.500 m/s	(Vf)
Desired Settling Velocity (recommended) =	0.0003 m/s	(Vs)

Settling Length

$$\text{Dist} = ((r \times Q_{25\text{mm}})/V_s)^{.5} = 24.9 \text{ m} \quad 25\text{mm}$$

Forebay length (34 m) exceeds the settling length (24.9 m).

Dispersion Length

$$\text{Dist} = (8 \times Q_5)/(d \times V_f) = 28.9 \text{ m} \quad 5 \text{ Year}$$

Forebay length (34 m) exceeds dispersion length (28.9 m).

Flow Velocity in Forebay

Cross-sectional Area =	8 m ²	
Cross-sectional Area (With Permanent Pool) =	11.75 m ²	A
Q5 =	1.806 m ³ /s	
Velocity = Q5/A =	0.15 m/s	5 Year

The average flow velocity through the forebay meets the allowable velocity of 0.15 m/s.

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104**

**Stormwater Management Facility
Forebay at Inlet 2 Sizing**

Forebay Length =	33.0 m	(Dist)
Forebay Top Width =	10.5 m	
Active Forebay Depth =	1.0 m	(d)
Active Forebay Bottom Width =	4.5 m	
Approximate Permanent Forebay Pool Volume =	247.5 m ³	
Length Width Ratio =	3.1 :1	(r)
25 mm Storm Peak Flowrate =	0.060 m ³ /s	(Q25mm)
5 Year Storm Inflow Rate =	0.462 m ³ /s	(Q5)
Desired Forebay Velocity =	0.500 m/s	(Vf)
Desired Settling Velocity (recommended) =	0.0003 m/s	(Vs)

Settling Length

$$\text{Dist} = ((r \times Q_{25\text{mm}})/V_s)^{.5} = 25.1 \text{ m} \quad 25\text{mm}$$

Forebay length (33 m) exceeds the settling length (25.1 m).

Dispersion Length

$$\text{Dist} = (8 \times Q_5)/(d \times V_f) = 7.4 \text{ m} \quad 5 \text{ Year}$$

Forebay length (33 m) exceeds dispersion length (7.4 m).

Flow Velocity in Forebay

Cross-sectional Area =	7.5 m ²	
Cross-sectional Area (With Permanent Pool) =	11.1 m ²	A
Q5 =	0.462 m ³ /s	

$$\text{Velocity} = Q_5/A = 0.04 \text{ m/s} \quad 5 \text{ Year}$$

The average flow velocity through the forebay meets the allowable velocity of 0.15 m/s.

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104**

Stormwater Management Facility

Forebay at Inlet 3 Sizing

Forebay Length =	38.5 m	(Dist)
Forebay Top Width =	14.5 m	
Active Forebay Depth =	1.0 m	(d)
Active Forebay Bottom Width =	8.5 m	

Approximate Permanent Forebay Pool Volume = 442.75 m³

Length Width Ratio =	2.7 :1	(r)
25 mm Storm Peak Flowrate =	0.060 m ³ /s	(Q25mm)
5 Year Storm Inflow Rate =	1.988 m ³ /s	(Q5)

Desired Forebay Velocity =	0.500 m/s	(Vf)
Desired Settling Velocity (recommended) =	0.0003 m/s	(Vs)

Settling Length

Dist = ((r x Q25mm)/Vs)^{.5} = 23.0 m 25mm

Forebay length (38.5 m) exceeds the settling length (23 m).

Dispersion Length

Dist = (8 x Q5)/(d x Vf) = 31.8 m 5 Year

Forebay length (38.5 m) exceeds dispersion length (31.8 m).

Flow Velocity in Forebay

Cross-sectional Area =	11.5 m ²	
Cross-sectional Area (With Permanent Pool) =	16.3 m ²	A
Q5 =	1.988 m ³ /s	

Velocity = Q5/A = 0.12 m/s 5 Year

The average flow velocity through the forebay meets the allowable velocity of 0.15 m/s.

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Equalizing Pipe Calculation**

Flow at Pond Inlets 1, 2 and 3

	Regional Storm (m³/s)	100 Year Storm (m³/s)	5 Year Storm (m³/s)	2 Year Storm (m³/s)	25mm Storm (m³/s)
Inlet 1	1.310	3.244	1.806	1.328	1.185
Inlet 2	0.325	0.979	0.462	0.323	0.334
Total 1 and 2	1.635	4.223	2.268	1.651	1.519
Inlet 3	1.153	3.416	1.988	1.490	1.375

**Ponding
Elevation (m)**

Regional Storm	457.01
100 Year Storm	457.09
5 Year Storm	456.77
2 Year Storm	456.59
25 mm Storm	456.5

1. Assume that the equalizing pipes act as an orifice, equalizing the flow between the two portions of the pond. The equalizing pipes must convey the greater of Inlet 1+2 flows or Inlet 3 flows.
2. Assume 3-900 mm diameter equalizing pipes installed in parallel at the bottom of permanent pool elevation.

Orifice Regional Storm

INV 455.70 m

Q = 1.568 m³/s
Cd = 0.600
H = 0.86 m
2g = 19.620

A = 0.636 m²
D = 0.900 m

Q for 3 pipes Greather than
(m³/s) (m³/s)
4.704 1.635

Orifice 100 Year Storm

INV 455.70 m

Q = 1.639 m³/s
Cd = 0.600
H = 0.94 m
2g = 19.620

A = 0.636 m²
D = 0.900 m

Q for 3 pipes Greather than
(m³/s) (m³/s)
4.918 4.223

**River's Edge Subdivision
Town of Grand Valley
File No. 104-104
Equalizing Pipe Calculation**

Orifice 5 Year Storm

INV	455.70	m		
			Q for 3 pipes	Greather than
Q =	1.331	m ³ /s	(m ³ /s)	(m ³ /s)
Cd =	0.600		3.994	2.268
H =	0.62	m		
2g =	19.620			
A =	0.636	m ²		
D =	0.900	m		

Orifice 2 Year Storm

INV	455.70	m		
			Q for 3 pipes	Greather than
Q =	1.122	m ³ /s	(m ³ /s)	(m ³ /s)
Cd =	0.600		3.365	1.651
H =	0.44	m		
2g =	19.620			
A =	0.636	m ²		
D =	0.900	m		

Orifice 25mm Storm

INV	455.70	m		
			Q for 3 pipes	Greather than
Q =	1.000	m ³ /s	(m ³ /s)	(m ³ /s)
Cd =	0.600		3.001	1.519
H =	0.35	m		
2g =	19.620			
A =	0.636	m ²		
D =	0.900	m		

**River's Edge Subdivision
Town of Grand Valley
File No: 104-104
Linear Dispersion Trench**

Storage Volume Calculations						
Elevation	Depth	Surface Area	Increase Stone Volume	Increase Storage Volume	Accum. Storage Volume	
(m)	(m)	(m ²)	(m ³)	(m ³)	(m ³)	
454.86	0.00	120.00	0.00	0.00	0.00	Bottom of Stone
454.96	0.10	120.00	12.00	4.00	4.00	
455.06	0.20	120.00	12.00	4.00	8.00	
455.16	0.30	120.00	12.00	4.00	12.00	
455.26	0.40	120.00	12.00	4.00	16.00	
455.36	0.50	120.00	12.00	4.00	20.00	Top of Stone/Weir
455.46	0.60	0.00	0.00	0.50	20.50	
455.56	0.70	0.00	0.00	0.50	21.00	
455.66	0.80	0.00	0.00	0.50	21.50	

Overflow Weir						
Elev	d1	h	H	2g	L	Q
m	m	m	m	m/s ²	m	m ³ /s
455.36	0.50	0.50	0.00	19.62	38.00	0.00
455.46	0.60	0.50	0.10	19.62	38.00	1.70
455.56	0.70	0.50	0.20	19.62	38.00	4.98
455.66	0.80	0.50	0.30	19.62	38.00	9.39

BOTTOM

L(dw) = 15.00 m
W(dw) = 8.00 m
D(dw) = 0.50 m

A(c) = 120.0 m²

K = 1.00E-05 cm/s

SIDES

L(dw) = 15.00 m

D(dw) = 0.50 m

A(c) = 15.0 sq m (both sides)

**River's Edge Subdivision
Town of Grand Valley
File No: 104-104
Linear Dispersion Trench**

Stage/Storage/Discharge Table

Stage (m)	Storage (m³)	Infiltration Discharge (m³/s)	Overflow Weir Discharge (m³/s)	Total Discharge (m³/s)	
454.860	0.00	0.00000	0.00	0.000	Bottom of Stone
454.960	4.00	0.00001	0.00	0.000	
455.060	8.00	0.00001	0.00	0.000	
455.160	12.00	0.00001	0.00	0.000	
455.260	16.00	0.00001	0.00	0.000	
455.360	20.00	0.00001	0.00	0.000	Top of Stone/Weir
455.460	20.50	0.00001	1.70	1.702	
455.560	21.00	0.00001	4.98	4.979	
455.660	21.50	0.00001	9.39	9.395	

Interpolate for Flow to Wetland and Grand River

	Linear Dispersion Trench		
	Wetland	Grand River	
Design Storm	Flow (m³/s)	Flow (m³/s)	Flow (m³/s)
25mm	0.060	0.024	0.036
2-Year	0.264	0.104	0.160
5-Year	1.235	0.488	0.747
100-Year	3.056	1.207	1.849
Regional	2.458	0.971	1.487

Notes:

1. Assumes that linear dispersion trench is flat and overflow is acting like a weir with three (3) sides (north, south and east)
2. Assumes that south side of linear dispersion trench discharges to the existing wetland. Wetland flow split is (15m / 38m = 39.5%)


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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 post 002 year-Uncontrolled.out"
"          Licensee name:                      gmbp"
"          Company                            "
"          Date & Time last used:              2/21/2023 at 1:57:01 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.047 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.292  mm/hr"
"          Total depth                          33.014  mm"
"          6 002hyd Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302 External lands and water tower"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.072  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	22.063	2.281	3.430	minutes"
"	Time to Centroid	96.472	86.938	87.492	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	99.54	121.66	221.19	c.m"
"	Rainfall losses	30.676	2.000	14.904	mm"
"	Runoff depth	2.338	31.014	18.110	mm"
"	Runoff volume	7.05	114.28	121.33	c.m"
"	Runoff coefficient	0.071	0.939	0.549	"
"	Maximum flow	0.004	0.072	0.072	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

	4	Add Runoff "			
"		0.072	0.072	0.000	0.000"

" 33 CATCHMENT 201"

	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.783	0.072	0.000	0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	68.866	7.119	7.629	minutes"
"	Time to Centroid	138.361	93.923	94.289	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	141.30	1271.69	1412.99	c.m"
"	Rainfall losses	30.669	1.681	4.580	mm"
"	Runoff depth	2.345	31.333	28.434	mm"
"	Runoff volume	10.04	1206.95	1216.98	c.m"
"	Runoff coefficient	0.071	0.949	0.861	"

"		Maximum flow	0.002	0.782	0.783	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.783	0.855	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.125	0.855	0.000	0.000	c.m/sec"
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	48.109	4.973	7.458	minutes"
"		Time to Centroid	119.817	90.843	92.512	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	161.93	197.92	359.85	c.m"
"		Rainfall losses	30.669	1.629	14.697	mm"
"		Runoff depth	2.345	31.385	18.317	mm"
"		Runoff volume	11.50	188.15	199.65	c.m"
"		Runoff coefficient	0.071	0.951	0.555	"
"		Maximum flow	0.004	0.124	0.125	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.125	0.979	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

```

"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.511    0.979    0.000    0.000 c.m/sec"
"      Catchment 202      Pervious      Impervious      Total Area  "
"      Surface Area      1.660      2.490      4.150      hectare"
"      Time of concentration  53.181    5.498      7.765      minutes"
"      Time to Centroid      124.334   91.560     93.119     minutes"
"      Rainfall depth      33.014    33.014     33.014     mm"
"      Rainfall volume      548.03    822.04     1370.07    c.m"
"      Rainfall losses      30.670    1.714      13.296     mm"
"      Runoff depth         2.344     31.300     19.718     mm"
"      Runoff volume        38.92     779.36     818.28     c.m"
"      Runoff coefficient    0.071     0.948      0.597      "
"      Maximum flow         0.011     0.510      0.511      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.511    1.490    0.000    0.000"
" 33  CATCHMENT 203"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      203  Internal to Pond Inet 2"
"    60.000  % Impervious"
"      2.760  Total Area"
"    30.000  Flow length"
"      4.000  Overland Slope"
"      1.104  Pervious Area"
"    30.000  Pervious length"
"      2.000  Pervious slope"
"      1.656  Impervious Area"
"    30.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

```

"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.323	1.490	0.000	0.000	c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	1.104	1.656	2.760	hectare"
"		Time of concentration	22.063	2.281	3.227	minutes"
"		Time to Centroid	96.472	86.938	87.394	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	364.47	546.71	911.18	c.m"
"		Rainfall losses	30.676	2.000	13.471	mm"
"		Runoff depth	2.338	31.014	19.543	mm"
"		Runoff volume	25.81	513.58	539.40	c.m"
"		Runoff coefficient	0.071	0.939	0.592	"
"		Maximum flow	0.016	0.322	0.323	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.323	1.813	0.000	0.000"	
" 33		CATCHMENT 303"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	303	External"				
"	55.000	% Impervious"				
"	3.550	Total Area"				
"	140.000	Flow length"				
"	2.000	Overland Slope"				
"	1.597	Pervious Area"				
"	140.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.952	Impervious Area"				
"	140.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.397	1.813	0.000	0.000	c.m/sec"

	Catchment 303	Pervious	Impervious	Total Area	
"	Surface Area	1.597	1.952	3.550	hectare"
"	Time of concentration	55.599	5.748	8.629	minutes"
"	Time to Centroid	126.518	91.941	93.939	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	527.40	644.59	1171.99	c.m"
"	Rainfall losses	30.669	1.730	14.752	mm"
"	Runoff depth	2.345	31.284	18.261	mm"
"	Runoff volume	37.47	610.81	648.28	c.m"
"	Runoff coefficient	0.071	0.948	0.553	"
"	Maximum flow	0.011	0.397	0.397	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.397	2.210	0.000	0.000"
" 33	CATCHMENT 204"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	204	Park"			
"	10.000	% Impervious"			
"	2.090	Total Area"			
"	140.000	Flow length"			
"	2.000	Overland Slope"			
"	1.881	Pervious Area"			
"	140.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.209	Impervious Area"			
"	140.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.043	2.210	0.000	0.000 c.m/sec"
"	Catchment 204	Pervious	Impervious	Total Area	"
"	Surface Area	1.881	0.209	2.090	hectare"
"	Time of concentration	55.599	5.748	25.832	minutes"
"	Time to Centroid	126.518	91.941	105.871	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	620.99	69.00	689.99	c.m"
"	Rainfall losses	30.669	1.730	27.775	mm"
"	Runoff depth	2.345	31.284	5.239	mm"
"	Runoff volume	44.12	65.38	109.50	c.m"
"	Runoff coefficient	0.071	0.948	0.159	"

"		Maximum flow	0.013	0.042	0.043	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.043	2.254	0.000	0.000"	
" 33		CATCHMENT 301"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	301	Scott Street External"				
"	55.000	% Impervious"				
"	2.600	Total Area"				
"	95.000	Flow length"				
"	2.000	Overland Slope"				
"	1.170	Pervious Area"				
"	95.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.430	Impervious Area"				
"	95.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.300	2.254	0.000	0.000	c.m/sec"
"		Catchment 301	Pervious	Impervious	Total Area	"
"		Surface Area	1.170	1.430	2.600	hectare"
"		Time of concentration	44.058	4.555	6.834	minutes"
"		Time to Centroid	116.161	90.239	91.735	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	386.26	472.10	858.36	c.m"
"		Rainfall losses	30.671	1.703	14.738	mm"
"		Runoff depth	2.343	31.311	18.275	mm"
"		Runoff volume	27.41	447.75	475.16	c.m"
"		Runoff coefficient	0.071	0.948	0.554	"
"		Maximum flow	0.009	0.299	0.300	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.300	2.554	0.000	0.000"	
" 33		CATCHMENT 205"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	205	Internal to Pond Inet 1"				
"	65.000	% Impervious"				

```

"      4.320  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      1.512  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.808  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.587    2.554    0.000    0.000 c.m/sec"
"      Catchment 205      Pervious      Impervious      Total Area  "
"      Surface Area      1.512      2.808      4.320      hectare"
"      Time of concentration 45.435      4.697      6.274      minutes"
"      Time to Centroid    117.422    90.442    91.487    minutes"
"      Rainfall depth      33.014      33.014      33.014    mm"
"      Rainfall volume      499.17      927.03      1426.20   c.m"
"      Rainfall losses      30.669      1.666      11.817    mm"
"      Runoff depth         2.345      31.347      21.197    mm"
"      Runoff volume         35.46      880.24      915.69    c.m"
"      Runoff coefficient    0.071      0.950      0.642     "
"      Maximum flow         0.012      0.586      0.587     c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.587    3.141    0.000    0.000"
" 33  CATCHMENT 206"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      206  Pond"
"      80.000  % Impervious"
"      3.290  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      0.658  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.632  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

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"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"              0.550      3.141      0.000      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration 45.435      4.697      5.445      minutes"
"      Time to Centroid 117.422      90.442      90.937      minutes"
"      Rainfall depth 33.014      33.014      33.014      mm"
"      Rainfall volume 217.23      868.92      1086.15      c.m"
"      Rainfall losses 30.669      1.666      7.467      mm"
"      Runoff depth 2.345      31.347      25.547      mm"
"      Runoff volume 15.43      825.06      840.49      c.m"
"      Runoff coefficient 0.071      0.950      0.774      "
"      Maximum flow 0.005      0.549      0.550      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.550      3.691      0.000      0.000"
" 33      CATCHMENT 200"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      200      Rear yard and Open Space to Grand River"
"      10.000      % Impervious"
"      7.530      Total Area"
"      100.000      Flow length"
"      5.000      Overland Slope"
"      6.777      Pervious Area"
"      100.000      Pervious length"
"      2.000      Pervious slope"
"      0.753      Impervious Area"
"      100.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"
"      0.015      Impervious Manning 'n'"
"      0.000      Impervious Max.infiltration"
"      0.000      Impervious Min.infiltration"
"      0.050      Impervious Lag constant (hours)"
"      1.500      Impervious Depression storage"
"              0.162      3.691      0.000      0.000 c.m/sec"

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	Catchment 200	Pervious	Impervious	Total Area	
"	Surface Area	6.777	0.753	7.530	hectare"
"	Time of concentration	45.435	4.697	21.089	minutes"
"	Time to Centroid	117.422	90.442	101.298	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	2237.35	248.59	2485.94	c.m"
"	Rainfall losses	30.669	1.666	27.769	mm"
"	Runoff depth	2.345	31.347	5.245	mm"
"	Runoff volume	158.93	236.05	394.97	c.m"
"	Runoff coefficient	0.071	0.950	0.159	"
"	Maximum flow	0.053	0.157	0.162	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.162	3.852	0.000	0.000"
" 38	START/RE-START TOTALS 200"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			36.330	hectare"
"	Total Impervious area			18.751	hectare"
"	Total % impervious			51.612"	
" 19	EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                        C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                   104-104 post 005 year-Uncontrolled.out"
"          Licensee name:                     gmbp"
"          Company                            "
"          Date & Time last used:            2/21/2023 at 1:57:33 PM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                   113.586  mm/hr"
"          Total depth                         49.792  mm"
"          6 005hyd Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302 External lands and water tower"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.104  0.000  0.000  0.000 c.m/sec"

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	Catchment 302	Pervious	Impervious	Total Area	
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	14.420	2.108	4.886	minutes"
"	Time to Centroid	100.204	85.724	88.991	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	150.12	183.48	333.60	c.m"
"	Rainfall losses	32.852	2.224	16.007	mm"
"	Runoff depth	16.939	47.568	33.785	mm"
"	Runoff volume	51.07	175.29	226.36	c.m"
"	Runoff coefficient	0.340	0.955	0.679	"
"	Maximum flow	0.028	0.095	0.104	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.104	0.104	0.000	0.000"
" 33	CATCHMENT 201"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		1.019	0.104	0.000	0.000 c.m/sec"
"	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	45.009	6.580	8.036	minutes"
"	Time to Centroid	135.435	91.865	93.515	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	213.11	1917.98	2131.08	c.m"
"	Rainfall losses	32.839	1.944	5.034	mm"
"	Runoff depth	16.953	47.847	44.758	mm"
"	Runoff volume	72.56	1843.08	1915.64	c.m"
"	Runoff coefficient	0.340	0.961	0.899	"

"		Maximum flow	0.018	1.018	1.019	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		1.019	1.124	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.169	1.124	0.000	0.000	c.m/sec"
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	31.443	4.597	10.620	minutes"
"		Time to Centroid	119.805	89.191	96.060	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	244.23	298.50	542.73	c.m"
"		Rainfall losses	32.853	1.876	15.816	mm"
"		Runoff depth	16.939	47.916	33.976	mm"
"		Runoff volume	83.09	287.25	370.34	c.m"
"		Runoff coefficient	0.340	0.962	0.682	"
"		Maximum flow	0.027	0.165	0.169	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.169	1.293	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

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"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.695    1.293    0.000    0.000 c.m/sec"
"      Catchment 202          Pervious    Impervious Total Area "
"      Surface Area          1.660      2.490      4.150    hectare"
"      Time of concentration  34.758    5.082    10.733    minutes"
"      Time to Centroid      123.626   89.815    96.253    minutes"
"      Rainfall depth        49.792    49.792    49.792    mm"
"      Rainfall volume        826.54    1239.81   2066.36   c.m"
"      Rainfall losses        32.847    1.767     14.199    mm"
"      Runoff depth           16.945    48.025    35.593    mm"
"      Runoff volume           281.29    1195.82   1477.11   c.m"
"      Runoff coefficient      0.340     0.965     0.715     "
"      Maximum flow           0.083     0.684     0.695     c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4      Add Runoff "
"          0.695    1.988    0.000    0.000"
" 33      CATCHMENT 203"
"          1      Triangular SCS"
"          1      Equal length"
"          2      Horton equation"
"          203    Internal to Pond Inet 2"
"          60.000  % Impervious"
"          2.760  Total Area"
"          30.000  Flow length"
"          4.000  Overland Slope"
"          1.104  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          1.656  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"

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"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.462	1.988	0.000	0.000	c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	1.104	1.656	2.760	hectare"
"		Time of concentration	14.420	2.108	4.470	minutes"
"		Time to Centroid	100.204	85.724	88.502	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	549.70	824.55	1374.25	c.m"
"		Rainfall losses	32.852	2.224	14.475	mm"
"		Runoff depth	16.939	47.568	35.316	mm"
"		Runoff volume	187.01	787.72	974.73	c.m"
"		Runoff coefficient	0.340	0.955	0.709	"
"		Maximum flow	0.103	0.429	0.462	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.462	2.450	0.000	0.000"	
" 33		CATCHMENT 303"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	303	External"				
"	55.000	% Impervious"				
"	3.550	Total Area"				
"	140.000	Flow length"				
"	2.000	Overland Slope"				
"	1.597	Pervious Area"				
"	140.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.952	Impervious Area"				
"	140.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.544	2.450	0.000	0.000	c.m/sec"

"	Catchment 303	Pervious	Impervious	Total Area	"
"	Surface Area	1.597	1.952	3.550	hectare"
"	Time of concentration	36.338	5.313	12.264	minutes"
"	Time to Centroid	125.451	90.132	98.045	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	795.42	972.18	1767.60	c.m"
"	Rainfall losses	32.839	1.757	15.744	mm"
"	Runoff depth	16.953	48.035	34.048	mm"
"	Runoff volume	270.82	937.88	1208.71	c.m"
"	Runoff coefficient	0.340	0.965	0.684	"
"	Maximum flow	0.078	0.534	0.544	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.544 2.993 0.000 0.000"				
" 33	CATCHMENT 204"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	204 Park"				
"	10.000 % Impervious"				
"	2.090 Total Area"				
"	140.000 Flow length"				
"	2.000 Overland Slope"				
"	1.881 Pervious Area"				
"	140.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.209 Impervious Area"				
"	140.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.103 2.993 0.000 0.000 c.m/sec"				
"	Catchment 204	Pervious	Impervious	Total Area	"
"	Surface Area	1.881	0.209	2.090	hectare"
"	Time of concentration	36.338	5.313	28.909	minutes"
"	Time to Centroid	125.451	90.132	116.994	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	936.58	104.06	1040.65	c.m"
"	Rainfall losses	32.839	1.757	29.731	mm"
"	Runoff depth	16.953	48.035	20.061	mm"
"	Runoff volume	318.89	100.39	419.28	c.m"
"	Runoff coefficient	0.340	0.965	0.403	"

"		Maximum flow	0.092	0.057	0.103	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.103	3.062	0.000	0.000"	
" 33		CATCHMENT 301"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	301	Scott Street External"				
"	55.000	% Impervious"				
"	2.600	Total Area"				
"	95.000	Flow length"				
"	2.000	Overland Slope"				
"	1.170	Pervious Area"				
"	95.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.430	Impervious Area"				
"	95.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.405	3.062	0.000	0.000 c.m/sec"	
"		Catchment 301	Pervious	Impervious	Total Area	"
"		Surface Area	1.170	1.430	2.600	hectare"
"		Time of concentration	28.795	4.210	9.735	minutes"
"		Time to Centroid	116.746	88.669	94.978	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	582.56	712.02	1294.58	c.m"
"		Rainfall losses	32.859	1.996	15.884	mm"
"		Runoff depth	16.933	47.796	33.908	mm"
"		Runoff volume	198.12	683.48	881.60	c.m"
"		Runoff coefficient	0.340	0.960	0.681	"
"		Maximum flow	0.068	0.395	0.405	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.405	3.467	0.000	0.000"	
" 33		CATCHMENT 205"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	205	Internal to Pond Inet 1"				
"	65.000	% Impervious"				

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"      4.320  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      1.512  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.808  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.788    3.467    0.000    0.000 c.m/sec"
"      Catchment 205      Pervious      Impervious      Total Area  "
"      Surface Area      1.512      2.808      4.320      hectare"
"      Time of concentration 29.695      4.341      8.402      minutes"
"      Time to Centroid      117.798      88.850      93.486      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      752.85      1398.15      2151.00      c.m"
"      Rainfall losses      32.840      1.920      12.742      mm"
"      Runoff depth      16.952      47.871      37.050      mm"
"      Runoff volume      256.32      1344.23      1600.55      c.m"
"      Runoff coefficient      0.340      0.961      0.744      "
"      Maximum flow      0.086      0.775      0.788      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.788    4.256    0.000    0.000"
" 33      CATCHMENT 206"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      206      Pond"
"      80.000  % Impervious"
"      3.290  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      0.658  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.632  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

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"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.732	4.256	0.000	0.000 c.m/sec"
"		Catchment 206	Pervious	Impervious	Total Area	"
"		Surface Area	0.658	2.632	3.290	hectare"
"		Time of concentration	29.695	4.341	6.403	minutes"
"		Time to Centroid	117.798	88.850	91.205	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	327.63	1310.52	1638.15	c.m"
"		Rainfall losses	32.840	1.920	8.104	mm"
"		Runoff depth	16.952	47.871	41.688	mm"
"		Runoff volume	111.55	1259.97	1371.52	c.m"
"		Runoff coefficient	0.340	0.961	0.837	"
"		Maximum flow	0.038	0.726	0.732	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.732	4.988	0.000	0.000"
" 33		CATCHMENT 200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	200	Rear yard and Open Space to Grand River"				
"	10.000	% Impervious"				
"	7.530	Total Area"				
"	100.000	Flow length"				
"	5.000	Overland Slope"				
"	6.777	Pervious Area"				
"	100.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.753	Impervious Area"				
"	100.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.437	4.988	0.000	0.000 c.m/sec"

	Catchment 200	Pervious	Impervious	Total Area	
"	Surface Area	6.777	0.753	7.530	hectare"
"	Time of concentration	29.695	4.341	23.640	minutes"
"	Time to Centroid	117.798	88.850	110.884	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	3374.38	374.93	3749.31	c.m"
"	Rainfall losses	32.840	1.920	29.748	mm"
"	Runoff depth	16.952	47.871	20.044	mm"
"	Runoff volume	1148.85	360.47	1509.32	c.m"
"	Runoff coefficient	0.340	0.961	0.403	"
"	Maximum flow	0.387	0.208	0.437	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.437	5.256	0.000	0.000"
" 38	START/RE-START TOTALS 200"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area			36.330	hectare"
"	Total Impervious area			18.751	hectare"
"	Total % impervious			51.612"	
" 19	EXIT"				

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"          MIDUSS Output ----->"
"          MIDUSS version                Version 2.25  rev. 473"
"          MIDUSS created                 Sunday, February 07, 2010"
"          10  Units used:                ie METRIC"
"          Job folder:                   C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:              104-104 post 100 year-Uncontrolled.out"
"          Licensee name:                gmbp"
"          Company                       "
"          Date & Time last used:        2/21/2023 at 1:58:24 PM"
" 31          TIME PARAMETERS"
"          5.000  Time Step"
"          180.000  Max. Storm length"
"          3600.000  Max. Hydrograph"
" 32          STORM Chicago storm"
"          1  Chicago storm"
"          6933.020  Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity              168.777  mm/hr"
"          Total depth                    97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302  External lands and water tower"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.233  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	"
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	10.076	1.799	4.711	minutes"
"	Time to Centroid	97.792	84.352	89.081	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	295.23	360.84	656.07	c.m"
"	Rainfall losses	34.803	2.791	17.197	mm"
"	Runoff depth	63.119	95.130	80.725	mm"
"	Runoff volume	190.30	350.55	540.86	c.m"
"	Runoff coefficient	0.645	0.971	0.824	"
"	Maximum flow	0.099	0.153	0.233	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

"	4	Add Runoff "			
"		0.233	0.233	0.000	0.000"

" 33 CATCHMENT 201"

"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		1.688	0.233	0.000	0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	31.450	5.616	7.390	minutes"
"	Time to Centroid	122.128	89.288	91.543	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	419.10	3771.93	4191.03	c.m"
"	Rainfall losses	34.482	2.307	5.524	mm"
"	Runoff depth	63.439	95.615	92.397	mm"
"	Runoff volume	271.52	3683.07	3954.59	c.m"
"	Runoff coefficient	0.648	0.976	0.944	"

"		Maximum flow	0.079	1.669	1.688	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		1.688	1.920	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.301	1.920	0.000	0.000 c.m/sec"	
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	21.971	3.923	10.297	minutes"
"		Time to Centroid	111.333	87.214	95.731	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	480.30	587.04	1067.34	c.m"
"		Rainfall losses	34.507	2.882	17.113	mm"
"		Runoff depth	63.414	95.039	80.808	mm"
"		Runoff volume	311.04	569.76	880.81	c.m"
"		Runoff coefficient	0.648	0.971	0.825	"
"		Maximum flow	0.114	0.260	0.301	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.301	2.221	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

"	4.150	Total Area"				
"	130.000	Flow length"				
"	4.000	Overland Slope"				
"	1.660	Pervious Area"				
"	130.000	Pervious length"				
"	2.000	Pervious slope"				
"	2.490	Impervious Area"				
"	130.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			1.195	2.221	0.000	0.000 c.m/sec"
"		Catchment 202	Pervious	Impervious	Total Area	"
"		Surface Area	1.660	2.490	4.150	hectare"
"		Time of concentration	24.287	4.337	10.458	minutes"
"		Time to Centroid	113.975	87.701	95.762	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	1625.49	2438.24	4063.73	c.m"
"		Rainfall losses	34.488	2.372	15.219	mm"
"		Runoff depth	63.434	95.549	82.703	mm"
"		Runoff volume	1053.00	2379.17	3432.17	c.m"
"		Runoff coefficient	0.648	0.976	0.845	"
"		Maximum flow	0.358	1.078	1.195	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			1.195	3.416	0.000	0.000"
" 33		CATCHMENT 203"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	203	Internal to Pond Inet 2"				
"	60.000	% Impervious"				
"	2.760	Total Area"				
"	30.000	Flow length"				
"	4.000	Overland Slope"				
"	1.104	Pervious Area"				
"	30.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.656	Impervious Area"				
"	30.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				

"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.979	3.416	0.000	0.000	c.m/sec"
"		Catchment 203	Pervious	Impervious	Total Area	"
"		Surface Area	1.104	1.656	2.760	hectare"
"		Time of concentration	10.076	1.799	4.338	minutes"
"		Time to Centroid	97.792	84.352	88.474	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	1081.05	1621.58	2702.63	c.m"
"		Rainfall losses	34.803	2.791	15.596	mm"
"		Runoff depth	63.119	95.130	82.325	mm"
"		Runoff volume	696.83	1575.35	2272.18	c.m"
"		Runoff coefficient	0.645	0.971	0.841	"
"		Maximum flow	0.363	0.689	0.979	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.979	4.396	0.000	0.000"	
" 33		CATCHMENT 303"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	303	External"				
"	55.000	% Impervious"				
"	3.550	Total Area"				
"	140.000	Flow length"				
"	2.000	Overland Slope"				
"	1.597	Pervious Area"				
"	140.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.952	Impervious Area"				
"	140.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.948	4.396	0.000	0.000	c.m/sec"

	Catchment 303	Pervious	Impervious	Total Area	
"	Surface Area	1.597	1.952	3.550	hectare"
"	Time of concentration	25.391	4.534	11.868	minutes"
"	Time to Centroid	115.220	87.952	97.540	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	1564.29	1911.91	3476.21	c.m"
"	Rainfall losses	34.527	2.269	16.785	mm"
"	Runoff depth	63.395	95.653	81.137	mm"
"	Runoff volume	1012.73	1867.62	2880.35	c.m"
"	Runoff coefficient	0.647	0.977	0.829	"
"	Maximum flow	0.333	0.843	0.948	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4	Add Runoff "			
"		0.948	5.343	0.000	0.000"
" 33	CATCHMENT 204"				
"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	204	Park"			
"	10.000	% Impervious"			
"	2.090	Total Area"			
"	140.000	Flow length"			
"	2.000	Overland Slope"			
"	1.881	Pervious Area"			
"	140.000	Pervious length"			
"	2.000	Pervious slope"			
"	0.209	Impervious Area"			
"	140.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.433	5.343	0.000	0.000 c.m/sec"
"	Catchment 204				
"	Surface Area	1.881	0.209	2.090	hectare"
"	Time of concentration	25.391	4.534	22.397	minutes"
"	Time to Centroid	115.220	87.952	111.305	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	1841.90	204.66	2046.56	c.m"
"	Rainfall losses	34.527	2.269	31.301	mm"
"	Runoff depth	63.395	95.653	66.620	mm"
"	Runoff volume	1192.45	199.91	1392.37	c.m"
"	Runoff coefficient	0.647	0.977	0.680	"

"		Maximum flow	0.393	0.090	0.433	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.433	5.557	0.000	0.000"	
" 33		CATCHMENT 301"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	301	Scott Street External"				
"	55.000	% Impervious"				
"	2.600	Total Area"				
"	95.000	Flow length"				
"	2.000	Overland Slope"				
"	1.170	Pervious Area"				
"	95.000	Pervious length"				
"	2.000	Pervious slope"				
"	1.430	Impervious Area"				
"	95.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.730	5.557	0.000	0.000	c.m/sec"
"		Catchment 301	Pervious	Impervious	Total Area	"
"		Surface Area	1.170	1.430	2.600	hectare"
"		Time of concentration	20.121	3.593	9.455	minutes"
"		Time to Centroid	109.215	86.819	94.762	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	1145.68	1400.27	2545.95	c.m"
"		Rainfall losses	34.520	3.529	17.475	mm"
"		Runoff depth	63.401	94.392	80.446	mm"
"		Runoff volume	741.80	1349.81	2091.61	c.m"
"		Runoff coefficient	0.647	0.964	0.822	"
"		Maximum flow	0.277	0.616	0.730	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.730	6.286	0.000	0.000"	
" 33		CATCHMENT 205"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	205	Internal to Pond Inet 1"				
"	65.000	% Impervious"				

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"      4.320  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      1.512  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.808  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          1.353    6.286    0.000    0.000 c.m/sec"
"      Catchment 205      Pervious      Impervious Total Area "
"      Surface Area      1.512      2.808      4.320      hectare"
"      Time of concentration 20.749      3.705      8.225      minutes"
"      Time to Centroid    109.932      86.956      93.049      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume     1480.57      2749.63      4230.20      c.m"
"      Rainfall losses     34.536      3.348      14.264      mm"
"      Runoff depth        63.385      94.574      83.658      mm"
"      Runoff volume       958.39      2655.63      3614.02      c.m"
"      Runoff coefficient   0.647      0.966      0.854      "
"      Maximum flow        0.359      1.213      1.353      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          1.353    7.639    0.000    0.000"
" 33  CATCHMENT 206"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      206  Pond"
"      80.000  % Impervious"
"      3.290  Total Area"
"    100.000  Flow length"
"      4.000  Overland Slope"
"      0.658  Pervious Area"
"    100.000  Pervious length"
"      2.000  Pervious slope"
"      2.632  Impervious Area"
"    100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"

```

"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		1.198	7.639	0.000	0.000	c.m/sec"
"		Catchment 206	Pervious	Impervious	Total Area	"
"		Surface Area	0.658	2.632	3.290	hectare"
"		Time of concentration	20.749	3.705	6.151	minutes"
"		Time to Centroid	109.932	86.956	90.254	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	644.32	2577.29	3221.61	c.m"
"		Rainfall losses	34.536	3.348	9.585	mm"
"		Runoff depth	63.385	94.574	88.336	mm"
"		Runoff volume	417.08	2489.18	2906.26	c.m"
"		Runoff coefficient	0.647	0.966	0.902	"
"		Maximum flow	0.156	1.137	1.198	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		1.198	8.837	0.000	0.000"	
" 33		CATCHMENT 200"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	200	Rear yard and Open Space to Grand River"				
"	10.000	% Impervious"				
"	7.530	Total Area"				
"	100.000	Flow length"				
"	5.000	Overland Slope"				
"	6.777	Pervious Area"				
"	100.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.753	Impervious Area"				
"	100.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		1.746	8.837	0.000	0.000	c.m/sec"

	Catchment 200	Pervious	Impervious	Total Area	
"	Surface Area	6.777	0.753	7.530	hectare"
"	Time of concentration	20.749	3.705	18.326	minutes"
"	Time to Centroid	109.932	86.956	106.665	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	6636.13	737.35	7373.48	c.m"
"	Rainfall losses	34.536	3.348	31.417	mm"
"	Runoff depth	63.385	94.574	66.504	mm"
"	Runoff volume	4295.63	712.14	5007.77	c.m"
"	Runoff coefficient	0.647	0.966	0.679	"
"	Maximum flow	1.608	0.325	1.746	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	1.746	9.788	0.000	0.000"	
" 38	START/RE-START TOTALS 200"				
"	3 Runoff Totals on EXIT"				
"	Total Catchment area		36.330		hectare"
"	Total Impervious area		18.751		hectare"
"	Total % impervious		51.612"		
" 19	EXIT"				

```

"          MIDUSS Output ----->"
"          MIDUSS version                Version 2.25  rev. 473"
"          MIDUSS created                Sunday, February 07, 2010"
"          10  Units used:                ie METRIC"
"          Job folder:                   C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:              104-104 post  Reg-Uncontrolled.out"
"          Licensee name:                gmbp"
"          Company                       "
"          Date & Time last used:        2/21/2023 at 1:59:03 PM"
" 31          TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          5000.000  Max. Hydrograph"
" 32          STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.026    2.026    2.026    2.028"
"                  2.026    6.000    4.000    6.000    13.000"
"                  17.000    13.000    23.000    13.000    13.000"
"                  53.000    38.000    13.000"
"          Maximum intensity              53.000  mm/hr"
"          Total depth                    285.000  mm"
"          7  9999hyd  Hydrograph extension used in this file"
" 33          CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302  External lands and water tower"
"          55.000  % Impervious"
"          0.670  Total Area"P
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"

```

"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.078	0.000	0.000	0.000	c.m/sec"
"		Catchment 302	Pervious	Impervious	Total Area	"
"		Surface Area	0.302	0.368	0.670	hectare"
"		Time of concentration	16.093	2.860	7.181	minutes"
"		Time to Centroid	2720.207	2251.465	2404.533	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	859.28	1050.22	1909.50	c.m"
"		Rainfall losses	139.563	39.596	84.581	mm"
"		Runoff depth	145.437	245.404	200.419	mm"
"		Runoff volume	438.49	904.31	1342.81	c.m"
"		Runoff coefficient	0.510	0.861	0.703	"
"		Maximum flow	0.031	0.047	0.078	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.078	0.078	0.000	0.000"	
" 33		CATCHMENT 201"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	201	Townhome and Apartment Blocks"				
"	90.000	% Impervious"				
"	4.280	Total Area"				
"	200.000	Flow length"				
"	4.000	Overland Slope"				
"	0.428	Pervious Area"				
"	200.000	Pervious length"				
"	2.000	Pervious slope"				
"	3.852	Impervious Area"				
"	200.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.512	0.078	0.000	0.000	c.m/sec"
"		Catchment 201	Pervious	Impervious	Total Area	"
"		Surface Area	0.428	3.852	4.280	hectare"
"		Time of concentration	50.233	8.926	11.288	minutes"
"		Time to Centroid	2757.880	2264.252	2292.474	minutes"

"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	0.1220	1.0978	1.2198	ha-m"
"		Rainfall losses	138.714	16.956	29.132	mm"
"		Runoff depth	146.286	268.044	255.868	mm"
"		Runoff volume	0.0626	1.0325	1.0951	ha-m"
"		Runoff coefficient	0.513	0.941	0.898	"
"		Maximum flow	0.046	0.484	0.512	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.512	0.590	0.000	0.000"
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.117	0.590	0.000	0.000 c.m/sec"
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	35.092	6.236	15.325	minutes"
"		Time to Centroid	2739.025	2235.505	2394.105	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	1397.93	1708.57	3106.50	c.m"
"		Rainfall losses	140.362	27.637	78.363	mm"
"		Runoff depth	144.638	257.363	206.637	mm"
"		Runoff volume	709.45	1542.89	2252.34	c.m"
"		Runoff coefficient	0.508	0.903	0.725	"
"		Maximum flow	0.054	0.076	0.117	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.117	0.707	0.000	0.000"

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" 33      CATCHMENT 202"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          202 Internal to Pond Inet 3"
"          60.000 % Impervious"
"          4.150 Total Area"
"         130.000 Flow length"
"          4.000 Overland Slope"
"          1.660 Pervious Area"
"         130.000 Pervious length"
"          2.000 Pervious slope"
"          2.490 Impervious Area"
"         130.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"
"          0.447      0.707      0.000      0.000 c.m/sec"
"          Catchment 202      Pervious      Impervious Total Area "
"          Surface Area      1.660      2.490      4.150      hectare"
"          Time of concentration 38.792      6.893      15.449      minutes"
"          Time to Centroid      2741.816      2242.481      2376.420      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      0.4731      0.7096      1.1828      ha-m"
"          Rainfall losses      141.611      24.215      71.174      mm"
"          Runoff depth      143.389      260.785      213.826      mm"
"          Runoff volume      2380.25      6493.54      8873.79      c.m"
"          Runoff coefficient      0.503      0.915      0.750      "
"          Maximum flow      0.182      0.315      0.447      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"
"          0.447      1.153      0.000      0.000"
" 33      CATCHMENT 203"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          203 Internal to Pond Inet 2"
"          60.000 % Impervious"
"          2.760 Total Area"
"          30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"

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"      30.000 Pervious length"
"      2.000 Pervious slope"
"      1.656 Impervious Area"
"      30.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          0.325      1.153      0.000      0.000 c.m/sec"
"      Catchment 203      Pervious      Impervious Total Area "
"      Surface Area      1.104      1.656      2.760      hectare"
"      Time of concentration 16.093      2.860      6.608      minutes"
"      Time to Centroid      2720.207      2251.465      2384.215      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      3146.40      4719.60      7866.00      c.m"
"      Rainfall losses      139.563      39.596      79.583      mm"
"      Runoff depth      145.437      245.404      205.417      mm"
"      Runoff volume      1605.63      4063.89      5669.52      c.m"
"      Runoff coefficient      0.510      0.861      0.721      "
"      Maximum flow      0.115      0.211      0.325      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.325      1.479      0.000      0.000"
" 33      CATCHMENT 303"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      303      External"
"      55.000      % Impervious"
"      3.550      Total Area"
"      140.000      Flow length"
"      2.000      Overland Slope"
"      1.597      Pervious Area"
"      140.000      Pervious length"
"      2.000      Pervious slope"
"      1.952      Impervious Area"
"      140.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"

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```

"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.371      1.479      0.000      0.000 c.m/sec"
"      Catchment 303      Pervious      Impervious Total Area "
"      Surface Area      1.597      1.952      3.550      hectare"
"      Time of concentration 40.556      7.207      17.519      minutes"
"      Time to Centroid      2744.076      2245.793      2399.878      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      0.4553      0.5565      1.0117      ha-m"
"      Rainfall losses      141.525      22.773      76.211      mm"
"      Runoff depth      143.475      262.227      208.789      mm"
"      Runoff volume      2292.02      5119.98      7412.00      c.m"
"      Runoff coefficient      0.503      0.920      0.733      "
"      Maximum flow      0.175      0.247      0.371      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.371      1.850      0.000      0.000"
" 33      CATCHMENT 204"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      204      Park"
"      10.000  % Impervious"
"      2.090  Total Area"
"      140.000 Flow length"
"      2.000  Overland Slope"
"      1.881  Pervious Area"
"      140.000 Pervious length"
"      2.000  Pervious slope"
"      0.209  Impervious Area"
"      140.000 Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.225      1.850      0.000      0.000 c.m/sec"
"      Catchment 204      Pervious      Impervious Total Area "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration 40.556      7.207      34.926      minutes"
"      Time to Centroid      2744.076      2245.793      2659.968      minutes"

```

"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	5360.85	595.65	5956.50	c.m"
"	Rainfall losses	141.525	22.773	129.649	mm"
"	Runoff depth	143.475	262.227	155.351	mm"
"	Runoff volume	2698.77	548.05	3246.83	c.m"
"	Runoff coefficient	0.503	0.920	0.545	"
"	Maximum flow	0.206	0.026	0.225	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.225 2.022 0.000 0.000"				
" 33	CATCHMENT 301"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	301 Scott Street External"				
"	55.000 % Impervious"				
"	2.600 Total Area"				
"	95.000 Flow length"				
"	2.000 Overland Slope"				
"	1.170 Pervious Area"				
"	95.000 Pervious length"				
"	2.000 Pervious slope"				
"	1.430 Impervious Area"				
"	95.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"	0.282 2.022 0.000 0.000 c.m/sec"				
"	Catchment 301 Pervious Impervious Total Area "				
"	Surface Area 1.170 1.430 2.600 hectare"				
"	Time of concentration 32.137 5.711 14.115 minutes"				
"	Time to Centroid 2737.280 2232.538 2393.050 minutes"				
"	Rainfall depth 285.000 285.000 285.000 mm"				
"	Rainfall volume 3334.50 4075.50 7410.00 c.m"				
"	Rainfall losses 139.558 29.801 79.192 mm"				
"	Runoff depth 145.442 255.199 205.808 mm"				
"	Runoff volume 1701.67 3649.34 5351.01 c.m"				
"	Runoff coefficient 0.510 0.895 0.722 "				
"	Maximum flow 0.128 0.181 0.282 c.m/sec"				
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.282 2.304 0.000 0.000"				

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" 33      CATCHMENT 205"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          205 Internal to Pond Inet 1"
"          65.000 % Impervious"
"          4.320 Total Area"
"         100.000 Flow length"
"          4.000 Overland Slope"
"          1.512 Pervious Area"
"         100.000 Pervious length"
"          2.000 Pervious slope"
"          2.808 Impervious Area"
"         100.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"          0.485      2.304      0.000      0.000 c.m/sec"
"          Catchment 205      Pervious      Impervious Total Area "
"          Surface Area      1.512      2.808      4.320      hectare"
"          Time of concentration 33.142      5.889      12.266      minutes"
"          Time to Centroid 2737.930      2233.505      2351.531      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      0.4309      0.8003      1.2312      ha-m"
"          Rainfall losses      139.790      29.018      67.788      mm"
"          Runoff depth      145.210      255.982      217.212      mm"
"          Runoff volume      2195.58      7187.97      9383.55      c.m"
"          Runoff coefficient      0.510      0.898      0.762      "
"          Maximum flow      0.166      0.355      0.485      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.485      2.789      0.000      0.000"
" 33      CATCHMENT 206"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          206 Pond"
"          80.000 % Impervious"
"          3.290 Total Area"
"         100.000 Flow length"
"          4.000 Overland Slope"
"          0.658 Pervious Area"

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```

"      100.000  Pervious length"
"      2.000   Pervious slope"
"      2.632   Impervious Area"
"     100.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"     75.000   Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"
"      0.015   Impervious Manning 'n'"
"      0.000   Impervious Max.infiltration"
"      0.000   Impervious Min.infiltration"
"      0.050   Impervious Lag constant (hours)"
"      1.500   Impervious Depression storage"
"           0.389   2.789   0.000   0.000 c.m/sec"
"      Catchment 206      Pervious  Impervious Total Area  "
"      Surface Area      0.658     2.632     3.290  hectare"
"      Time of concentration 33.142   5.889     9.274  minutes"
"      Time to Centroid    2737.929 2233.505 2296.156 minutes"
"      Rainfall depth     285.000  285.000  285.000 mm"
"      Rainfall volume    1875.30  7501.20  9376.50 c.m"
"      Rainfall losses    139.790  29.018   51.172 mm"
"      Runoff depth       145.210  255.982  233.828 mm"
"      Runoff volume      955.48  6737.44  7692.93 c.m"
"      Runoff coefficient  0.510   0.898   0.820  "
"      Maximum flow      0.072   0.333   0.389  c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"           0.389   3.178   0.000   0.000"
" 33  CATCHMENT 200"
"      1  Triangular SCS"
"      1  Equal length"
"      2  Horton equation"
"      200 Rear yard and Open Space to Grand River"
"     10.000 % Impervious"
"      7.530 Total Area"
"     100.000 Flow length"
"      5.000 Overland Slope"
"      6.777 Pervious Area"
"     100.000 Pervious length"
"      2.000 Pervious slope"
"      0.753 Impervious Area"
"     100.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"

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"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.809      3.178      0.000      0.000 c.m/sec"
"      Catchment 200      Pervious      Impervious Total Area "
"      Surface Area      6.777      0.753      7.530      hectare"
"      Time of concentration 33.142      5.889      28.678      minutes"
"      Time to Centroid      2737.929      2233.505      2655.311      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1.9314      0.2146      2.1461      ha-m"
"      Rainfall losses      139.790      29.018      128.713      mm"
"      Runoff depth      145.210      255.982      156.287      mm"
"      Runoff volume      0.9841      0.1928      1.1768      ha-m"
"      Runoff coefficient      0.510      0.898      0.548      "
"      Maximum flow      0.744      0.095      0.809      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.809      3.853      0.000      0.000"
" 38      START/RE-START TOTALS 200"
"      3      Runoff Totals on EXIT"
"      Total Catchment area      36.330      hectare"
"      Total Impervious area      18.751      hectare"
"      Total % impervious      51.612"
" 19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                      25mm.out"
"          Licensee name:                        gmbp"
"          Company                               "
"          Date & Time last used:                2/22/2023 at 10:32:47 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          240.000  Max. Storm length"
"          3600.000  Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1581.200  Coefficient A"
"          13.000  Constant B"
"          1.000  Exponent C"
"          0.400  Fraction R"
"          240.000  Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    84.723  mm/hr"
"          Total depth                          24.999  mm"
"          8  00025hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302  External lands and water tower"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.074  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	"
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	---	2.370	2.370	minutes"
"	Time to Centroid	0.000	110.927	110.927	minutes"
"	Rainfall depth	24.999	24.999	24.999	mm"
"	Rainfall volume	75.37	92.12	167.49	c.m"
"	Rainfall losses	24.999	1.866	12.276	mm"
"	Runoff depth	0.000	23.134	12.724	mm"
"	Runoff volume	0.00	85.25	85.25	c.m"
"	Runoff coefficient	0.000	0.925	0.509	"
"	Maximum flow	0.000	0.074	0.074	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

" 4 Add Runoff "

" 0.074 0.074 0.000 0.000"

" 33 CATCHMENT 201"

" 1 Triangular SCS"

" 1 Equal length"

" 2 Horton equation"

" 201 Townhome and Apartment Blocks"

" 90.000 % Impervious"

" 4.280 Total Area"

" 200.000 Flow length"

" 4.000 Overland Slope"

" 0.428 Pervious Area"

" 200.000 Pervious length"

" 2.000 Pervious slope"

" 3.852 Impervious Area"

" 200.000 Impervious length"

" 2.000 Impervious slope"

" 0.250 Pervious Manning 'n'"

" 75.000 Pervious Max.infiltration"

" 5.000 Pervious Min.infiltration"

" 0.250 Pervious Lag constant (hours)"

" 5.000 Pervious Depression storage"

" 0.015 Impervious Manning 'n'"

" 0.000 Impervious Max.infiltration"

" 0.000 Impervious Min.infiltration"

" 0.050 Impervious Lag constant (hours)"

" 1.500 Impervious Depression storage"

" 0.742 0.074 0.000 0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	---	7.399	7.399	minutes"
"	Time to Centroid	0.000	117.918	117.918	minutes"
"	Rainfall depth	24.999	24.999	24.999	mm"
"	Rainfall volume	107.00	962.97	1069.97	c.m"
"	Rainfall losses	24.999	1.678	4.010	mm"
"	Runoff depth	0.000	23.321	20.989	mm"
"	Runoff volume	0.00	898.33	898.33	c.m"
"	Runoff coefficient	0.000	0.933	0.840	"

"		Maximum flow	0.000	0.742	0.742	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.742	0.797	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.112	0.797	0.000	0.000 c.m/sec"	
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	---	5.169	5.169	minutes"
"		Time to Centroid	0.000	114.879	114.879	minutes"
"		Rainfall depth	24.999	24.999	24.999	mm"
"		Rainfall volume	122.62	149.87	272.49	c.m"
"		Rainfall losses	24.999	1.698	12.183	mm"
"		Runoff depth	0.000	23.302	12.816	mm"
"		Runoff volume	0.00	139.69	139.69	c.m"
"		Runoff coefficient	0.000	0.932	0.513	"
"		Maximum flow	0.000	0.112	0.112	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.112	0.909	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

```

"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.467   0.909   0.000   0.000 c.m/sec"
"      Catchment 202      Pervious      Impervious      Total Area  "
"      Surface Area      1.660      2.490      4.150      hectare"
"      Time of concentration      ---      5.714      5.714      minutes"
"      Time to Centroid      0.000      115.659      115.659      minutes"
"      Rainfall depth      24.999      24.999      24.999      mm"
"      Rainfall volume      414.99      622.48      1037.47      c.m"
"      Rainfall losses      24.999      1.646      10.987      mm"
"      Runoff depth      0.000      23.353      14.012      mm"
"      Runoff volume      0.00      581.50      581.50      c.m"
"      Runoff coefficient      0.000      0.934      0.560      "
"      Maximum flow      0.000      0.467      0.467      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.467   1.375   0.000   0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.467   1.375   1.375   0.000"
" 40  HYDROGRAPH Combine 3"
"      6  Combine "
"      3  Node #"
"      Pond Inlet 3"
"      Maximum flow      1.375      c.m/sec"
"      Hydrograph volume      1704.772      c.m"
"          0.467   1.375   1.375   1.375"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.467   0.000   1.375   1.375"
" 33  CATCHMENT 203"
"      1  Triangular SCS"
"      1  Equal length"

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"          2 Horton equation"
"          203 Internal to Pond Inet 2"
" 60.000 % Impervious"
"          2.760 Total Area"
" 30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"
" 30.000 Pervious length"
"          2.000 Pervious slope"
"          1.656 Impervious Area"
" 30.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
" 75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.334      0.000      1.375      1.375 c.m/sec"
"          Catchment 203      Pervious      Impervious Total Area "
"          Surface Area      1.104      1.656      2.760      hectare"
"          Time of concentration      ---      2.370      2.370      minutes"
"          Time to Centroid      0.000      110.927      110.927      minutes"
"          Rainfall depth      24.999      24.999      24.999      mm"
"          Rainfall volume      275.99      413.99      689.98      c.m"
"          Rainfall losses      24.999      1.866      11.119      mm"
"          Runoff depth      0.000      23.134      13.880      mm"
"          Runoff volume      0.00      383.09      383.09      c.m"
"          Runoff coefficient      0.000      0.925      0.555      "
"          Maximum flow      0.000      0.334      0.334      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"              0.334      0.334      1.375      1.375"
" 40 HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"              0.334      0.334      0.334      1.375"
" 40 HYDROGRAPH Combine 2"
"          6 Combine "
"          2 Node #"
"          Pond Inlet 2"
"          Maximum flow      0.334      c.m/sec"
"          Hydrograph volume      383.093      c.m"
"              0.334      0.334      0.334      0.334"
" 40 HYDROGRAPH Start - New Tributary"
"          2 Start - New Tributary"
"              0.334      0.000      0.334      0.334"

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" 33      CATCHMENT 303"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          303 External"
"          55.000 % Impervious"
"          3.550 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.597 Pervious Area"
"         140.000 Pervious length"
"          2.000 Pervious slope"
"          1.952 Impervious Area"
"         140.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"                0.366      0.000      0.334      0.334 c.m/sec"
"          Catchment 303      Pervious      Impervious      Total Area  "
"          Surface Area      1.597      1.952      3.550      hectare"
"          Time of concentration      ---      5.974      5.974      minutes"
"          Time to Centroid      0.000      116.025      116.025      minutes"
"          Rainfall depth      24.999      24.999      24.999      mm"
"          Rainfall volume      399.36      488.11      887.47      c.m"
"          Rainfall losses      24.999      1.621      12.141      mm"
"          Runoff depth      0.000      23.379      12.858      mm"
"          Runoff volume      0.00      456.47      456.47      c.m"
"          Runoff coefficient      0.000      0.935      0.514      "
"          Maximum flow      0.000      0.366      0.366      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"                0.366      0.366      0.334      0.334"
" 33      CATCHMENT 204"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          204 Park"
"          10.000 % Impervious"
"          2.090 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.881 Pervious Area"

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"      140.000  Pervious length"
"      2.000   Pervious slope"
"      0.209   Impervious Area"
"      140.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"      75.000   Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"
"      0.015   Impervious Manning 'n'"
"      0.000   Impervious Max.infiltration"
"      0.000   Impervious Min.infiltration"
"      0.050   Impervious Lag constant (hours)"
"      1.500   Impervious Depression storage"
"              0.039      0.366      0.334      0.334 c.m/sec"
"      Catchment 204      Pervious      Impervious Total Area  "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration      ---      5.974      5.974      minutes"
"      Time to Centroid      0.000      116.025      116.025      minutes"
"      Rainfall depth      24.999      24.999      24.999      mm"
"      Rainfall volume      470.24      52.25      522.48      c.m"
"      Rainfall losses      24.999      1.621      22.661      mm"
"      Runoff depth      0.000      23.379      2.338      mm"
"      Runoff volume      0.00      48.86      48.86      c.m"
"      Runoff coefficient      0.000      0.935      0.094      "
"      Maximum flow      0.000      0.039      0.039      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"              0.039      0.405      0.334      0.334"
" 33      CATCHMENT 301"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      301   Scott Street External"
"      55.000  % Impervious"
"      2.600   Total Area"
"      95.000  Flow length"
"      2.000   Overland Slope"
"      1.170   Pervious Area"
"      95.000  Pervious length"
"      2.000   Pervious slope"
"      1.430   Impervious Area"
"      95.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"      75.000  Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"

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"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.262      0.405      0.334      0.334 c.m/sec"
"      Catchment 301      Pervious      Impervious Total Area "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration      ---      4.734      4.734      minutes"
"      Time to Centroid      0.000      114.243      114.243      minutes"
"      Rainfall depth      24.999      24.999      24.999      mm"
"      Rainfall volume      292.49      357.49      649.98      c.m"
"      Rainfall losses      24.999      1.677      12.172      mm"
"      Runoff depth      0.000      23.322      12.827      mm"
"      Runoff volume      0.00      333.50      333.50      c.m"
"      Runoff coefficient      0.000      0.933      0.513      "
"      Maximum flow      0.000      0.262      0.262      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.262      0.667      0.334      0.334"
" 33      CATCHMENT 205"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      205      Internal to Pond Inet 1"
"      65.000      % Impervious"
"      4.320      Total Area"
"      100.000      Flow length"
"      4.000      Overland Slope"
"      1.512      Pervious Area"
"      100.000      Pervious length"
"      2.000      Pervious slope"
"      2.808      Impervious Area"
"      100.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"
"      0.015      Impervious Manning 'n'"
"      0.000      Impervious Max.infiltration"
"      0.000      Impervious Min.infiltration"
"      0.050      Impervious Lag constant (hours)"
"      1.500      Impervious Depression storage"
"              0.518      0.667      0.334      0.334 c.m/sec"
"      Catchment 205      Pervious      Impervious Total Area "
"      Surface Area      1.512      2.808      4.320      hectare"
"      Time of concentration      ---      4.882      4.882      minutes"
"      Time to Centroid      0.000      114.456      114.456      minutes"

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"	Rainfall depth	24.999	24.999	24.999	mm"
"	Rainfall volume	377.99	701.98	1079.97	c.m"
"	Rainfall losses	24.999	1.695	9.851	mm"
"	Runoff depth	0.000	23.304	15.148	mm"
"	Runoff volume	0.00	654.38	654.38	c.m"
"	Runoff coefficient	0.000	0.932	0.606	"
"	Maximum flow	0.000	0.518	0.518	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.518 1.185 0.334 0.334"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.518 1.185 1.185 0.334"				
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		1.185		c.m/sec"
"	Hydrograph volume		1493.211		c.m"
"	0.518 1.185 1.185 1.185"				
" 40	HYDROGRAPH Confluence 1"				
"	7 Confluence "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		1.185		c.m/sec"
"	Hydrograph volume		1493.211		c.m"
"	0.518 1.185 1.185 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.518 1.185 1.185 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		1.185		c.m/sec"
"	Hydrograph volume		1493.211		c.m"
"	0.518 1.185 1.185 1.185"				
" 40	HYDROGRAPH Confluence 3"				
"	7 Confluence "				
"	3 Node #"				
"	Pond Inlet 3"				
"	Maximum flow		1.375		c.m/sec"
"	Hydrograph volume		1704.772		c.m"
"	0.518 1.375 1.185 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.518 1.375 1.375 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				

"	Pond"				
"	Maximum flow		2.561	c.m/sec"	
"	Hydrograph volume		3197.983	c.m"	
"	0.518	1.375	1.375	2.561"	
" 40	HYDROGRAPH Confluence		2"		
"	7 Confluence "				
"	2 Node #"				
"	Pond Inlet 2"				
"	Maximum flow		0.334	c.m/sec"	
"	Hydrograph volume		383.093	c.m"	
"	0.518	0.334	1.375	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.518	0.334	0.334	0.000"	
" 40	HYDROGRAPH Combine		1000"		
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		2.808	c.m/sec"	
"	Hydrograph volume		3581.077	c.m"	
"	0.518	0.334	0.334	2.808"	
" 40	HYDROGRAPH Confluence		1000"		
"	7 Confluence "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		2.808	c.m/sec"	
"	Hydrograph volume		3581.077	c.m"	
"	0.518	2.808	0.334	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.518	2.808	2.808	0.000"	
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.518	2.808	2.808	0.000"	
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	206 Pond"				
"	80.000 % Impervious"				
"	3.290 Total Area"				
"	100.000 Flow length"				
"	4.000 Overland Slope"				
"	0.658 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	2.632 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				

```

"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.486      2.808      2.808      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration      ---      4.882      4.882      minutes"
"      Time to Centroid      0.000      114.456      114.456      minutes"
"      Rainfall depth      24.999      24.999      24.999      mm"
"      Rainfall volume      164.49      657.98      822.47      c.m"
"      Rainfall losses      24.999      1.695      6.356      mm"
"      Runoff depth      0.000      23.304      18.643      mm"
"      Runoff volume      0.00      613.37      613.37      c.m"
"      Runoff coefficient      0.000      0.932      0.746      "
"      Maximum flow      0.000      0.486      0.486      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.486      3.294      2.808      0.000"
" 54      POND DESIGN"
"      3.294      Current peak flow      c.m/sec"
"      0.206      Target outflow      c.m/sec"
"      4194.4      Hydrograph volume      c.m"
"      17.      Number of stages"
"      456.800      Minimum water level      metre"
"      458.000      Maximum water level      metre"
"      456.800      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      456.000      0.000      0.000"
"      456.100      0.01500      680.300"
"      456.200      0.02900      1395.900"
"      456.300      0.04200      2147.500"
"      456.400      0.05200      2935.400"
"      456.480      0.05900      3592.400"
"      456.500      0.06000      3760.400"
"      456.600      0.2840      4627.400"
"      456.700      0.7630      5539.800"
"      456.800      1.491      6496.900"
"      456.900      2.386      7500.300"
"      457.000      2.490      8551.600"
"      457.100      6.036      9652.000"
"      457.200      12.632      10803.10"
"      457.300      21.379      12006.50"
"      457.400      31.960      13275.50"

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"          457.500    44.189  14711.10"
"      Peak outflow                0.060    c.m/sec"
"      Maximum level                456.499    metre"
"      Maximum storage              3748.784    c.m"
"      Centroidal lag               15.759    hours"
"          0.486    3.294    0.060    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.486    0.060    0.060    0.000"
" 54      POND DESIGN"
"          0.060 Current peak flow    c.m/sec"
"          0.206 Target outflow    c.m/sec"
"          4141.6 Hydrograph volume    c.m"
"          9. Number of stages"
"          456.800 Minimum water level    metre"
"          458.000 Maximum water level    metre"
"          456.800 Starting water level    metre"
"          0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          454.860    0.000    0.000"
"          454.960  1.20E-05    4.000"
"          455.060  1.20E-05    8.000"
"          455.160  1.20E-05   12.000"
"          455.260  1.20E-05   16.000"
"          455.360  1.20E-05   20.000"
"          455.460    1.702   20.500"
"          455.560    4.979   21.000"
"          455.660    9.395   21.500"
"      Peak outflow                0.060    c.m/sec"
"      Maximum level                455.363    metre"
"      Maximum storage              20.018    c.m"
"      Centroidal lag               16.043    hours"
"          0.486    0.060    0.060    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.486    0.060    0.060    0.000"
" 56      DIVERSION"
"          3000 Node number"
"          0.000 Overflow threshold"
"          0.399 Computed diverted fraction"
"          0 Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow    0.024    c.m/sec"
"          Volume of diverted flow  1625.398    c.m"
"          DIV03000.00025hyd"
"          Major flow at 3000"
"          0.486    0.060    0.036    0.000 c.m/sec"
" 81      ADD COMMENT=====
"          1 Lines of comment"
"          Diverted to existing wetland on site"
" 40      HYDROGRAPH Combine 4000"

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"          6  Combine "
"      4000  Node #"
"          Site"
"          Maximum flow          0.036  c.m/sec"
"          Hydrograph volume      2497.057  c.m"
"              0.486  0.060  0.036  0.036"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.486  0.000  0.036  0.036"
" 33      CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200 Rear yard and Open Space to Grand River"
"      10.000 % Impervious"
"          7.530 Total Area"
"      100.000 Flow length"
"          5.000 Overland Slope"
"          6.777 Pervious Area"
"      100.000 Pervious length"
"          2.000 Pervious slope"
"          0.753 Impervious Area"
"      100.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.139  0.000  0.036  0.036 c.m/sec"
"          Catchment 200      Pervious  Impervious Total Area "
"          Surface Area      6.777  0.753  7.530  hectare"
"          Time of concentration  ---  4.882  4.882  minutes"
"          Time to Centroid  0.000  114.456  114.456  minutes"
"          Rainfall depth    24.999  24.999  24.999  mm"
"          Rainfall volume    1694.20  188.24  1882.44  c.m"
"          Rainfall losses    24.999  1.695  22.669  mm"
"          Runoff depth       0.000  23.304  2.330  mm"
"          Runoff volume      0.00  175.48  175.48  c.m"
"          Runoff coefficient  0.000  0.932  0.093  "
"          Maximum flow      0.000  0.139  0.139  c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.139  0.139  0.036  0.036"
" 40      HYDROGRAPH Copy to Outflow"

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"      8  Copy to Outflow"
"      0.139    0.139    0.139    0.036"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          0.139    c.m/sec"
"      Hydrograph volume     2672.536  c.m"
"      0.139    0.139    0.139    0.139"
" 40    HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.139    0.000    0.139    0.139"
" 47    FILEI_0 Read/Open DIV03000.00025hyd"
"      1  1=read/open; 2=write/save"
"      2  1=rainfall; 2=hydrograph"
"      1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03000.00025hyd"
"      Major flow at 3000"
"      Total volume          1625.398  c.m"
"      Maximum flow          0.024    c.m/sec"
"      0.024    0.000    0.139    0.139 c.m/sec"
" 40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.024    0.024    0.139    0.139"
" 40    HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.024    0.024    0.024    0.139"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          0.139    c.m/sec"
"      Hydrograph volume     4297.937  c.m"
"      0.024    0.024    0.024    0.139"
" 40    HYDROGRAPH Confluence 4000"
"      7  Confluence "
" 4000  Node #"
"      Site"
"      Maximum flow          0.139    c.m/sec"
"      Hydrograph volume     4297.937  c.m"
"      0.024    0.139    0.024    0.000"
" 81    ADD COMMENT=====
"      1  Lines of comment"
"      Total flow to Grand River"
" 38    START/RE-START TOTALS 4000"
"      3  Runoff Totals on EXIT"
"      Total Catchment area          36.330  hectare"
"      Total Impervious area         18.751  hectare"
"      Total % impervious            51.612"
" 19    EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 post  002 year.out"
"          Licensee name:                      gmbp"
"          Company                            "
"          Date & Time last used:              2/22/2023 at 10:44:38 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          695.050 Coefficient A"
"          6.387  Constant B"
"          0.793  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    93.293  mm/hr"
"          Total depth                          33.014  mm"
"          6  002hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302 External lands and water tower"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.072  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	"
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	22.062	2.281	3.430	minutes"
"	Time to Centroid	96.472	86.938	87.492	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	99.54	121.66	221.19	c.m"
"	Rainfall losses	30.676	2.000	14.904	mm"
"	Runoff depth	2.338	31.014	18.110	mm"
"	Runoff volume	7.05	114.29	121.33	c.m"
"	Runoff coefficient	0.071	0.939	0.549	"
"	Maximum flow	0.004	0.072	0.072	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

"	4	Add Runoff "			
"		0.072	0.072	0.000	0.000"

" 33 CATCHMENT 201"

"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		0.783	0.072	0.000	0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	"
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	68.865	7.119	7.629	minutes"
"	Time to Centroid	138.360	93.923	94.289	minutes"
"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	141.30	1271.70	1413.00	c.m"
"	Rainfall losses	30.669	1.681	4.580	mm"
"	Runoff depth	2.345	31.333	28.434	mm"
"	Runoff volume	10.04	1206.95	1216.99	c.m"
"	Runoff coefficient	0.071	0.949	0.861	"

"		Maximum flow	0.002	0.782	0.783	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.783	0.855	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.125	0.855	0.000	0.000 c.m/sec"	
"		Catchment 304	Pervious	Impervious	Total Area "	
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	48.108	4.973	7.458	minutes"
"		Time to Centroid	119.816	90.843	92.512	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	161.93	197.92	359.85	c.m"
"		Rainfall losses	30.669	1.629	14.697	mm"
"		Runoff depth	2.345	31.385	18.317	mm"
"		Runoff volume	11.50	188.15	199.65	c.m"
"		Runoff coefficient	0.071	0.951	0.555	"
"		Maximum flow	0.004	0.124	0.125	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.125	0.979	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

```

"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.511    0.979    0.000    0.000 c.m/sec"
"      Catchment 202      Pervious      Impervious      Total Area  "
"      Surface Area      1.660      2.490      4.150      hectare"
"      Time of concentration  53.180      5.498      7.765      minutes"
"      Time to Centroid      124.334      91.560      93.119      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      548.03      822.05      1370.08      c.m"
"      Rainfall losses      30.670      1.714      13.296      mm"
"      Runoff depth      2.344      31.300      19.718      mm"
"      Runoff volume      38.92      779.37      818.28      c.m"
"      Runoff coefficient      0.071      0.948      0.597      "
"      Maximum flow      0.011      0.510      0.511      c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.511    1.490    0.000    0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.511    1.490    1.490    0.000"
" 40  HYDROGRAPH  Combine  3"
"      6  Combine "
"      3  Node #"
"      Pond Inlet 3"
"      Maximum flow      1.490      c.m/sec"
"      Hydrograph volume      2356.259      c.m"
"          0.511    1.490    1.490    1.490"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.511    0.000    1.490    1.490"
" 33  CATCHMENT 203"
"      1  Triangular SCS"
"      1  Equal length"

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"          2 Horton equation"
"          203 Internal to Pond Inet 2"
" 60.000 % Impervious"
"          2.760 Total Area"
" 30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"
" 30.000 Pervious length"
"          2.000 Pervious slope"
"          1.656 Impervious Area"
" 30.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
" 75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.323      0.000      1.490      1.490 c.m/sec"
"          Catchment 203      Pervious      Impervious Total Area "
"          Surface Area      1.104      1.656      2.760      hectare"
"          Time of concentration 22.062      2.281      3.227      minutes"
"          Time to Centroid 96.472      86.938      87.394      minutes"
"          Rainfall depth 33.014      33.014      33.014      mm"
"          Rainfall volume 364.47      546.71      911.19      c.m"
"          Rainfall losses 30.676      2.000      13.471      mm"
"          Runoff depth 2.338      31.014      19.543      mm"
"          Runoff volume 25.81      513.59      539.40      c.m"
"          Runoff coefficient 0.071      0.939      0.592      "
"          Maximum flow 0.016      0.322      0.323      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"              0.323      0.323      1.490      1.490"
" 40 HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"              0.323      0.323      0.323      1.490"
" 40 HYDROGRAPH Combine 2"
"          6 Combine "
"          2 Node #"
"          Pond Inlet 2"
"          Maximum flow      0.323      c.m/sec"
"          Hydrograph volume 539.399      c.m"
"              0.323      0.323      0.323      0.323"
" 40 HYDROGRAPH Start - New Tributary"
"          2 Start - New Tributary"
"              0.323      0.000      0.323      0.323"

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" 33      CATCHMENT 303"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          303 External"
"          55.000 % Impervious"
"          3.550 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.597 Pervious Area"
"         140.000 Pervious length"
"          2.000 Pervious slope"
"          1.952 Impervious Area"
"         140.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"          0.397      0.000      0.323      0.323 c.m/sec"
"          Catchment 303      Pervious      Impervious Total Area "
"          Surface Area      1.597      1.952      3.550      hectare"
"          Time of concentration 55.598      5.748      8.629      minutes"
"          Time to Centroid      126.518      91.941      93.939      minutes"
"          Rainfall depth      33.014      33.014      33.014      mm"
"          Rainfall volume      527.40      644.60      1172.00      c.m"
"          Rainfall losses      30.669      1.730      14.753      mm"
"          Runoff depth      2.345      31.284      18.261      mm"
"          Runoff volume      37.47      610.81      648.28      c.m"
"          Runoff coefficient      0.071      0.948      0.553      "
"          Maximum flow      0.011      0.397      0.397      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.397      0.397      0.323      0.323"
" 33      CATCHMENT 204"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          204 Park"
"          10.000 % Impervious"
"          2.090 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.881 Pervious Area"

```

```

"      140.000 Pervious length"
"      2.000 Pervious slope"
"      0.209 Impervious Area"
"      140.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          0.043      0.397      0.323      0.323 c.m/sec"
"      Catchment 204      Pervious      Impervious Total Area "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration 55.598      5.748      25.832      minutes"
"      Time to Centroid      126.517      91.941      105.871      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      620.99      69.00      689.99      c.m"
"      Rainfall losses      30.669      1.730      27.775      mm"
"      Runoff depth      2.345      31.284      5.239      mm"
"      Runoff volume      44.12      65.38      109.50      c.m"
"      Runoff coefficient      0.071      0.948      0.159      "
"      Maximum flow      0.013      0.042      0.043      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          0.043      0.441      0.323      0.323"
" 33 CATCHMENT 301"
"      1 Triangular SCS"
"      1 Equal length"
"      2 Horton equation"
"      301 Scott Street External"
"      55.000 % Impervious"
"      2.600 Total Area"
"      95.000 Flow length"
"      2.000 Overland Slope"
"      1.170 Pervious Area"
"      95.000 Pervious length"
"      2.000 Pervious slope"
"      1.430 Impervious Area"
"      95.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"

```

"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.300	0.441	0.323	0.323 c.m/sec"	
"		Catchment 301	Pervious	Impervious	Total Area	"
"		Surface Area	1.170	1.430	2.600	hectare"
"		Time of concentration	44.057	4.555	6.834	minutes"
"		Time to Centroid	116.160	90.239	91.735	minutes"
"		Rainfall depth	33.014	33.014	33.014	mm"
"		Rainfall volume	386.26	472.10	858.36	c.m"
"		Rainfall losses	30.671	1.703	14.738	mm"
"		Runoff depth	2.343	31.311	18.276	mm"
"		Runoff volume	27.41	447.75	475.16	c.m"
"		Runoff coefficient	0.071	0.948	0.554	"
"		Maximum flow	0.009	0.299	0.300	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.300	0.741	0.323	0.323"	
" 33		CATCHMENT 205"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	205	Internal to Pond Inet 1"				
"	65.000	% Impervious"				
"	4.320	Total Area"				
"	100.000	Flow length"				
"	4.000	Overland Slope"				
"	1.512	Pervious Area"				
"	100.000	Pervious length"				
"	2.000	Pervious slope"				
"	2.808	Impervious Area"				
"	100.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.587	0.741	0.323	0.323 c.m/sec"	
"		Catchment 205	Pervious	Impervious	Total Area	"
"		Surface Area	1.512	2.808	4.320	hectare"
"		Time of concentration	45.434	4.697	6.274	minutes"
"		Time to Centroid	117.422	90.442	91.487	minutes"

"	Rainfall depth	33.014	33.014	33.014	mm"
"	Rainfall volume	499.17	927.03	1426.20	c.m"
"	Rainfall losses	30.669	1.666	11.817	mm"
"	Runoff depth	2.345	31.348	21.197	mm"
"	Runoff volume	35.46	880.24	915.70	c.m"
"	Runoff coefficient	0.071	0.950	0.642	"
"	Maximum flow	0.012	0.586	0.587	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.587 1.328 0.323 0.323"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.587 1.328 1.328 0.323"				
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		1.328		c.m/sec"
"	Hydrograph volume		2148.645		c.m"
"	0.587 1.328 1.328 1.328"				
" 40	HYDROGRAPH Confluence 1"				
"	7 Confluence "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		1.328		c.m/sec"
"	Hydrograph volume		2148.645		c.m"
"	0.587 1.328 1.328 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.587 1.328 1.328 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		1.328		c.m/sec"
"	Hydrograph volume		2148.645		c.m"
"	0.587 1.328 1.328 1.328"				
" 40	HYDROGRAPH Confluence 3"				
"	7 Confluence "				
"	3 Node #"				
"	Pond Inlet 3"				
"	Maximum flow		1.490		c.m/sec"
"	Hydrograph volume		2356.259		c.m"
"	0.587 1.490 1.328 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.587 1.490 1.490 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				

"		Pond"			
"		Maximum flow	2.818	c.m/sec"	
"		Hydrograph volume	4504.905	c.m"	
"		0.587 1.490	1.490	2.818"	
" 40		HYDROGRAPH Confluence	2"		
"		7 Confluence "			
"		2 Node #"			
"		Pond Inlet 2"			
"		Maximum flow	0.323	c.m/sec"	
"		Hydrograph volume	539.399	c.m"	
"		0.587 0.323	1.490	0.000"	
" 40		HYDROGRAPH Copy to Outflow"			
"		8 Copy to Outflow"			
"		0.587 0.323	0.323	0.000"	
" 40		HYDROGRAPH Combine 1000"			
"		6 Combine "			
"	1000	Node #"			
"		Pond"			
"		Maximum flow	3.141	c.m/sec"	
"		Hydrograph volume	5044.305	c.m"	
"		0.587 0.323	0.323	3.141"	
" 40		HYDROGRAPH Confluence	1000"		
"		7 Confluence "			
"	1000	Node #"			
"		Pond"			
"		Maximum flow	3.141	c.m/sec"	
"		Hydrograph volume	5044.305	c.m"	
"		0.587 3.141	0.323	0.000"	
" 40		HYDROGRAPH Copy to Outflow"			
"		8 Copy to Outflow"			
"		0.587 3.141	3.141	0.000"	
" 40		HYDROGRAPH Next link "			
"		5 Next link "			
"		0.587 3.141	3.141	0.000"	
" 33		CATCHMENT 206"			
"		1 Triangular SCS"			
"		1 Equal length"			
"		2 Horton equation"			
"		206 Pond"			
"	80.000	% Impervious"			
"	3.290	Total Area"			
"	100.000	Flow length"			
"	4.000	Overland Slope"			
"	0.658	Pervious Area"			
"	100.000	Pervious length"			
"	2.000	Pervious slope"			
"	2.632	Impervious Area"			
"	100.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			


```

"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.550      3.141      3.141      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration 45.434      4.697      5.445      minutes"
"      Time to Centroid      117.422      90.442      90.937      minutes"
"      Rainfall depth      33.014      33.014      33.014      mm"
"      Rainfall volume      217.23      868.93      1086.16      c.m"
"      Rainfall losses      30.669      1.666      7.467      mm"
"      Runoff depth      2.345      31.348      25.547      mm"
"      Runoff volume      15.43      825.07      840.50      c.m"
"      Runoff coefficient      0.071      0.950      0.774      "
"      Maximum flow      0.005      0.549      0.550      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.550      3.691      3.141      0.000"
" 54      POND DESIGN"
"      3.691      Current peak flow      c.m/sec"
"      0.206      Target outflow      c.m/sec"
"      5884.8      Hydrograph volume      c.m"
"      17.      Number of stages"
"      456.800      Minimum water level      metre"
"      458.000      Maximum water level      metre"
"      456.800      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      456.000      0.000      0.000"
"      456.100      0.01500      680.300"
"      456.200      0.02900      1395.900"
"      456.300      0.04200      2147.500"
"      456.400      0.05200      2935.400"
"      456.480      0.05900      3592.400"
"      456.500      0.06000      3760.400"
"      456.600      0.2840      4627.400"
"      456.700      0.7630      5539.800"
"      456.800      1.491      6496.900"
"      456.900      2.386      7500.300"
"      457.000      2.490      8551.600"
"      457.100      6.036      9652.000"
"      457.200      12.632      10803.10"
"      457.300      21.379      12006.50"
"      457.400      31.960      13275.50"

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"          457.500    44.189  14711.10"
"          Peak outflow                0.264    c.m/sec"
"          Maximum level                456.591    metre"
"          Maximum storage              4551.645    c.m"
"          Centroidal lag               12.538    hours"
"          0.550    3.691    0.264    0.000 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.550    0.264    0.264    0.000"
" 54    POND DESIGN"
"          0.264    Current peak flow    c.m/sec"
"          0.206    Target outflow    c.m/sec"
"          5827.4    Hydrograph volume    c.m"
"          9.    Number of stages"
"          456.800    Minimum water level    metre"
"          458.000    Maximum water level    metre"
"          456.800    Starting water level    metre"
"          0    Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          454.860    0.000    0.000"
"          454.960    1.20E-05    4.000"
"          455.060    1.20E-05    8.000"
"          455.160    1.20E-05    12.000"
"          455.260    1.20E-05    16.000"
"          455.360    1.20E-05    20.000"
"          455.460    1.702    20.500"
"          455.560    4.979    21.000"
"          455.660    9.395    21.500"
"          Peak outflow                0.264    c.m/sec"
"          Maximum level                455.376    metre"
"          Maximum storage              20.078    c.m"
"          Centroidal lag               12.813    hours"
"          0.550    0.264    0.264    0.000 c.m/sec"
" 40    HYDROGRAPH Next link "
"          5    Next link "
"          0.550    0.264    0.264    0.000"
" 56    DIVERSION"
"          3000    Node number"
"          0.000    Overflow threshold"
"          0.394    Computed diverted fraction"
"          0    Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow        0.104    c.m/sec"
"          Volume of diverted flow      2288.765    c.m"
"          DIV03000.002hyd"
"          Major flow at 3000"
"          0.550    0.264    0.160    0.000 c.m/sec"
" 81    ADD COMMENT=====
"          1    Lines of comment"
"          Diverted to existing wetland on site"
" 40    HYDROGRAPH    Combine    4000"

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"          6  Combine "
"      4000  Node #"
"          Site"
"          Maximum flow          0.160   c.m/sec"
"          Hydrograph volume      3513.095   c.m"
"              0.550   0.264   0.160   0.160"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.550   0.000   0.160   0.160"
" 33      CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200 Rear yard and Open Space to Grand River"
"          10.000 % Impervious"
"          7.530 Total Area"
"          100.000 Flow length"
"          5.000 Overland Slope"
"          6.777 Pervious Area"
"          100.000 Pervious length"
"          2.000 Pervious slope"
"          0.753 Impervious Area"
"          100.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.162   0.000   0.160   0.160 c.m/sec"
"          Catchment 200      Pervious      Impervious Total Area "
"          Surface Area      6.777      0.753      7.530      hectare"
"          Time of concentration 45.434      4.697      21.089      minutes"
"          Time to Centroid 117.422      90.442      101.298      minutes"
"          Rainfall depth      33.014      33.014      33.014      mm"
"          Rainfall volume      2237.36      248.60      2485.95      c.m"
"          Rainfall losses      30.669      1.666      27.769      mm"
"          Runoff depth      2.345      31.348      5.245      mm"
"          Runoff volume      158.93      236.05      394.98      c.m"
"          Runoff coefficient      0.071      0.950      0.159      "
"          Maximum flow      0.053      0.157      0.162      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.162   0.162   0.160   0.160"
" 40      HYDROGRAPH Copy to Outflow"

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"      8  Copy to Outflow"
"          0.162    0.162    0.162    0.160"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          0.205    c.m/sec"
"      Hydrograph volume     3908.075    c.m"
"          0.162    0.162    0.162    0.205"
" 40    HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.162    0.000    0.162    0.205"
" 47    FILEI_0 Read/Open DIV03000.002hyd"
"      1  1=read/open; 2=write/save"
"      2  1=rainfall; 2=hydrograph"
"      1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03000.002hyd"
"      Major flow at 3000"
"      Total volume          2288.765    c.m"
"      Maximum flow          0.104    c.m/sec"
"          0.104    0.000    0.162    0.205 c.m/sec"
" 40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.104    0.104    0.162    0.205"
" 40    HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.104    0.104    0.104    0.205"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          0.305    c.m/sec"
"      Hydrograph volume     6196.842    c.m"
"          0.104    0.104    0.104    0.305"
" 40    HYDROGRAPH Confluence 4000"
"      7  Confluence "
" 4000  Node #"
"      Site"
"      Maximum flow          0.305    c.m/sec"
"      Hydrograph volume     6196.842    c.m"
"          0.104    0.305    0.104    0.000"
" 81    ADD COMMENT=====
"      1  Lines of comment"
"      Total flow to Grand River"
" 38    START/RE-START TOTALS 4000"
"      3  Runoff Totals on EXIT"
"      Total Catchment area          36.330    hectare"
"      Total Impervious area         18.751    hectare"
"      Total % impervious            51.612"
" 19    EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                          C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                     104-104 post  005 year.out"
"          Licensee name:                       gmbp"
"          Company                              "
"          Date & Time last used:               2/22/2023 at 10:50:36 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          1459.072 Coefficient A"
"          13.690  Constant B"
"          0.850  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    113.586  mm/hr"
"          Total depth                          49.792  mm"
"          6  005hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302 External lands and water tower"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.104  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	14.420	2.108	4.886	minutes"
"	Time to Centroid	100.204	85.724	88.991	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	150.12	183.48	333.60	c.m"
"	Rainfall losses	32.852	2.224	16.007	mm"
"	Runoff depth	16.939	47.568	33.785	mm"
"	Runoff volume	51.07	175.29	226.36	c.m"
"	Runoff coefficient	0.340	0.955	0.679	"
"	Maximum flow	0.028	0.095	0.104	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

"	4	Add Runoff "			
"		0.104	0.104	0.000	0.000"

" 33 CATCHMENT 201"

"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		1.019	0.104	0.000	0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	45.009	6.580	8.036	minutes"
"	Time to Centroid	135.435	91.865	93.515	minutes"
"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	213.11	1917.98	2131.08	c.m"
"	Rainfall losses	32.839	1.944	5.034	mm"
"	Runoff depth	16.953	47.847	44.758	mm"
"	Runoff volume	72.56	1843.08	1915.64	c.m"
"	Runoff coefficient	0.340	0.961	0.899	"

"		Maximum flow	0.018	1.018	1.019	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		1.019	1.124	0.000	0.000"	
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.169	1.124	0.000	0.000 c.m/sec"	
"		Catchment 304	Pervious	Impervious	Total Area "	
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	31.443	4.597	10.620	minutes"
"		Time to Centroid	119.805	89.191	96.060	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	244.23	298.50	542.73	c.m"
"		Rainfall losses	32.853	1.876	15.816	mm"
"		Runoff depth	16.939	47.916	33.976	mm"
"		Runoff volume	83.09	287.25	370.34	c.m"
"		Runoff coefficient	0.340	0.962	0.682	"
"		Maximum flow	0.027	0.165	0.169	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.169	1.293	0.000	0.000"	
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

```

"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.695    1.293    0.000    0.000 c.m/sec"
"      Catchment 202      Pervious      Impervious      Total Area  "
"      Surface Area      1.660      2.490      4.150      hectare"
"      Time of concentration  34.758    5.082    10.733    minutes"
"      Time to Centroid      123.626  89.815    96.253    minutes"
"      Rainfall depth      49.792    49.792    49.792    mm"
"      Rainfall volume      826.54    1239.81    2066.36    c.m"
"      Rainfall losses      32.847    1.767    14.199    mm"
"      Runoff depth      16.945    48.025    35.593    mm"
"      Runoff volume      281.29    1195.82    1477.11    c.m"
"      Runoff coefficient    0.340    0.965    0.715    "
"      Maximum flow      0.083    0.684    0.695    c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          0.695    1.988    0.000    0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          0.695    1.988    1.988    0.000"
" 40  HYDROGRAPH  Combine  3"
"      6  Combine "
"      3  Node #"
"      Pond Inlet 3"
"      Maximum flow      1.988    c.m/sec"
"      Hydrograph volume    3989.447    c.m"
"          0.695    1.988    1.988    1.988"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          0.695    0.000    1.988    1.988"
" 33  CATCHMENT 203"
"      1  Triangular SCS"
"      1  Equal length"

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"          2 Horton equation"
"          203 Internal to Pond Inet 2"
" 60.000 % Impervious"
"          2.760 Total Area"
" 30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"
" 30.000 Pervious length"
"          2.000 Pervious slope"
"          1.656 Impervious Area"
" 30.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
" 75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"                0.462      0.000      1.988      1.988 c.m/sec"
" Catchment 203      Pervious      Impervious Total Area "
" Surface Area      1.104      1.656      2.760      hectare"
" Time of concentration 14.420      2.108      4.470      minutes"
" Time to Centroid 100.204      85.724      88.502      minutes"
" Rainfall depth      49.792      49.792      49.792      mm"
" Rainfall volume      549.70      824.55      1374.25      c.m"
" Rainfall losses      32.852      2.224      14.475      mm"
" Runoff depth      16.939      47.568      35.316      mm"
" Runoff volume      187.01      787.72      974.73      c.m"
" Runoff coefficient      0.340      0.955      0.709      "
" Maximum flow      0.103      0.429      0.462      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
" 4 Add Runoff "
"          0.462      0.462      1.988      1.988"
" 40 HYDROGRAPH Copy to Outflow"
" 8 Copy to Outflow"
"          0.462      0.462      0.462      1.988"
" 40 HYDROGRAPH Combine 2"
" 6 Combine "
" 2 Node #"
" Pond Inlet 2"
" Maximum flow      0.462      c.m/sec"
" Hydrograph volume      974.732      c.m"
"          0.462      0.462      0.462      0.462"
" 40 HYDROGRAPH Start - New Tributary"
" 2 Start - New Tributary"
"          0.462      0.000      0.462      0.462"

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" 33      CATCHMENT 303"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          303 External"
"          55.000 % Impervious"
"          3.550 Total Area"
"         140.000 Flow length"
"           2.000 Overland Slope"
"           1.597 Pervious Area"
"         140.000 Pervious length"
"           2.000 Pervious slope"
"           1.952 Impervious Area"
"         140.000 Impervious length"
"           2.000 Impervious slope"
"           0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"           5.000 Pervious Min.infiltration"
"           0.250 Pervious Lag constant (hours)"
"           5.000 Pervious Depression storage"
"           0.015 Impervious Manning 'n'"
"           0.000 Impervious Max.infiltration"
"           0.000 Impervious Min.infiltration"
"           0.050 Impervious Lag constant (hours)"
"           1.500 Impervious Depression storage"
"                0.544      0.000      0.462      0.462 c.m/sec"
"          Catchment 303      Pervious      Impervious      Total Area  "
"          Surface Area      1.597      1.952      3.550      hectare"
"          Time of concentration 36.338      5.313      12.264      minutes"
"          Time to Centroid      125.451      90.132      98.045      minutes"
"          Rainfall depth      49.792      49.792      49.792      mm"
"          Rainfall volume      795.42      972.18      1767.60      c.m"
"          Rainfall losses      32.839      1.757      15.744      mm"
"          Runoff depth      16.953      48.035      34.048      mm"
"          Runoff volume      270.82      937.88      1208.71      c.m"
"          Runoff coefficient      0.340      0.965      0.684      "
"          Maximum flow      0.078      0.534      0.544      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"                0.544      0.544      0.462      0.462"
" 33      CATCHMENT 204"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          204 Park"
"          10.000 % Impervious"
"           2.090 Total Area"
"         140.000 Flow length"
"           2.000 Overland Slope"
"           1.881 Pervious Area"

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"      140.000 Pervious length"
"      2.000 Pervious slope"
"      0.209 Impervious Area"
"      140.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"      75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          0.103      0.544      0.462      0.462 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area      "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration      36.338      5.313      28.909      minutes"
"      Time to Centroid      125.451      90.132      116.994      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      936.58      104.06      1040.65      c.m"
"      Rainfall losses      32.839      1.757      29.731      mm"
"      Runoff depth      16.953      48.035      20.061      mm"
"      Runoff volume      318.89      100.39      419.28      c.m"
"      Runoff coefficient      0.340      0.965      0.403      "
"      Maximum flow      0.092      0.057      0.103      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.103      0.613      0.462      0.462"
" 33      CATCHMENT 301"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      301      Scott Street External"
"      55.000      % Impervious"
"      2.600      Total Area"
"      95.000      Flow length"
"      2.000      Overland Slope"
"      1.170      Pervious Area"
"      95.000      Pervious length"
"      2.000      Pervious slope"
"      1.430      Impervious Area"
"      95.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"

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"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.405	0.613	0.462	0.462 c.m/sec"	
"		Catchment 301	Pervious	Impervious	Total Area	"
"		Surface Area	1.170	1.430	2.600	hectare"
"		Time of concentration	28.795	4.210	9.735	minutes"
"		Time to Centroid	116.746	88.669	94.978	minutes"
"		Rainfall depth	49.792	49.792	49.792	mm"
"		Rainfall volume	582.56	712.02	1294.58	c.m"
"		Rainfall losses	32.859	1.996	15.884	mm"
"		Runoff depth	16.933	47.796	33.908	mm"
"		Runoff volume	198.12	683.48	881.60	c.m"
"		Runoff coefficient	0.340	0.960	0.681	"
"		Maximum flow	0.068	0.395	0.405	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.405	1.018	0.462	0.462"	
" 33		CATCHMENT 205"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	205	Internal to Pond Inet 1"				
"	65.000	% Impervious"				
"	4.320	Total Area"				
"	100.000	Flow length"				
"	4.000	Overland Slope"				
"	1.512	Pervious Area"				
"	100.000	Pervious length"				
"	2.000	Pervious slope"				
"	2.808	Impervious Area"				
"	100.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.788	1.018	0.462	0.462 c.m/sec"	
"		Catchment 205	Pervious	Impervious	Total Area	"
"		Surface Area	1.512	2.808	4.320	hectare"
"		Time of concentration	29.695	4.341	8.402	minutes"
"		Time to Centroid	117.798	88.850	93.486	minutes"

"	Rainfall depth	49.792	49.792	49.792	mm"
"	Rainfall volume	752.85	1398.15	2151.00	c.m"
"	Rainfall losses	32.840	1.920	12.742	mm"
"	Runoff depth	16.952	47.871	37.050	mm"
"	Runoff volume	256.32	1344.23	1600.55	c.m"
"	Runoff coefficient	0.340	0.961	0.744	"
"	Maximum flow	0.086	0.775	0.788	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	0.788 1.806 0.462 0.462"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.788 1.806 1.806 0.462"				
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow	1.806		c.m/sec"	
"	Hydrograph volume	4110.126		c.m"	
"	0.788 1.806 1.806 1.806"				
" 40	HYDROGRAPH Confluence 1"				
"	7 Confluence "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow	1.806		c.m/sec"	
"	Hydrograph volume	4110.127		c.m"	
"	0.788 1.806 1.806 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.788 1.806 1.806 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow	1.806		c.m/sec"	
"	Hydrograph volume	4110.127		c.m"	
"	0.788 1.806 1.806 1.806"				
" 40	HYDROGRAPH Confluence 3"				
"	7 Confluence "				
"	3 Node #"				
"	Pond Inlet 3"				
"	Maximum flow	1.988		c.m/sec"	
"	Hydrograph volume	3989.446		c.m"	
"	0.788 1.988 1.806 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.788 1.988 1.988 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				

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"          Pond"
"          Maximum flow          3.794    c.m/sec"
"          Hydrograph volume     8099.575    c.m"
"          0.788    1.988    1.988    3.794"
" 40    HYDROGRAPH Confluence    2"
"          7 Confluence "
"          2 Node #"
"          Pond Inlet 2"
"          Maximum flow          0.462    c.m/sec"
"          Hydrograph volume     974.732    c.m"
"          0.788    0.462    1.988    0.000"
" 40    HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"          0.788    0.462    0.462    0.000"
" 40    HYDROGRAPH Combine    1000"
"          6 Combine "
"          1000 Node #"
"          Pond"
"          Maximum flow          4.256    c.m/sec"
"          Hydrograph volume     9074.310    c.m"
"          0.788    0.462    0.462    4.256"
" 40    HYDROGRAPH Confluence    1000"
"          7 Confluence "
"          1000 Node #"
"          Pond"
"          Maximum flow          4.256    c.m/sec"
"          Hydrograph volume     9074.310    c.m"
"          0.788    4.256    0.462    0.000"
" 40    HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"          0.788    4.256    4.256    0.000"
" 40    HYDROGRAPH Next link "
"          5 Next link "
"          0.788    4.256    4.256    0.000"
" 33    CATCHMENT 206"
"          1 Triangular SCS"
"          1 Equal length"
"          2 Horton equation"
"          206 Pond"
"          80.000 % Impervious"
"          3.290 Total Area"
"          100.000 Flow length"
"          4.000 Overland Slope"
"          0.658 Pervious Area"
"          100.000 Pervious length"
"          2.000 Pervious slope"
"          2.632 Impervious Area"
"          100.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"

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"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.732      4.256      4.256      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration 29.695      4.341      6.403      minutes"
"      Time to Centroid      117.798      88.850      91.205      minutes"
"      Rainfall depth      49.792      49.792      49.792      mm"
"      Rainfall volume      327.63      1310.52      1638.15      c.m"
"      Rainfall losses      32.840      1.920      8.104      mm"
"      Runoff depth      16.952      47.871      41.688      mm"
"      Runoff volume      111.55      1259.97      1371.52      c.m"
"      Runoff coefficient      0.340      0.961      0.837      "
"      Maximum flow      0.038      0.726      0.732      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.732      4.988      4.256      0.000"
" 54      POND DESIGN"
"      4.988      Current peak flow      c.m/sec"
"      0.206      Target outflow      c.m/sec"
"      10445.8      Hydrograph volume      c.m"
"      17.      Number of stages"
"      456.800      Minimum water level      metre"
"      458.000      Maximum water level      metre"
"      456.800      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      456.000      0.000      0.000"
"      456.100      0.01500      680.300"
"      456.200      0.02900      1395.900"
"      456.300      0.04200      2147.500"
"      456.400      0.05200      2935.400"
"      456.480      0.05900      3592.400"
"      456.500      0.06000      3760.400"
"      456.600      0.2840      4627.400"
"      456.700      0.7630      5539.800"
"      456.800      1.491      6496.900"
"      456.900      2.386      7500.300"
"      457.000      2.490      8551.600"
"      457.100      6.036      9652.000"
"      457.200      12.632      10803.10"
"      457.300      21.379      12006.50"
"      457.400      31.960      13275.50"

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"          457.500    44.189  14711.10"
"      Peak outflow                1.235    c.m/sec"
"      Maximum level                456.765    metre"
"      Maximum storage              6159.941    c.m"
"      Centroidal lag                8.236    hours"
"          0.732    4.988    1.235    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.732    1.235    1.235    0.000"
" 54      POND DESIGN"
"          1.235 Current peak flow    c.m/sec"
"          0.206 Target outflow    c.m/sec"
"      10392.8 Hydrograph volume    c.m"
"          9. Number of stages"
"      456.800 Minimum water level    metre"
"      458.000 Maximum water level    metre"
"      456.800 Starting water level    metre"
"          0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          454.860    0.000    0.000"
"          454.960  1.20E-05    4.000"
"          455.060  1.20E-05    8.000"
"          455.160  1.20E-05   12.000"
"          455.260  1.20E-05   16.000"
"          455.360  1.20E-05   20.000"
"          455.460    1.702   20.500"
"          455.560    4.979   21.000"
"          455.660    9.395   21.500"
"      Peak outflow                1.235    c.m/sec"
"      Maximum level                455.433    metre"
"      Maximum storage              20.363    c.m"
"      Centroidal lag                8.348    hours"
"          0.732    1.235    1.235    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          0.732    1.235    1.235    0.000"
" 56      DIVERSION"
"      3000 Node number"
"      0.000 Overflow threshold"
"      0.395 Computed diverted fraction"
"          0 Conduit type; 1=Pipe;2=Channel"
"      Peak of diverted flow        0.488    c.m/sec"
"      Volume of diverted flow     4097.439    c.m"
"      DIV03000.005hyd"
"      Major flow at 3000"
"          0.732    1.235    0.747    0.000 c.m/sec"
" 81      ADD COMMENT=====
"          1 Lines of comment"
"      Diverted to existing wetland on site"
" 40      HYDROGRAPH Combine    4000"

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"          6  Combine "
"      4000  Node #"
"          Site"
"          Maximum flow          0.747    c.m/sec"
"          Hydrograph volume      6276.480  c.m"
"          0.732    1.235    0.747    0.747"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          0.732    0.000    0.747    0.747"
" 33      CATCHMENT 200"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          200 Rear yard and Open Space to Grand River"
"          10.000 % Impervious"
"          7.530 Total Area"
"          100.000 Flow length"
"          5.000 Overland Slope"
"          6.777 Pervious Area"
"          100.000 Pervious length"
"          2.000 Pervious slope"
"          0.753 Impervious Area"
"          100.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"          0.437    0.000    0.747    0.747 c.m/sec"
"          Catchment 200      Pervious      Impervious Total Area "
"          Surface Area      6.777      0.753      7.530      hectare"
"          Time of concentration 29.695      4.341      23.640      minutes"
"          Time to Centroid    117.798     88.850     110.884     minutes"
"          Rainfall depth     49.792     49.792     49.792     mm"
"          Rainfall volume     3374.38    374.93     3749.31     c.m"
"          Rainfall losses     32.840     1.920     29.748     mm"
"          Runoff depth        16.952     47.871     20.044     mm"
"          Runoff volume       1148.85    360.47     1509.32     c.m"
"          Runoff coefficient   0.340     0.961     0.403     "
"          Maximum flow       0.387     0.208     0.437     c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.437    0.437    0.747    0.747"
" 40      HYDROGRAPH Copy to Outflow"

```

```

"      8  Copy to Outflow"
"      0.437    0.437    0.437    0.747"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          1.136    c.m/sec"
"      Hydrograph volume     7785.801  c.m"
"      0.437    0.437    0.437    1.136"
" 40    HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"      0.437    0.000    0.437    1.136"
" 47    FILEI_0 Read/Open DIV03000.005hyd"
"      1  1=read/open; 2=write/save"
"      2  1=rainfall; 2=hydrograph"
"      1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03000.005hyd"
"      Major flow at 3000"
"      Total volume          4097.439  c.m"
"      Maximum flow          0.488    c.m/sec"
"      0.488    0.000    0.437    1.136 c.m/sec"
" 40    HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"      0.488    0.488    0.437    1.136"
" 40    HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"      0.488    0.488    0.488    1.136"
" 40    HYDROGRAPH  Combine  4000"
"      6  Combine "
" 4000  Node #"
"      Site"
"      Maximum flow          1.616    c.m/sec"
"      Hydrograph volume     11883.240  c.m"
"      0.488    0.488    0.488    1.616"
" 40    HYDROGRAPH Confluence  4000"
"      7  Confluence "
" 4000  Node #"
"      Site"
"      Maximum flow          1.616    c.m/sec"
"      Hydrograph volume     11883.240  c.m"
"      0.488    1.616    0.488    0.000"
" 81    ADD COMMENT=====
"      1  Lines of comment"
"      Total flow to Grand River"
" 38    START/RE-START TOTALS 4000"
"      3  Runoff Totals on EXIT"
"      Total Catchment area          36.330  hectare"
"      Total Impervious area         18.751  hectare"
"      Total % impervious           51.612"
" 19    EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 post  100 year.out"
"          Licensee name:                      gmbp"
"          Company                             "
"          Date & Time last used:              2/22/2023 at 11:06:09 AM"
" 31      TIME PARAMETERS"
"          5.000  Time Step"
"          180.000 Max. Storm length"
"          3600.000 Max. Hydrograph"
" 32      STORM Chicago storm"
"          1  Chicago storm"
"          6933.020 Coefficient A"
"          34.699  Constant B"
"          0.998  Exponent C"
"          0.380  Fraction R"
"          180.000 Duration"
"          1.000  Time step multiplier"
"          Maximum intensity                    168.777  mm/hr"
"          Total depth                          97.921  mm"
"          6  100hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302 External lands and water tower"
"          55.000 % Impervious"
"          0.670  Total Area"
"          30.000 Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000 Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000 Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"
"          0.015  Impervious Manning 'n'"
"          0.000  Impervious Max.infiltration"
"          0.000  Impervious Min.infiltration"
"          0.050  Impervious Lag constant (hours)"
"          1.500  Impervious Depression storage"
"          0.233  0.000  0.000  0.000 c.m/sec"

```

	Catchment 302	Pervious	Impervious	Total Area	
"	Surface Area	0.302	0.368	0.670	hectare"
"	Time of concentration	10.076	1.799	4.711	minutes"
"	Time to Centroid	97.792	84.352	89.081	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	295.23	360.84	656.07	c.m"
"	Rainfall losses	34.803	2.791	17.197	mm"
"	Runoff depth	63.119	95.130	80.725	mm"
"	Runoff volume	190.30	350.55	540.86	c.m"
"	Runoff coefficient	0.645	0.971	0.824	"
"	Maximum flow	0.099	0.153	0.233	c.m/sec"

" 40 HYDROGRAPH Add Runoff "

"	4	Add Runoff "			
"		0.233	0.233	0.000	0.000"

" 33 CATCHMENT 201"

"	1	Triangular SCS"			
"	1	Equal length"			
"	2	Horton equation"			
"	201	Townhome and Apartment Blocks"			
"	90.000	% Impervious"			
"	4.280	Total Area"			
"	200.000	Flow length"			
"	4.000	Overland Slope"			
"	0.428	Pervious Area"			
"	200.000	Pervious length"			
"	2.000	Pervious slope"			
"	3.852	Impervious Area"			
"	200.000	Impervious length"			
"	2.000	Impervious slope"			
"	0.250	Pervious Manning 'n'"			
"	75.000	Pervious Max.infiltration"			
"	5.000	Pervious Min.infiltration"			
"	0.250	Pervious Lag constant (hours)"			
"	5.000	Pervious Depression storage"			
"	0.015	Impervious Manning 'n'"			
"	0.000	Impervious Max.infiltration"			
"	0.000	Impervious Min.infiltration"			
"	0.050	Impervious Lag constant (hours)"			
"	1.500	Impervious Depression storage"			
"		1.688	0.233	0.000	0.000 c.m/sec"

	Catchment 201	Pervious	Impervious	Total Area	
"	Surface Area	0.428	3.852	4.280	hectare"
"	Time of concentration	31.450	5.616	7.390	minutes"
"	Time to Centroid	122.128	89.288	91.543	minutes"
"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	419.10	3771.93	4191.03	c.m"
"	Rainfall losses	34.482	2.307	5.524	mm"
"	Runoff depth	63.439	95.615	92.397	mm"
"	Runoff volume	271.52	3683.07	3954.59	c.m"
"	Runoff coefficient	0.648	0.976	0.944	"

"		Maximum flow	0.079	1.669	1.688	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			1.688	1.920	0.000	0.000"
" 33		CATCHMENT 304"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	304	External - Luther Rd."				
"	55.000	% Impervious"				
"	1.090	Total Area"				
"	110.000	Flow length"				
"	2.000	Overland Slope"				
"	0.491	Pervious Area"				
"	110.000	Pervious length"				
"	2.000	Pervious slope"				
"	0.600	Impervious Area"				
"	110.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"			0.301	1.920	0.000	0.000 c.m/sec"
"		Catchment 304	Pervious	Impervious	Total Area	"
"		Surface Area	0.491	0.600	1.090	hectare"
"		Time of concentration	21.971	3.923	10.297	minutes"
"		Time to Centroid	111.333	87.214	95.731	minutes"
"		Rainfall depth	97.921	97.921	97.921	mm"
"		Rainfall volume	480.30	587.04	1067.34	c.m"
"		Rainfall losses	34.507	2.882	17.113	mm"
"		Runoff depth	63.414	95.039	80.808	mm"
"		Runoff volume	311.04	569.76	880.81	c.m"
"		Runoff coefficient	0.648	0.971	0.825	"
"		Maximum flow	0.114	0.260	0.301	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"			0.301	2.221	0.000	0.000"
" 33		CATCHMENT 202"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	202	Internal to Pond Inet 3"				
"	60.000	% Impervious"				

```

"      4.150  Total Area"
"    130.000  Flow length"
"      4.000  Overland Slope"
"      1.660  Pervious Area"
"    130.000  Pervious length"
"      2.000  Pervious slope"
"      2.490  Impervious Area"
"    130.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"    75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          1.195    2.221    0.000    0.000 c.m/sec"
"      Catchment 202      Pervious      Impervious      Total Area  "
"      Surface Area      1.660      2.490      4.150      hectare"
"      Time of concentration  24.287    4.337    10.458    minutes"
"      Time to Centroid      113.975   87.701    95.762    minutes"
"      Rainfall depth      97.921    97.921    97.921    mm"
"      Rainfall volume      1625.49   2438.24   4063.73   c.m"
"      Rainfall losses      34.488    2.372    15.219    mm"
"      Runoff depth         63.434    95.549    82.703    mm"
"      Runoff volume        1053.00   2379.17   3432.17   c.m"
"      Runoff coefficient    0.648     0.976     0.845     "
"      Maximum flow         0.358     1.078     1.195     c.m/sec"
" 40  HYDROGRAPH Add Runoff "
"      4  Add Runoff "
"          1.195    3.416    0.000    0.000"
" 40  HYDROGRAPH Copy to Outflow"
"      8  Copy to Outflow"
"          1.195    3.416    3.416    0.000"
" 40  HYDROGRAPH Combine 3"
"      6  Combine "
"      3  Node #"
"      Pond Inlet 3"
"      Maximum flow          3.416    c.m/sec"
"      Hydrograph volume      8808.417  c.m"
"          1.195    3.416    3.416    3.416"
" 40  HYDROGRAPH Start - New Tributary"
"      2  Start - New Tributary"
"          1.195    0.000    3.416    3.416"
" 33  CATCHMENT 203"
"      1  Triangular SCS"
"      1  Equal length"

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"          2 Horton equation"
"          203 Internal to Pond Inet 2"
" 60.000 % Impervious"
"          2.760 Total Area"
" 30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"
" 30.000 Pervious length"
"          2.000 Pervious slope"
"          1.656 Impervious Area"
" 30.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
" 75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.979      0.000      3.416      3.416 c.m/sec"
"          Catchment 203          Pervious  Impervious Total Area "
"          Surface Area          1.104      1.656      2.760      hectare"
"          Time of concentration 10.076      1.799      4.338      minutes"
"          Time to Centroid      97.792      84.352      88.474      minutes"
"          Rainfall depth        97.921      97.921      97.921      mm"
"          Rainfall volume        1081.05     1621.58     2702.63     c.m"
"          Rainfall losses        34.803      2.791      15.596      mm"
"          Runoff depth           63.119      95.130      82.325      mm"
"          Runoff volume          696.83     1575.35     2272.18     c.m"
"          Runoff coefficient      0.645      0.971      0.841      "
"          Maximum flow           0.363      0.689      0.979      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"          4 Add Runoff "
"              0.979      0.979      3.416      3.416"
" 40 HYDROGRAPH Copy to Outflow"
"          8 Copy to Outflow"
"              0.979      0.979      0.979      3.416"
" 40 HYDROGRAPH Combine 2"
"          6 Combine "
"          2 Node #"
"          Pond Inlet 2"
"          Maximum flow           0.979      c.m/sec"
"          Hydrograph volume      2272.181     c.m"
"              0.979      0.979      0.979      0.979"
" 40 HYDROGRAPH Start - New Tributary"
"          2 Start - New Tributary"
"              0.979      0.000      0.979      0.979"

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" 33      CATCHMENT 303"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          303 External"
"          55.000 % Impervious"
"          3.550 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.597 Pervious Area"
"         140.000 Pervious length"
"          2.000 Pervious slope"
"          1.952 Impervious Area"
"         140.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"                0.948      0.000      0.979      0.979 c.m/sec"
"          Catchment 303      Pervious      Impervious      Total Area  "
"          Surface Area      1.597      1.952      3.550      hectare"
"          Time of concentration  25.391      4.534      11.868      minutes"
"          Time to Centroid      115.220      87.952      97.540      minutes"
"          Rainfall depth      97.921      97.921      97.921      mm"
"          Rainfall volume      1564.29      1911.91      3476.21      c.m"
"          Rainfall losses      34.527      2.269      16.785      mm"
"          Runoff depth      63.395      95.653      81.137      mm"
"          Runoff volume      1012.73      1867.62      2880.35      c.m"
"          Runoff coefficient      0.647      0.977      0.829      "
"          Maximum flow      0.333      0.843      0.948      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"                0.948      0.948      0.979      0.979"
" 33      CATCHMENT 204"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          204 Park"
"          10.000 % Impervious"
"          2.090 Total Area"
"         140.000 Flow length"
"          2.000 Overland Slope"
"          1.881 Pervious Area"

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"      140.000  Pervious length"
"      2.000   Pervious slope"
"      0.209   Impervious Area"
"      140.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"      75.000   Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"
"      0.015   Impervious Manning 'n'"
"      0.000   Impervious Max.infiltration"
"      0.000   Impervious Min.infiltration"
"      0.050   Impervious Lag constant (hours)"
"      1.500   Impervious Depression storage"
"              0.433      0.948      0.979      0.979 c.m/sec"
"      Catchment 204      Pervious      Impervious Total Area  "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration 25.391      4.534      22.397      minutes"
"      Time to Centroid      115.220      87.952      111.305      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      1841.90      204.66      2046.56      c.m"
"      Rainfall losses      34.527      2.269      31.301      mm"
"      Runoff depth      63.395      95.653      66.620      mm"
"      Runoff volume      1192.45      199.91      1392.37      c.m"
"      Runoff coefficient      0.647      0.977      0.680      "
"      Maximum flow      0.393      0.090      0.433      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.433      1.178      0.979      0.979"
" 33      CATCHMENT 301"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      301      Scott Street External"
"      55.000  % Impervious"
"      2.600   Total Area"
"      95.000  Flow length"
"      2.000   Overland Slope"
"      1.170   Pervious Area"
"      95.000  Pervious length"
"      2.000   Pervious slope"
"      1.430   Impervious Area"
"      95.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"      75.000  Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"

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"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.730      1.178      0.979      0.979 c.m/sec"
"      Catchment 301      Pervious      Impervious Total Area "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration 20.121      3.593      9.455      minutes"
"      Time to Centroid      109.215      86.819      94.762      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      1145.68      1400.27      2545.95      c.m"
"      Rainfall losses      34.520      3.529      17.475      mm"
"      Runoff depth      63.401      94.392      80.446      mm"
"      Runoff volume      741.80      1349.81      2091.61      c.m"
"      Runoff coefficient      0.647      0.964      0.822      "
"      Maximum flow      0.277      0.616      0.730      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.730      1.891      0.979      0.979"
" 33      CATCHMENT 205"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      205      Internal to Pond Inet 1"
"      65.000      % Impervious"
"      4.320      Total Area"
"      100.000      Flow length"
"      4.000      Overland Slope"
"      1.512      Pervious Area"
"      100.000      Pervious length"
"      2.000      Pervious slope"
"      2.808      Impervious Area"
"      100.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"      75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"
"      0.015      Impervious Manning 'n'"
"      0.000      Impervious Max.infiltration"
"      0.000      Impervious Min.infiltration"
"      0.050      Impervious Lag constant (hours)"
"      1.500      Impervious Depression storage"
"              1.353      1.891      0.979      0.979 c.m/sec"
"      Catchment 205      Pervious      Impervious Total Area "
"      Surface Area      1.512      2.808      4.320      hectare"
"      Time of concentration 20.749      3.705      8.225      minutes"
"      Time to Centroid      109.932      86.956      93.049      minutes"

```

"	Rainfall depth	97.921	97.921	97.921	mm"
"	Rainfall volume	1480.57	2749.63	4230.20	c.m"
"	Rainfall losses	34.536	3.348	14.264	mm"
"	Runoff depth	63.385	94.574	83.658	mm"
"	Runoff volume	958.39	2655.63	3614.02	c.m"
"	Runoff coefficient	0.647	0.966	0.854	"
"	Maximum flow	0.359	1.213	1.353	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"	1.353 3.244 0.979 0.979"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	1.353 3.244 3.244 0.979"				
" 40	HYDROGRAPH Combine 1"				
"	6 Combine "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		3.244		c.m/sec"
"	Hydrograph volume		9978.332		c.m"
"	1.353 3.244 3.244 3.244"				
" 40	HYDROGRAPH Confluence 1"				
"	7 Confluence "				
"	1 Node #"				
"	Pond Inlet 1"				
"	Maximum flow		3.244		c.m/sec"
"	Hydrograph volume		9978.331		c.m"
"	1.353 3.244 3.244 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	1.353 3.244 3.244 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		3.244		c.m/sec"
"	Hydrograph volume		9978.331		c.m"
"	1.353 3.244 3.244 3.244"				
" 40	HYDROGRAPH Confluence 3"				
"	7 Confluence "				
"	3 Node #"				
"	Pond Inlet 3"				
"	Maximum flow		3.416		c.m/sec"
"	Hydrograph volume		8808.417		c.m"
"	1.353 3.416 3.244 0.000"				
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	1.353 3.416 3.416 0.000"				
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				

"	Pond"				
"	Maximum flow		6.660	c.m/sec"	
"	Hydrograph volume		18786.760	c.m"	
"	1.353	3.416	3.416	6.660"	
" 40	HYDROGRAPH Confluence	2"			
"	7 Confluence "				
"	2 Node #"				
"	Pond Inlet 2"				
"	Maximum flow		0.979	c.m/sec"	
"	Hydrograph volume		2272.181	c.m"	
"	1.353	0.979	3.416	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	1.353	0.979	0.979	0.000"	
" 40	HYDROGRAPH Combine	1000"			
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		7.639	c.m/sec"	
"	Hydrograph volume		21058.924	c.m"	
"	1.353	0.979	0.979	7.639"	
" 40	HYDROGRAPH Confluence	1000"			
"	7 Confluence "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		7.639	c.m/sec"	
"	Hydrograph volume		21058.926	c.m"	
"	1.353	7.639	0.979	0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	1.353	7.639	7.639	0.000"	
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	1.353	7.639	7.639	0.000"	
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	206 Pond"				
"	80.000 % Impervious"				
"	3.290 Total Area"				
"	100.000 Flow length"				
"	4.000 Overland Slope"				
"	0.658 Pervious Area"				
"	100.000 Pervious length"				
"	2.000 Pervious slope"				
"	2.632 Impervious Area"				
"	100.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				

```

"      75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          1.198      7.639      7.639      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration 20.749      3.705      6.151      minutes"
"      Time to Centroid 109.932      86.956      90.254      minutes"
"      Rainfall depth 97.921      97.921      97.921      mm"
"      Rainfall volume 644.32      2577.29      3221.61      c.m"
"      Rainfall losses 34.536      3.348      9.585      mm"
"      Runoff depth 63.385      94.574      88.336      mm"
"      Runoff volume 417.08      2489.18      2906.26      c.m"
"      Runoff coefficient 0.647      0.966      0.902      "
"      Maximum flow 0.156      1.137      1.198      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          1.198      8.837      7.639      0.000"
" 54      POND DESIGN"
"      8.837      Current peak flow      c.m/sec"
"      0.206      Target outflow      c.m/sec"
"      23965.2      Hydrograph volume      c.m"
"      17.      Number of stages"
"      456.800      Minimum water level      metre"
"      458.000      Maximum water level      metre"
"      456.800      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"      456.000      0.000      0.000"
"      456.100      0.01500      680.300"
"      456.200      0.02900      1395.900"
"      456.300      0.04200      2147.500"
"      456.400      0.05200      2935.400"
"      456.480      0.05900      3592.400"
"      456.500      0.06000      3760.400"
"      456.600      0.2840      4627.400"
"      456.700      0.7630      5539.800"
"      456.800      1.491      6496.900"
"      456.900      2.386      7500.300"
"      457.000      2.490      8551.600"
"      457.100      6.036      9652.000"
"      457.200      12.632      10803.10"
"      457.300      21.379      12006.50"
"      457.400      31.960      13275.50"

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"          457.500    44.189  14711.10"
"      Peak outflow                5.771    c.m/sec"
"      Maximum level                457.093    metre"
"      Maximum storage              9569.881    c.m"
"      Centroidal lag                4.769    hours"
"          1.198    8.837    5.771    0.000 c.m/sec"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          1.198    5.771    5.771    0.000"
" 56      DIVERSION"
"          2000 Node number"
"          3.056 Overflow threshold"
"          1.000 Required diverted fraction"
"          0 Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow        2.715    c.m/sec"
"          Volume of diverted flow      3134.302    c.m"
"          DIV02000.100hyd"
"          Divert Weir 1 and 2 away from Dispersion Trench"
"          1.198    5.771    3.056    0.000 c.m/sec"
" 81      ADD COMMENT=====
"          1 Lines of comment"
"          Divert Flow from Weirs 1 and 2 away from Dispersion Trench"
" 40      HYDROGRAPH Combine 4000"
"          6 Combine "
"          4000 Node #"
"          Site"
"          Maximum flow                3.056    c.m/sec"
"          Hydrograph volume            20685.248    c.m"
"          1.198    5.771    3.056    3.056"
" 40      HYDROGRAPH Next link "
"          5 Next link "
"          1.198    3.056    3.056    3.056"
" 54      POND DESIGN"
"          3.056 Current peak flow    c.m/sec"
"          0.206 Target outflow    c.m/sec"
"          20685.2 Hydrograph volume    c.m"
"          9. Number of stages"
"          456.800 Minimum water level    metre"
"          458.000 Maximum water level    metre"
"          456.800 Starting water level    metre"
"          0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge    Volume"
"          454.860    0.000    0.000"
"          454.960  1.20E-05    4.000"
"          455.060  1.20E-05    8.000"
"          455.160  1.20E-05   12.000"
"          455.260  1.20E-05   16.000"
"          455.360  1.20E-05   20.000"
"          455.460    1.702   20.500"
"          455.560    4.979   21.000"

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"           455.660      9.395      21.500"
"           Peak outflow                3.056      c.m/sec"
"           Maximum level                455.501      metre"
"           Maximum storage              20.707      c.m"
"           Centroidal lag                5.258      hours"
"           1.198      3.056      3.056      3.056 c.m/sec"
" 40      HYDROGRAPH Next link "
"           5      Next link "
"           1.198      3.056      3.056      3.056"
" 56      DIVERSION"
"           3000      Node number"
"           0.000      Overflow threshold"
"           0.395      Computed diverted fraction"
"           0      Conduit type; 1=Pipe;2=Channel"
"           Peak of diverted flow        1.207      c.m/sec"
"           Volume of diverted flow      8163.324      c.m"
"           DIV03000.100hyd"
"           Major flow at 3000"
"           1.198      3.056      1.849      3.056 c.m/sec"
" 81      ADD COMMENT=====
"           1      Lines of comment"
"           Diverted to existing wetland on site"
" 40      HYDROGRAPH Combine 4000"
"           6      Combine "
"           4000      Node #"
"           Site"
"           Maximum flow                4.905      c.m/sec"
"           Hydrograph volume          33190.559      c.m"
"           1.198      3.056      1.849      4.905"
" 40      HYDROGRAPH Start - New Tributary"
"           2      Start - New Tributary"
"           1.198      0.000      1.849      4.905"
" 33      CATCHMENT 200"
"           1      Triangular SCS"
"           1      Equal length"
"           2      Horton equation"
"           200      Rear yard and Open Space to Grand River"
"           10.000      % Impervious"
"           7.530      Total Area"
"           100.000      Flow length"
"           5.000      Overland Slope"
"           6.777      Pervious Area"
"           100.000      Pervious length"
"           2.000      Pervious slope"
"           0.753      Impervious Area"
"           100.000      Impervious length"
"           2.000      Impervious slope"
"           0.250      Pervious Manning 'n'"
"           75.000      Pervious Max.infiltration"
"           5.000      Pervious Min.infiltration"

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"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          1.746      0.000      1.849      4.905 c.m/sec"
"      Catchment 200 Pervious Impervious Total Area "
"      Surface Area      6.777      0.753      7.530      hectare"
"      Time of concentration 20.749      3.705      18.326      minutes"
"      Time to Centroid 109.932      86.956      106.665      minutes"
"      Rainfall depth      97.921      97.921      97.921      mm"
"      Rainfall volume      6636.13      737.35      7373.48      c.m"
"      Rainfall losses      34.536      3.348      31.417      mm"
"      Runoff depth      63.385      94.574      66.504      mm"
"      Runoff volume      4295.63      712.14      5007.77      c.m"
"      Runoff coefficient      0.647      0.966      0.679      "
"      Maximum flow      1.608      0.325      1.746      c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          1.746      1.746      1.849      4.905"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"          1.746      1.746      1.746      4.905"
" 40 HYDROGRAPH Combine 4000"
"      6 Combine "
"      4000 Node #"
"      Site"
"      Maximum flow      6.651      c.m/sec"
"      Hydrograph volume      38198.348      c.m"
"          1.746      1.746      1.746      6.651"
" 40 HYDROGRAPH Start - New Tributary"
"      2 Start - New Tributary"
"          1.746      0.000      1.746      6.651"
" 47 FILEI_0 Read/Open DIV03000.100hyd"
"      1 1=read/open; 2=write/save"
"      2 1=rainfall; 2=hydrograph"
"      1 1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03000.100hyd"
"      Major flow at 3000"
"      Total volume      8163.322      c.m"
"      Maximum flow      1.207      c.m/sec"
"          1.207      0.000      1.746      6.651 c.m/sec"
" 40 HYDROGRAPH Add Runoff "
"      4 Add Runoff "
"          1.207      1.207      1.746      6.651"
" 40 HYDROGRAPH Copy to Outflow"
"      8 Copy to Outflow"
"          1.207      1.207      1.207      6.651"

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" 40      HYDROGRAPH  Combine  4000"
"          6  Combine  "
"          4000 Node #"
"          Site"
"          Maximum flow          7.858   c.m/sec"
"          Hydrograph volume      46361.719 c.m"
"          1.207   1.207   1.207   7.858"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"          1.207   0.000   1.207   7.858"
" 47      FILEI_0 Read/Open DIV02000.100hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV02000.100hyd"
"          Divert Weir 1 and 2 away from Dispersion Trench"
"          Total volume          3134.302   c.m"
"          Maximum flow          2.715   c.m/sec"
"          2.715   0.000   1.207   7.858 c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"          4  Add Runoff  "
"          2.715   2.715   1.207   7.858"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          2.715   2.715   2.715   7.858"
" 40      HYDROGRAPH  Combine  4000"
"          6  Combine  "
"          4000 Node #"
"          Site"
"          Maximum flow          10.574   c.m/sec"
"          Hydrograph volume      49496.012 c.m"
"          2.715   2.715   2.715   10.574"
" 40      HYDROGRAPH  Confluence  4000"
"          7  Confluence  "
"          4000 Node #"
"          Site"
"          Maximum flow          10.573   c.m/sec"
"          Hydrograph volume      49496.012 c.m"
"          2.715   10.573   2.715   0.000"
" 81      ADD COMMENT=====
"          1  Lines of comment"
"          Total flow to Grand River"
" 38      START/RE-START TOTALS 4000"
"          3  Runoff Totals on EXIT"
"          Total Catchment area          36.330   hectare"
"          Total Impervious area        18.751   hectare"
"          Total % impervious          51.612"
" 19      EXIT"

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"          MIDUSS Output ----->"
"          MIDUSS version                      Version 2.25  rev. 473"
"          MIDUSS created                      Sunday, February 07, 2010"
"          10  Units used:                      ie METRIC"
"          Job folder:                         C:\Users\szaga\Documents\MIDUSS\104104"
"          Output filename:                    104-104 post  Reg.out"
"          Licensee name:                      gmbp"
"          Company                            "
"          Date & Time last used:              2/22/2023 at 11:46:29 AM"
" 31      TIME PARAMETERS"
"          60.000  Time Step"
"          2880.000  Max. Storm length"
"          5000.000  Max. Hydrograph"
" 32      STORM Historic"
"          5  Historic"
"          2880.000  Duration"
"          48.000  Rainfall intensity values"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.028    2.028    2.028    2.028"
"                  2.028    2.026    2.026    2.026    2.028"
"                  2.026    6.000    4.000    6.000    13.000"
"                  17.000    13.000    23.000    13.000    13.000"
"                  53.000    38.000    13.000"
"          Maximum intensity                    53.000  mm/hr"
"          Total depth                          285.000  mm"
"          7  9999hyd  Hydrograph extension used in this file"
" 33      CATCHMENT 302"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          302  External lands and water tower"
"          55.000  % Impervious"
"          0.670  Total Area"
"          30.000  Flow length"
"          2.000  Overland Slope"
"          0.302  Pervious Area"
"          30.000  Pervious length"
"          2.000  Pervious slope"
"          0.368  Impervious Area"
"          30.000  Impervious length"
"          2.000  Impervious slope"
"          0.250  Pervious Manning 'n'"
"          75.000  Pervious Max.infiltration"
"          5.000  Pervious Min.infiltration"
"          0.250  Pervious Lag constant (hours)"
"          5.000  Pervious Depression storage"

```

"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.078	0.000	0.000	0.000 c.m/sec"	
"		Catchment 302	Pervious	Impervious	Total Area	"
"		Surface Area	0.302	0.368	0.670	hectare"
"		Time of concentration	16.093	2.860	7.181	minutes"
"		Time to Centroid	2720.207	2251.465	2404.533	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	859.28	1050.22	1909.50	c.m"
"		Rainfall losses	139.563	39.596	84.581	mm"
"		Runoff depth	145.437	245.404	200.419	mm"
"		Runoff volume	438.49	904.31	1342.81	c.m"
"		Runoff coefficient	0.510	0.861	0.703	"
"		Maximum flow	0.031	0.047	0.078	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.078	0.078	0.000	0.000"	
" 33		CATCHMENT 201"				
"	1	Triangular SCS"				
"	1	Equal length"				
"	2	Horton equation"				
"	201	Townhome and Apartment Blocks"				
"	90.000	% Impervious"				
"	4.280	Total Area"				
"	200.000	Flow length"				
"	4.000	Overland Slope"				
"	0.428	Pervious Area"				
"	200.000	Pervious length"				
"	2.000	Pervious slope"				
"	3.852	Impervious Area"				
"	200.000	Impervious length"				
"	2.000	Impervious slope"				
"	0.250	Pervious Manning 'n'"				
"	75.000	Pervious Max.infiltration"				
"	5.000	Pervious Min.infiltration"				
"	0.250	Pervious Lag constant (hours)"				
"	5.000	Pervious Depression storage"				
"	0.015	Impervious Manning 'n'"				
"	0.000	Impervious Max.infiltration"				
"	0.000	Impervious Min.infiltration"				
"	0.050	Impervious Lag constant (hours)"				
"	1.500	Impervious Depression storage"				
"		0.512	0.078	0.000	0.000 c.m/sec"	
"		Catchment 201	Pervious	Impervious	Total Area	"
"		Surface Area	0.428	3.852	4.280	hectare"
"		Time of concentration	50.233	8.926	11.288	minutes"
"		Time to Centroid	2757.880	2264.252	2292.474	minutes"

"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	0.1220	1.0978	1.2198	ha-m"
"	Rainfall losses	138.714	16.956	29.132	mm"
"	Runoff depth	146.286	268.044	255.868	mm"
"	Runoff volume	0.0626	1.0325	1.0951	ha-m"
"	Runoff coefficient	0.513	0.941	0.898	"
"	Maximum flow	0.046	0.484	0.512	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.512	0.590	0.000	0.000"
" 33	CATCHMENT 304"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	304 External - Luther Rd."				
"	55.000 % Impervious"				
"	1.090 Total Area"				
"	110.000 Flow length"				
"	2.000 Overland Slope"				
"	0.491 Pervious Area"				
"	110.000 Pervious length"				
"	2.000 Pervious slope"				
"	0.600 Impervious Area"				
"	110.000 Impervious length"				
"	2.000 Impervious slope"				
"	0.250 Pervious Manning 'n'"				
"	75.000 Pervious Max.infiltration"				
"	5.000 Pervious Min.infiltration"				
"	0.250 Pervious Lag constant (hours)"				
"	5.000 Pervious Depression storage"				
"	0.015 Impervious Manning 'n'"				
"	0.000 Impervious Max.infiltration"				
"	0.000 Impervious Min.infiltration"				
"	0.050 Impervious Lag constant (hours)"				
"	1.500 Impervious Depression storage"				
"		0.117	0.590	0.000	0.000 c.m/sec"
"	Catchment 304	Pervious	Impervious	Total Area	"
"	Surface Area	0.491	0.600	1.090	hectare"
"	Time of concentration	35.092	6.236	15.325	minutes"
"	Time to Centroid	2739.025	2235.505	2394.105	minutes"
"	Rainfall depth	285.000	285.000	285.000	mm"
"	Rainfall volume	1397.93	1708.57	3106.50	c.m"
"	Rainfall losses	140.362	27.637	78.363	mm"
"	Runoff depth	144.638	257.363	206.637	mm"
"	Runoff volume	709.45	1542.89	2252.34	c.m"
"	Runoff coefficient	0.508	0.903	0.725	"
"	Maximum flow	0.054	0.076	0.117	c.m/sec"
" 40	HYDROGRAPH Add Runoff "				
"	4 Add Runoff "				
"		0.117	0.707	0.000	0.000"

```

" 33      CATCHMENT 202"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          202 Internal to Pond Inlet 3"
"          60.000 % Impervious"
"          4.150 Total Area"
"         130.000 Flow length"
"          4.000 Overland Slope"
"          1.660 Pervious Area"
"         130.000 Pervious length"
"          2.000 Pervious slope"
"          2.490 Impervious Area"
"         130.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"         75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"          0.447      0.707      0.000      0.000 c.m/sec"
"          Catchment 202      Pervious      Impervious Total Area "
"          Surface Area      1.660      2.490      4.150      hectare"
"          Time of concentration 38.792      6.893      15.449      minutes"
"          Time to Centroid      2741.816      2242.481      2376.420      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      0.4731      0.7096      1.1828      ha-m"
"          Rainfall losses      141.611      24.215      71.174      mm"
"          Runoff depth      143.389      260.785      213.826      mm"
"          Runoff volume      2380.25      6493.54      8873.79      c.m"
"          Runoff coefficient      0.503      0.915      0.750      "
"          Maximum flow      0.182      0.315      0.447      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"          0.447      1.153      0.000      0.000"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"          0.447      1.153      1.153      0.000"
" 40      HYDROGRAPH Combine 3"
"          6  Combine "
"          3  Node #"
"          Pond Inlet 3"
"          Maximum flow      1.153      c.m/sec"
"          Hydrograph volume      23420.094      c.m"
"          0.447      1.153      1.153      1.153"

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" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.447      0.000      1.153      1.153"
" 33      CATCHMENT 203"
"          1  Triangular SCS"
"          1  Equal length"
"          2  Horton equation"
"          203 Internal to Pond Inet 2"
"          60.000 % Impervious"
"          2.760 Total Area"
"          30.000 Flow length"
"          4.000 Overland Slope"
"          1.104 Pervious Area"
"          30.000 Pervious length"
"          2.000 Pervious slope"
"          1.656 Impervious Area"
"          30.000 Impervious length"
"          2.000 Impervious slope"
"          0.250 Pervious Manning 'n'"
"          75.000 Pervious Max.infiltration"
"          5.000 Pervious Min.infiltration"
"          0.250 Pervious Lag constant (hours)"
"          5.000 Pervious Depression storage"
"          0.015 Impervious Manning 'n'"
"          0.000 Impervious Max.infiltration"
"          0.000 Impervious Min.infiltration"
"          0.050 Impervious Lag constant (hours)"
"          1.500 Impervious Depression storage"
"              0.325      0.000      1.153      1.153 c.m/sec"
"          Catchment 203      Pervious      Impervious      Total Area  "
"          Surface Area      1.104      1.656      2.760      hectare"
"          Time of concentration 16.093      2.860      6.608      minutes"
"          Time to Centroid      2720.207      2251.465      2384.215      minutes"
"          Rainfall depth      285.000      285.000      285.000      mm"
"          Rainfall volume      3146.40      4719.60      7866.00      c.m"
"          Rainfall losses      139.563      39.596      79.583      mm"
"          Runoff depth      145.437      245.404      205.417      mm"
"          Runoff volume      1605.63      4063.89      5669.52      c.m"
"          Runoff coefficient      0.510      0.861      0.721      "
"          Maximum flow      0.115      0.211      0.325      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.325      0.325      1.153      1.153"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.325      0.325      0.325      1.153"
" 40      HYDROGRAPH Combine 2"
"          6  Combine "
"          2  Node #"
"          Pond Inlet 2"

```

"		Maximum flow	0.325	c.m/sec"
"		Hydrograph volume	5669.517	c.m"
"		0.325 0.325 0.325		0.325"
" 40		HYDROGRAPH Start - New Tributary"		
"	2	Start - New Tributary"		
"		0.325 0.000 0.325		0.325"
" 33		CATCHMENT 303"		
"	1	Triangular SCS"		
"	1	Equal length"		
"	2	Horton equation"		
"	303	External"		
"	55.000	% Impervious"		
"	3.550	Total Area"		
"	140.000	Flow length"		
"	2.000	Overland Slope"		
"	1.597	Pervious Area"		
"	140.000	Pervious length"		
"	2.000	Pervious slope"		
"	1.952	Impervious Area"		
"	140.000	Impervious length"		
"	2.000	Impervious slope"		
"	0.250	Pervious Manning 'n'"		
"	75.000	Pervious Max.infiltration"		
"	5.000	Pervious Min.infiltration"		
"	0.250	Pervious Lag constant (hours)"		
"	5.000	Pervious Depression storage"		
"	0.015	Impervious Manning 'n'"		
"	0.000	Impervious Max.infiltration"		
"	0.000	Impervious Min.infiltration"		
"	0.050	Impervious Lag constant (hours)"		
"	1.500	Impervious Depression storage"		
"		0.371 0.000 0.325		0.325 c.m/sec"
"		Catchment 303	Pervious	Impervious Total Area "
"		Surface Area	1.597	1.952 3.550 hectare"
"		Time of concentration	40.556	7.207 17.519 minutes"
"		Time to Centroid	2744.076	2245.793 2399.878 minutes"
"		Rainfall depth	285.000	285.000 285.000 mm"
"		Rainfall volume	0.4553	0.5565 1.0117 ha-m"
"		Rainfall losses	141.525	22.773 76.211 mm"
"		Runoff depth	143.475	262.227 208.789 mm"
"		Runoff volume	2292.02	5119.98 7412.00 c.m"
"		Runoff coefficient	0.503	0.920 0.733 "
"		Maximum flow	0.175	0.247 0.371 c.m/sec"
" 40		HYDROGRAPH Add Runoff "		
"	4	Add Runoff "		
"		0.371 0.371 0.325		0.325"
" 33		CATCHMENT 204"		
"	1	Triangular SCS"		
"	1	Equal length"		
"	2	Horton equation"		

```

"      204   Park"
"      10.000 % Impervious"
"      2.090   Total Area"
"     140.000 Flow length"
"      2.000 Overland Slope"
"      1.881 Pervious Area"
"     140.000 Pervious length"
"      2.000 Pervious slope"
"      0.209 Impervious Area"
"     140.000 Impervious length"
"      2.000 Impervious slope"
"      0.250 Pervious Manning 'n'"
"     75.000 Pervious Max.infiltration"
"      5.000 Pervious Min.infiltration"
"      0.250 Pervious Lag constant (hours)"
"      5.000 Pervious Depression storage"
"      0.015 Impervious Manning 'n'"
"      0.000 Impervious Max.infiltration"
"      0.000 Impervious Min.infiltration"
"      0.050 Impervious Lag constant (hours)"
"      1.500 Impervious Depression storage"
"          0.225      0.371      0.325      0.325 c.m/sec"
"      Catchment 204      Pervious      Impervious      Total Area "
"      Surface Area      1.881      0.209      2.090      hectare"
"      Time of concentration 40.556      7.207      34.926      minutes"
"      Time to Centroid      2744.076      2245.793      2659.968      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      5360.85      595.65      5956.50      c.m"
"      Rainfall losses      141.525      22.773      129.649      mm"
"      Runoff depth      143.475      262.227      155.351      mm"
"      Runoff volume      2698.77      548.05      3246.83      c.m"
"      Runoff coefficient      0.503      0.920      0.545      "
"      Maximum flow      0.206      0.026      0.225      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"          0.225      0.574      0.325      0.325"
" 33      CATCHMENT 301"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      301      Scott Street External"
"     55.000 % Impervious"
"      2.600   Total Area"
"     95.000 Flow length"
"      2.000 Overland Slope"
"      1.170 Pervious Area"
"     95.000 Pervious length"
"      2.000 Pervious slope"
"      1.430 Impervious Area"
"     95.000 Impervious length"

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"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
"     75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"              0.282      0.574      0.325      0.325 c.m/sec"
"      Catchment 301      Pervious      Impervious      Total Area  "
"      Surface Area      1.170      1.430      2.600      hectare"
"      Time of concentration  32.137      5.711      14.115      minutes"
"      Time to Centroid      2737.280      2232.538      2393.050      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      3334.50      4075.50      7410.00      c.m"
"      Rainfall losses      139.558      29.801      79.192      mm"
"      Runoff depth      145.442      255.199      205.808      mm"
"      Runoff volume      1701.67      3649.34      5351.01      c.m"
"      Runoff coefficient      0.510      0.895      0.722      "
"      Maximum flow      0.128      0.181      0.282      c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"      4      Add Runoff "
"              0.282      0.826      0.325      0.325"
" 33      CATCHMENT 205"
"      1      Triangular SCS"
"      1      Equal length"
"      2      Horton equation"
"      205      Internal to Pond Inet 1"
"      65.000      % Impervious"
"      4.320      Total Area"
"     100.000      Flow length"
"      4.000      Overland Slope"
"      1.512      Pervious Area"
"     100.000      Pervious length"
"      2.000      Pervious slope"
"      2.808      Impervious Area"
"     100.000      Impervious length"
"      2.000      Impervious slope"
"      0.250      Pervious Manning 'n'"
"     75.000      Pervious Max.infiltration"
"      5.000      Pervious Min.infiltration"
"      0.250      Pervious Lag constant (hours)"
"      5.000      Pervious Depression storage"
"      0.015      Impervious Manning 'n'"
"      0.000      Impervious Max.infiltration"
"      0.000      Impervious Min.infiltration"
"      0.050      Impervious Lag constant (hours)"

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"	1.500	Impervious Depression storage"				
"		0.485	0.826	0.325	0.325	c.m/sec"
"		Catchment 205	Pervious	Impervious	Total Area	"
"		Surface Area	1.512	2.808	4.320	hectare"
"		Time of concentration	33.142	5.889	12.266	minutes"
"		Time to Centroid	2737.930	2233.505	2351.531	minutes"
"		Rainfall depth	285.000	285.000	285.000	mm"
"		Rainfall volume	0.4309	0.8003	1.2312	ha-m"
"		Rainfall losses	139.790	29.018	67.788	mm"
"		Runoff depth	145.210	255.982	217.212	mm"
"		Runoff volume	2195.58	7187.97	9383.55	c.m"
"		Runoff coefficient	0.510	0.898	0.762	"
"		Maximum flow	0.166	0.355	0.485	c.m/sec"
" 40		HYDROGRAPH Add Runoff "				
"	4	Add Runoff "				
"		0.485	1.310	0.325	0.325"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.485	1.310	1.310	0.325"	
" 40		HYDROGRAPH Combine 1"				
"	6	Combine "				
"	1	Node #"				
"		Pond Inlet 1"				
"		Maximum flow		1.310	c.m/sec"	
"		Hydrograph volume		25393.383	c.m"	
"		0.485	1.310	1.310	1.310"	
" 40		HYDROGRAPH Confluence 1"				
"	7	Confluence "				
"	1	Node #"				
"		Pond Inlet 1"				
"		Maximum flow		1.310	c.m/sec"	
"		Hydrograph volume		25393.383	c.m"	
"		0.485	1.310	1.310	0.000"	
" 40		HYDROGRAPH Copy to Outflow"				
"	8	Copy to Outflow"				
"		0.485	1.310	1.310	0.000"	
" 40		HYDROGRAPH Combine 1000"				
"	6	Combine "				
"	1000	Node #"				
"		Pond"				
"		Maximum flow		1.310	c.m/sec"	
"		Hydrograph volume		25393.383	c.m"	
"		0.485	1.310	1.310	1.310"	
" 40		HYDROGRAPH Confluence 3"				
"	7	Confluence "				
"	3	Node #"				
"		Pond Inlet 3"				
"		Maximum flow		1.153	c.m/sec"	
"		Hydrograph volume		23420.096	c.m"	
"		0.485	1.153	1.310	0.000"	

" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.485 1.153 1.153			0.000"	
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		2.463		c.m/sec"
"	Hydrograph volume		48813.477		c.m"
"	0.485 1.153 1.153			2.463"	
" 40	HYDROGRAPH Confluence 2"				
"	7 Confluence "				
"	2 Node #"				
"	Pond Inlet 2"				
"	Maximum flow		0.325		c.m/sec"
"	Hydrograph volume		5669.517		c.m"
"	0.485 0.325 1.153			0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.485 0.325 0.325			0.000"	
" 40	HYDROGRAPH Combine 1000"				
"	6 Combine "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		2.789		c.m/sec"
"	Hydrograph volume		54482.996		c.m"
"	0.485 0.325 0.325			2.789"	
" 40	HYDROGRAPH Confluence 1000"				
"	7 Confluence "				
"	1000 Node #"				
"	Pond"				
"	Maximum flow		2.789		c.m/sec"
"	Hydrograph volume		54482.996		c.m"
"	0.485 2.789 0.325			0.000"	
" 40	HYDROGRAPH Copy to Outflow"				
"	8 Copy to Outflow"				
"	0.485 2.789 2.789			0.000"	
" 40	HYDROGRAPH Next link "				
"	5 Next link "				
"	0.485 2.789 2.789			0.000"	
" 33	CATCHMENT 206"				
"	1 Triangular SCS"				
"	1 Equal length"				
"	2 Horton equation"				
"	206 Pond"				
"	80.000 % Impervious"				
"	3.290 Total Area"				
"	100.000 Flow length"				
"	4.000 Overland Slope"				
"	0.658 Pervious Area"				

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"      100.000  Pervious length"
"      2.000   Pervious slope"
"      2.632   Impervious Area"
"     100.000  Impervious length"
"      2.000   Impervious slope"
"      0.250   Pervious Manning 'n'"
"     75.000   Pervious Max.infiltration"
"      5.000   Pervious Min.infiltration"
"      0.250   Pervious Lag constant (hours)"
"      5.000   Pervious Depression storage"
"      0.015   Impervious Manning 'n'"
"      0.000   Impervious Max.infiltration"
"      0.000   Impervious Min.infiltration"
"      0.050   Impervious Lag constant (hours)"
"      1.500   Impervious Depression storage"
"          0.389      2.789      2.789      0.000 c.m/sec"
"      Catchment 206      Pervious      Impervious Total Area  "
"      Surface Area      0.658      2.632      3.290      hectare"
"      Time of concentration 33.142      5.889      9.274      minutes"
"      Time to Centroid      2737.929      2233.505      2296.156      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1875.30      7501.20      9376.50      c.m"
"      Rainfall losses      139.790      29.018      51.172      mm"
"      Runoff depth      145.210      255.982      233.828      mm"
"      Runoff volume      955.48      6737.44      7692.93      c.m"
"      Runoff coefficient      0.510      0.898      0.820      "
"      Maximum flow      0.072      0.333      0.389      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.389      3.178      2.789      0.000"
" 54      POND DESIGN"
"      3.178      Current peak flow      c.m/sec"
"      0.206      Target outflow      c.m/sec"
"     62175.9      Hydrograph volume      c.m"
"      17.      Number of stages"
"     456.800      Minimum water level      metre"
"     458.000      Maximum water level      metre"
"     456.800      Starting water level      metre"
"      0      Keep Design Data: 1 = True; 0 = False"
"          Level Discharge      Volume"
"     456.000      0.000      0.000"
"     456.100      0.01500      680.300"
"     456.200      0.02900      1395.900"
"     456.300      0.04200      2147.500"
"     456.400      0.05200      2935.400"
"     456.480      0.05900      3592.400"
"     456.500      0.06000      3760.400"
"     456.600      0.2840      4627.400"
"     456.700      0.7630      5539.800"
"     456.800      1.491      6496.900"

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"          456.900      2.386  7500.300"
"          457.000      2.490  8551.600"
"          457.100      6.036  9652.000"
"          457.200     12.632 10803.10"
"          457.300     21.379 12006.50"
"          457.400     31.960 13275.50"
"          457.500     44.189 14711.10"
"          Peak outflow          2.808   c.m/sec"
"          Maximum level        457.012  metre"
"          Maximum storage      8684.418   c.m"
"          Centroidal lag       43.065   hours"
"          0.389    3.178    2.808    0.000 c.m/sec"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.389    2.808    2.808    0.000"
" 56 DIVERSION"
"      2000 Node number"
"      2.458 Overflow threshold"
"      1.000 Required diverted fraction"
"      0 Conduit type; 1=Pipe;2=Channel"
"          Peak of diverted flow    0.350   c.m/sec"
"          Volume of diverted flow  1259.027 c.m"
"          DIV02000.9999hyd"
"          Divert Weir 1 and 2 away from Dispersion Trench"
"          0.389    2.808    2.458    0.000 c.m/sec"
" 81 ADD COMMENT=====
"      1 Lines of comment"
"          Divert Flow from Weirs 1 and 2 away from Dispersion Trench"
" 40 HYDROGRAPH Combine 4000"
"      6 Combine "
"      4000 Node #"
"          Site"
"          Maximum flow          2.458   c.m/sec"
"          Hydrograph volume     61031.441 c.m"
"          0.389    2.808    2.458    2.458"
" 40 HYDROGRAPH Next link "
"      5 Next link "
"          0.389    2.458    2.458    2.458"
" 54 POND DESIGN"
"      2.458 Current peak flow   c.m/sec"
"      0.206 Target outflow   c.m/sec"
"      61031.4 Hydrograph volume c.m"
"      9. Number of stages"
"      456.800 Minimum water level  metre"
"      458.000 Maximum water level  metre"
"      456.800 Starting water level  metre"
"      0 Keep Design Data: 1 = True; 0 = False"
"          Level Discharge Volume"
"          454.860    0.000    0.000"
"          454.960  1.20E-05    4.000"

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"           455.060  1.20E-05    8.000"
"           455.160  1.20E-05   12.000"
"           455.260  1.20E-05   16.000"
"           455.360  1.20E-05   20.000"
"           455.460    1.702   20.500"
"           455.560    4.979   21.000"
"           455.660    9.395   21.500"
"           Peak outflow                2.458   c.m/sec"
"           Maximum level                455.483  metre"
"           Maximum storage              20.615   c.m"
"           Centroidal lag              43.075   hours"
"           0.389    2.458    2.458    2.458 c.m/sec"
" 40 HYDROGRAPH Next link "
"           5 Next link "
"           0.389    2.458    2.458    2.458"
" 56 DIVERSION"
"           3000 Node number"
"           0.000 Overflow threshold"
"           0.395 Computed diverted fraction"
"           0 Conduit type; 1=Pipe;2=Channel"
"           Peak of diverted flow        0.971   c.m/sec"
"           Volume of diverted flow     24097.531 c.m"
"           DIV03000.9999hyd"
"           Major flow at 3000"
"           0.389    2.458    1.487    2.458 c.m/sec"
" 81 ADD COMMENT=====
"           1 Lines of comment"
"           Diverted to existing wetland on site"
" 40 HYDROGRAPH Combine 4000"
"           6 Combine "
"           4000 Node #"
"           Site"
"           Maximum flow                3.945   c.m/sec"
"           Hydrograph volume          97949.086 c.m"
"           0.389    2.458    1.487    3.945"
" 40 HYDROGRAPH Start - New Tributary"
"           2 Start - New Tributary"
"           0.389    0.000    1.487    3.945"
" 33 CATCHMENT 200"
"           1 Triangular SCS"
"           1 Equal length"
"           2 Horton equation"
"           200 Rear yard and Open Space to Grand River"
"           10.000 % Impervious"
"           7.530 Total Area"
"           100.000 Flow length"
"           5.000 Overland Slope"
"           6.777 Pervious Area"
"           100.000 Pervious length"
"           2.000 Pervious slope"

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"      0.753  Impervious Area"
" 100.000  Impervious length"
"      2.000  Impervious slope"
"      0.250  Pervious Manning 'n'"
" 75.000  Pervious Max.infiltration"
"      5.000  Pervious Min.infiltration"
"      0.250  Pervious Lag constant (hours)"
"      5.000  Pervious Depression storage"
"      0.015  Impervious Manning 'n'"
"      0.000  Impervious Max.infiltration"
"      0.000  Impervious Min.infiltration"
"      0.050  Impervious Lag constant (hours)"
"      1.500  Impervious Depression storage"
"          0.809      0.000      1.487      3.945 c.m/sec"
"      Catchment 200      Pervious      Impervious      Total Area  "
"      Surface Area      6.777      0.753      7.530      hectare"
"      Time of concentration  33.142      5.889      28.678      minutes"
"      Time to Centroid      2737.929      2233.505      2655.311      minutes"
"      Rainfall depth      285.000      285.000      285.000      mm"
"      Rainfall volume      1.9314      0.2146      2.1461      ha-m"
"      Rainfall losses      139.790      29.018      128.713      mm"
"      Runoff depth      145.210      255.982      156.287      mm"
"      Runoff volume      0.9841      0.1928      1.1768      ha-m"
"      Runoff coefficient      0.510      0.898      0.548      "
"      Maximum flow      0.744      0.095      0.809      c.m/sec"
" 40      HYDROGRAPH Add Runoff  "
"      4      Add Runoff  "
"          0.809      0.809      1.487      3.945"
" 40      HYDROGRAPH Copy to Outflow"
"      8      Copy to Outflow"
"          0.809      0.809      0.809      3.945"
" 40      HYDROGRAPH  Combine  4000"
"      6      Combine  "
"      4000  Node #"
"      Site"
"      Maximum flow      4.754      c.m/sec"
"      Hydrograph volume      109717.539      c.m"
"          0.809      0.809      0.809      4.754"
" 40      HYDROGRAPH Start - New Tributary"
"      2      Start - New Tributary"
"          0.809      0.000      0.809      4.754"
" 47      FILEI_0 Read/Open DIV03000.9999hyd"
"      1      1=read/open; 2=write/save"
"      2      1=rainfall; 2=hydrograph"
"      1      1=runoff; 2=inflow; 3=outflow; 4=junction"
"      DIV03000.9999hyd"
"      Major flow at 3000"
"      Total volume      24097.531      c.m"
"      Maximum flow      0.971      c.m/sec"
"          0.971      0.000      0.809      4.754 c.m/sec"

```

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" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.971      0.971      0.809      4.754"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.971      0.971      0.971      4.754"
" 40      HYDROGRAPH  Combine  4000"
"          6  Combine "
" 4000  Node #"
"          Site"
"          Maximum flow              5.725      c.m/sec"
"          Hydrograph volume          133815.031  c.m"
"              0.971      0.971      0.971      5.725"
" 40      HYDROGRAPH Start - New Tributary"
"          2  Start - New Tributary"
"              0.971      0.000      0.971      5.725"
" 47      FILEI_0 Read/Open DIV02000.9999hyd"
"          1  1=read/open; 2=write/save"
"          2  1=rainfall; 2=hydrograph"
"          1  1=runoff; 2=inflow; 3=outflow; 4=junction"
"          DIV02000.9999hyd"
"          Divert Weir 1 and 2 away from Dispersion Trench"
"          Total volume              1259.027      c.m"
"          Maximum flow              0.350      c.m/sec"
"              0.350      0.000      0.971      5.725 c.m/sec"
" 40      HYDROGRAPH Add Runoff "
"          4  Add Runoff "
"              0.350      0.350      0.971      5.725"
" 40      HYDROGRAPH Copy to Outflow"
"          8  Copy to Outflow"
"              0.350      0.350      0.350      5.725"
" 40      HYDROGRAPH  Combine  4000"
"          6  Combine "
" 4000  Node #"
"          Site"
"          Maximum flow              6.075      c.m/sec"
"          Hydrograph volume          135074.063  c.m"
"              0.350      0.350      0.350      6.075"
" 40      HYDROGRAPH  Confluence  4000"
"          7  Confluence "
" 4000  Node #"
"          Site"
"          Maximum flow              6.075      c.m/sec"
"          Hydrograph volume          135074.063  c.m"
"              0.350      6.075      0.350      0.000"
" 81      ADD COMMENT=====
"          1  Lines of comment"
"          Total flow to Grand River"
" 38      START/RE-START TOTALS 4000"
"          3  Runoff Totals on EXIT"

```


"	Total Catchment area	36.330	hectare"
"	Total Impervious area	18.751	hectare"
"	Total % impervious	51.612"	
" 19	EXIT"		



APPENDIX E

Oil/Grit Separator Sizing



**River's Edge Subdivision
Town of Grand Valley
File No. 104-104**

Oil/Grit Separator Drainage Area

Inlet 1

Catchment	Area (ha)	%Imp
204	2.09	10%
205	4.32	65%
303	3.55	55%
Total	9.96	50%

Inlet 2

Catchment	Area (ha)	%Imp
203	2.76	60%
Total	2.76	60%

Inlet 3

Catchment	Area (ha)	%Imp
201	4.28	90%
202	4.15	60%
302	0.67	55%
304	1.09	55%
Total	10.19	72%

Stormceptor® EF Sizing Report

**STORMCEPTOR®
ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION**

02/22/2023

Province:	Ontario
City:	Grand Valley
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	River's Edge Subdivision
Project Number:	104104
Designer Name:	Sergio Zaga
Designer Company:	GM BluePlan Engineering Limited
Designer Email:	sergio.zaga@gmblueplan.ca
Designer Phone:	519-824-8150
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Inlet 1
------------	---------

Drainage Area (ha):	9.96
% Imperviousness:	50.00

Runoff Coefficient 'c': 0.60

Particle Size Distribution:	Fine
Target TSS Removal (%):	60.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	185.84
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	36
EFO6	52
EFO8	64
EFO10	73
EFO12	78

Recommended Stormceptor EFO Model: EFO8
Estimated Net Annual Sediment (TSS) Load Reduction (%): 64
Water Quality Runoff Volume Capture (%): > 90

Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

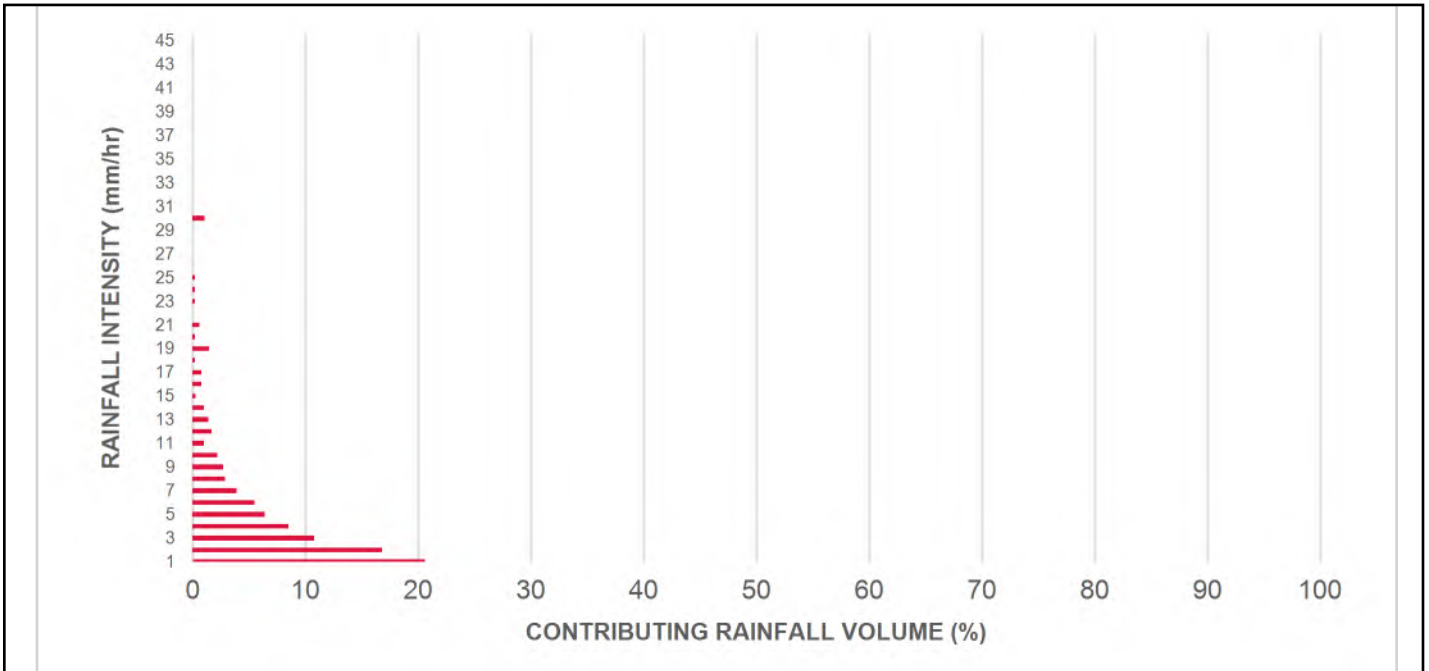
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m ²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	8.31	498.0	106.0	96	8.2	8.2
1	20.6	29.1	16.61	997.0	212.0	83	17.0	25.2
2	16.8	45.9	33.23	1994.0	424.0	73	12.3	37.5
3	10.8	56.7	49.84	2990.0	636.0	64	6.9	44.4
4	8.5	65.2	66.45	3987.0	848.0	63	5.3	49.7
5	6.4	71.6	83.07	4984.0	1060.0	60	3.9	53.5
6	5.5	77.0	99.68	5981.0	1273.0	55	3.0	56.6
7	3.9	81.0	116.29	6978.0	1485.0	49	2.0	58.5
8	2.9	83.9	132.91	7974.0	1697.0	43	1.3	59.8
9	2.7	86.5	149.52	8971.0	1909.0	39	1.0	60.8
10	2.2	88.7	166.13	9968.0	2121.0	35	0.8	61.6
11	1.0	89.7	182.75	10965.0	2333.0	31	0.3	61.9
12	1.7	91.3	199.36	11962.0	2545.0	29	0.5	62.3
13	1.4	92.8	215.97	12958.0	2757.0	27	0.4	62.7
14	1.0	93.7	232.59	13955.0	2969.0	25	0.2	63.0
15	0.3	94.0	249.20	14952.0	3181.0	24	0.1	63.0
16	0.8	94.8	265.81	15949.0	3393.0	22	0.2	63.2
17	0.8	95.7	282.43	16946.0	3605.0	20	0.2	63.4
18	0.2	95.8	299.04	17942.0	3818.0	19	0.0	63.4
19	1.5	97.3	315.65	18939.0	4030.0	18	0.3	63.7
20	0.2	97.5	332.27	19936.0	4242.0	17	0.0	63.7
21	0.6	98.2	348.88	20933.0	4454.0	17	0.1	63.8
22	0.0	98.2	365.49	21930.0	4666.0	16	0.0	63.8
23	0.2	98.4	382.11	22926.0	4878.0	15	0.0	63.9
24	0.2	98.6	398.72	23923.0	5090.0	15	0.0	63.9
25	0.2	98.9	415.33	24920.0	5302.0	14	0.0	63.9
30	1.1	100.0	498.40	29904.0	6363.0	12	0.1	64.1
35	0.0	100.0	581.46	34888.0	7423.0	10	0.0	64.1
40	0.0	100.0	664.53	39872.0	8483.0	9	0.0	64.1
45	0.0	100.0	747.60	44856.0	9544.0	8	0.0	64.1
Estimated Net Annual Sediment (TSS) Load Reduction =								64 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

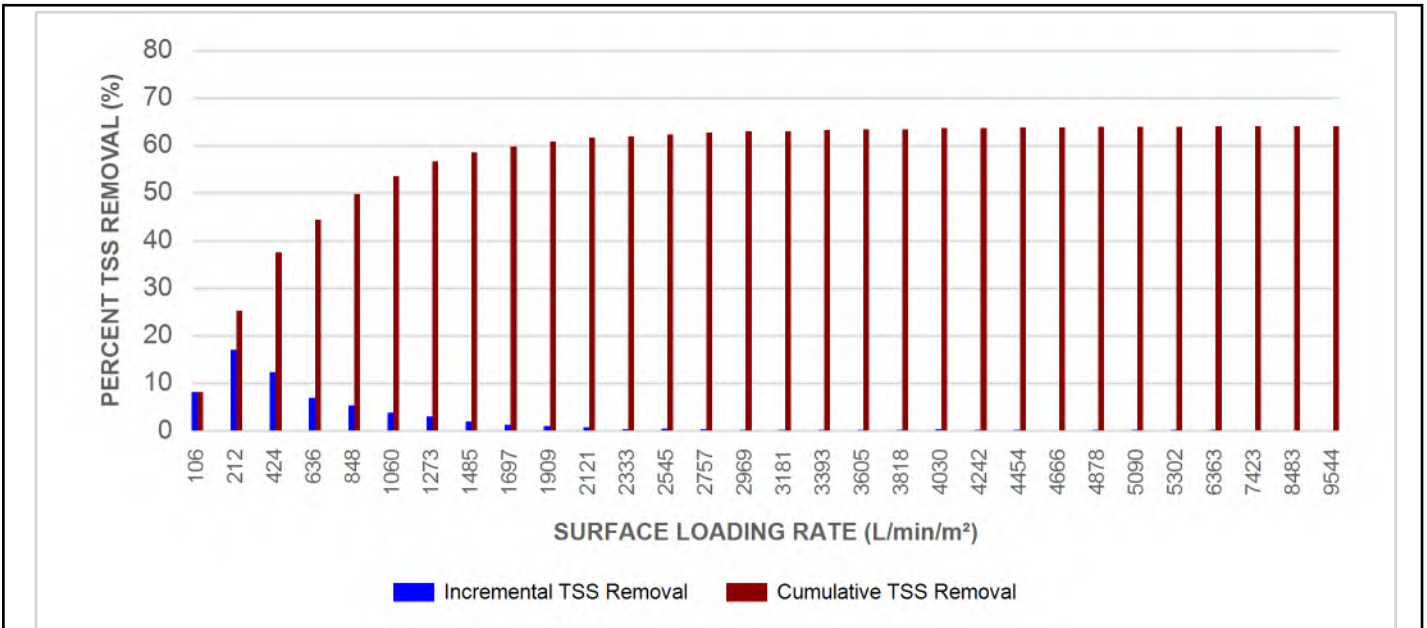


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

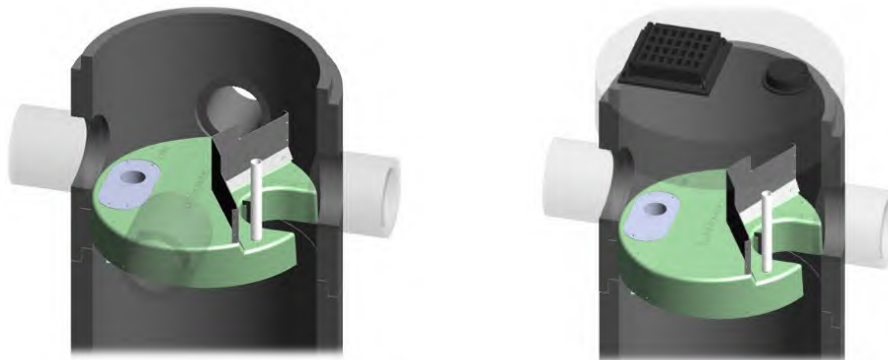
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

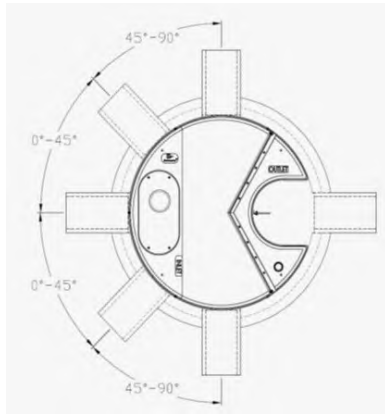
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® EF Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor® EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

09/15/2022

Province:	Ontario
City:	Grand Valley
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	River's Edge Subdivision
Project Number:	104104
Designer Name:	Sergio Zaga
Designer Company:	GM BluePlan Engineering Limited
Designer Email:	sergio.zaga@gmblueplan.ca
Designer Phone:	519-824-8150
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Inlet 2
------------	---------

Drainage Area (ha):	2.76
% Imperviousness:	60.00

Runoff Coefficient 'c': 0.66

Particle Size Distribution:	Fine
Target TSS Removal (%):	60.0

Required Water Quality Runoff Volume Capture (%):	90.00
Estimated Water Quality Flow Rate (L/s):	56.65
Oil / Fuel Spill Risk Site?	Yes
Upstream Flow Control?	No
Peak Conveyance (maximum) Flow Rate (L/s):	
Site Sediment Transport Rate (kg/ha/yr):	

Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	61
EFO6	75
EFO8	84
EFO10	89
EFO12	93

Recommended Stormceptor EFO Model: EFO4
Estimated Net Annual Sediment (TSS) Load Reduction (%): 61
Water Quality Runoff Volume Capture (%): > 90



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

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PERFORMANCE

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PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5



Stormceptor® EF Sizing Report

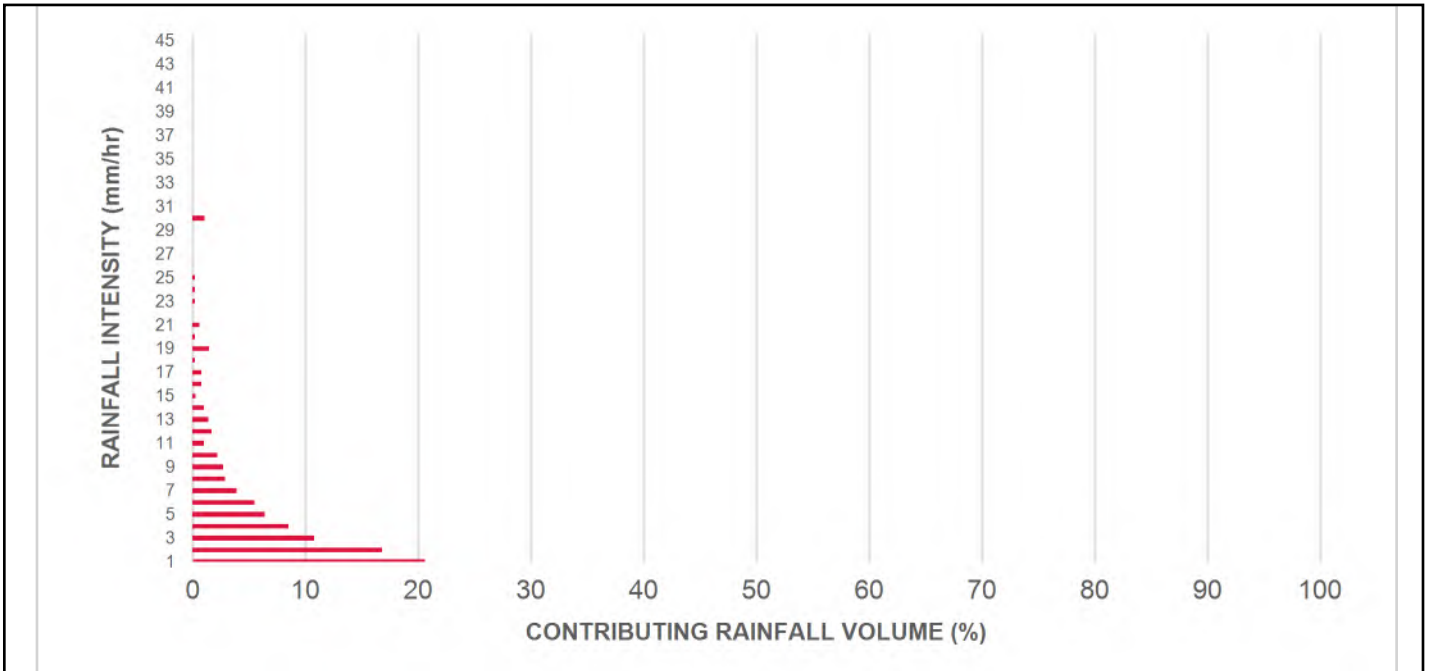
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m ²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	2.53	152.0	127.0	93	8.0	8.0
1	20.6	29.1	5.06	304.0	253.0	81	16.7	24.6
2	16.8	45.9	10.13	608.0	506.0	69	11.6	36.2
3	10.8	56.7	15.19	912.0	760.0	63	6.8	43.1
4	8.5	65.2	20.26	1215.0	1013.0	61	5.2	48.2
5	6.4	71.6	25.32	1519.0	1266.0	56	3.6	51.8
6	5.5	77.0	30.38	1823.0	1519.0	48	2.6	54.4
7	3.9	81.0	35.45	2127.0	1772.0	41	1.6	56.1
8	2.9	83.9	40.51	2431.0	2026.0	36	1.0	57.1
9	2.7	86.5	45.58	2735.0	2279.0	32	0.9	58.0
10	2.2	88.7	50.64	3038.0	2532.0	29	0.6	58.6
11	1.0	89.7	55.70	3342.0	2785.0	27	0.3	58.9
12	1.7	91.3	60.77	3646.0	3038.0	24	0.4	59.3
13	1.4	92.8	65.83	3950.0	3292.0	23	0.3	59.6
14	1.0	93.7	70.90	4254.0	3545.0	21	0.2	59.8
15	0.3	94.0	75.96	4558.0	3798.0	20	0.1	59.9
16	0.8	94.8	81.02	4861.0	4051.0	18	0.1	60.0
17	0.8	95.7	86.09	5165.0	4304.0	17	0.1	60.1
18	0.2	95.8	91.15	5469.0	4558.0	16	0.0	60.2
19	1.5	97.3	96.22	5773.0	4811.0	15	0.2	60.4
20	0.2	97.5	101.28	6077.0	5064.0	15	0.0	60.4
21	0.6	98.2	106.35	6381.0	5317.0	14	0.1	60.5
22	0.0	98.2	111.41	6685.0	5570.0	13	0.0	60.5
23	0.2	98.4	116.47	6988.0	5824.0	13	0.0	60.5
24	0.2	98.6	121.54	7292.0	6077.0	12	0.0	60.6
25	0.2	98.9	126.60	7596.0	6330.0	12	0.0	60.6
30	1.1	100.0	151.92	9115.0	7596.0	10	0.1	60.7
35	0.0	100.0	177.24	10635.0	8862.0	8	0.0	60.7
40	0.0	100.0	202.56	12154.0	10128.0	7	0.0	60.7
45	0.0	100.0	227.88	13673.0	11394.0	7	0.0	60.7
Estimated Net Annual Sediment (TSS) Load Reduction =								61 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

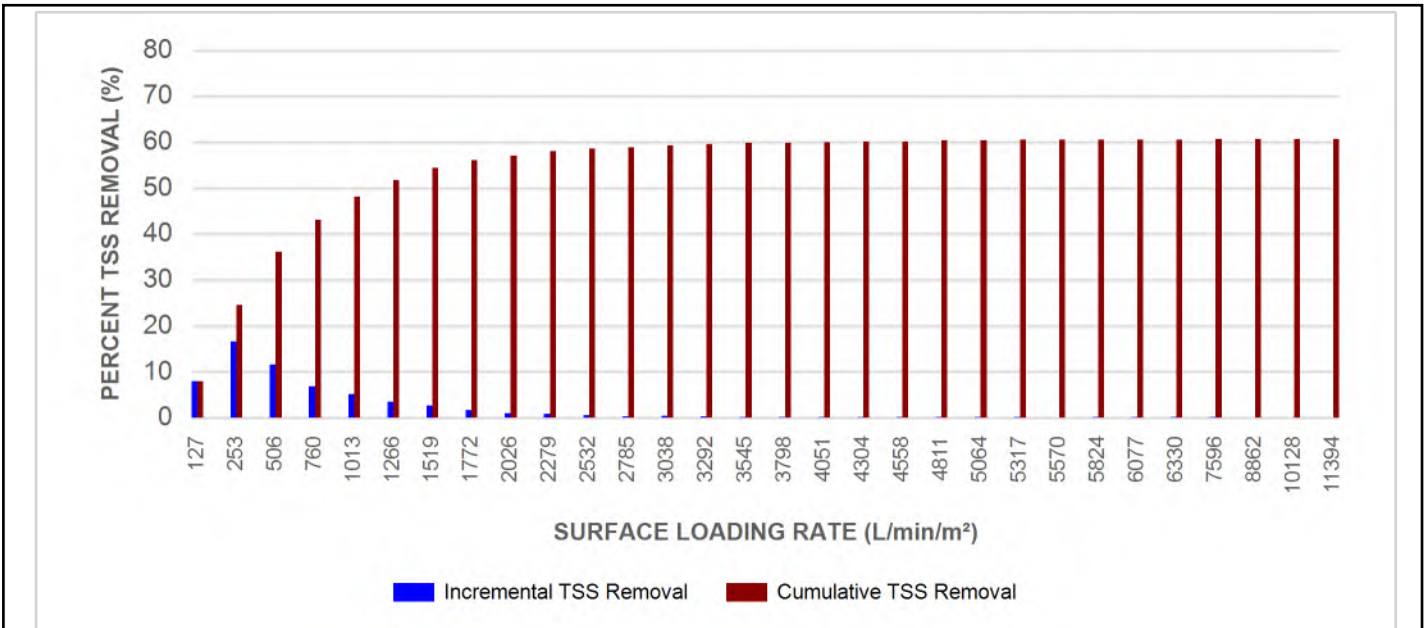


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

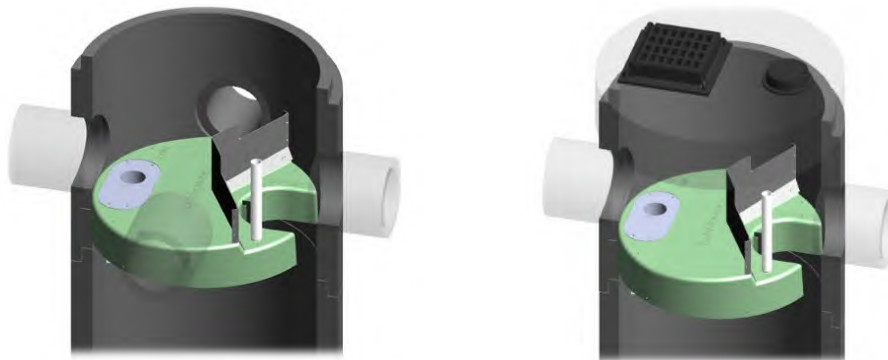
► Stormceptor® EF and EFO feature an internal bypass and superior scour prevention technology that have been demonstrated in third-party testing according to the scour testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**, and the exceptional scour test performance has been third-party verified in accordance with the ISO 14034 ETV protocol. As a result, Stormceptor EF and EFO are approved for online installation, eliminating the need for costly additional bypass structures, piping, and installation expense.

DESIGN FLEXIBILITY

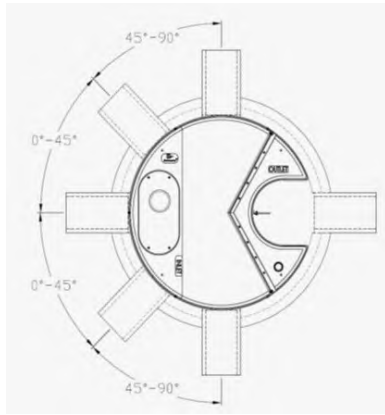
► Stormceptor® EF and EFO offers design flexibility in one simplified platform, accepting stormwater flow from a single inlet pipe or multiple inlet pipes, and/or surface runoff through an inlet grate. The device can also serve as a junction structure, accommodate a 90-degree inlet-to-outlet bend angle, and can be modified to ensure performance in submerged conditions.

OIL CAPTURE AND RETENTION

► While Stormceptor® EF will capture and retain oil from dry weather spills and low intensity runoff, Stormceptor® EFO has demonstrated superior oil capture and greater than 99% oil retention in third-party testing according to the light liquid re-entrainment testing provisions of the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators**. Stormceptor EFO is recommended for sites where oil capture and retention is a requirement.



Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

0° - 45° : The inlet pipe is 1-inch (25mm) higher than the outlet pipe.

45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft ³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



Stormceptor® EF Sizing Report

remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

Stormceptor® **EF** Sizing Report

assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.

Stormceptor® EF Sizing Report

STORMCEPTOR®

ESTIMATED NET ANNUAL SEDIMENT (TSS) LOAD REDUCTION

09/15/2022

Province:	Ontario
City:	Grand Valley
Nearest Rainfall Station:	TORONTO INTL AP
Climate Station Id:	6158731
Years of Rainfall Data:	20

Project Name:	River's Edge Subdivision
Project Number:	104104
Designer Name:	Sergio Zaga
Designer Company:	GM BluePlan Engineering Limited
Designer Email:	sergio.zaga@gmblueplan.ca
Designer Phone:	519-824-8150
EOR Name:	
EOR Company:	
EOR Email:	
EOR Phone:	

Site Name:	Inlet 3
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Drainage Area (ha):	10.19
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% Imperviousness:	72.00
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Runoff Coefficient 'c': 0.73

Particle Size Distribution:	Fine
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Target TSS Removal (%):	60.0
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Required Water Quality Runoff Volume Capture (%):	90.00
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Estimated Water Quality Flow Rate (L/s):	182.50
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Oil / Fuel Spill Risk Site?	Yes
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Upstream Flow Control?	No
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Peak Conveyance (maximum) Flow Rate (L/s):	
--	--

Site Sediment Transport Rate (kg/ha/yr):	
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Net Annual Sediment (TSS) Load Reduction Sizing Summary	
Stormceptor Model	TSS Removal Provided (%)
EFO4	32
EFO6	48
EFO8	60
EFO10	68
EFO12	75

Recommended Stormceptor EFO Model: EFO8

Estimated Net Annual Sediment (TSS) Load Reduction (%): 60

Water Quality Runoff Volume Capture (%): > 90



Stormceptor® EF Sizing Report

THIRD-PARTY TESTING AND VERIFICATION

► **Stormceptor® EF and Stormceptor® EFO** are the latest evolutions in the Stormceptor® oil-grit separator (OGS) technology series, and are designed to remove a wide variety of pollutants from stormwater and snowmelt runoff. These technologies have been third-party tested in accordance with the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** and performance has been third-party verified in accordance with the **ISO 14034 Environmental Technology Verification (ETV)** protocol.

PERFORMANCE

► **Stormceptor® EF and EFO** remove stormwater pollutants through gravity separation and floatation, and feature a patent-pending design that generates positive removal of total suspended solids (TSS) throughout each storm event, including high-intensity storms. Captured pollutants include sediment, free oils, and sediment-bound pollutants such as nutrients, heavy metals, and petroleum hydrocarbons. Stormceptor is sized to remove a high level of TSS from the frequent rainfall events that contribute the vast majority of annual runoff volume and pollutant load. The technology incorporates an internal bypass to convey excessive stormwater flows from high-intensity storms through the device without resuspension and washout (scour) of previously captured pollutants. Proper routine maintenance ensures high pollutant removal performance and protection of downstream waterways.

PARTICLE SIZE DISTRIBUTION (PSD)

► The **Canadian ETV PSD** shown in the table below was used, or in part, for this sizing. This is the identical PSD that is referenced in the Canadian ETV **Procedure for Laboratory Testing of Oil-Grit Separators** for both sediment removal testing and scour testing. The Canadian ETV PSD contains a wide range of particle sizes in the sand and silt fractions, and is considered reasonably representative of the particle size fractions found in typical urban stormwater runoff.

Particle Size (µm)	Percent Less Than	Particle Size Fraction (µm)	Percent
1000	100	500-1000	5
500	95	250-500	5
250	90	150-250	15
150	75	100-150	15
100	60	75-100	10
75	50	50-75	5
50	45	20-50	10
20	35	8-20	15
8	20	5-8	10
5	10	2-5	5
2	5	<2	5

Stormceptor® EF Sizing Report

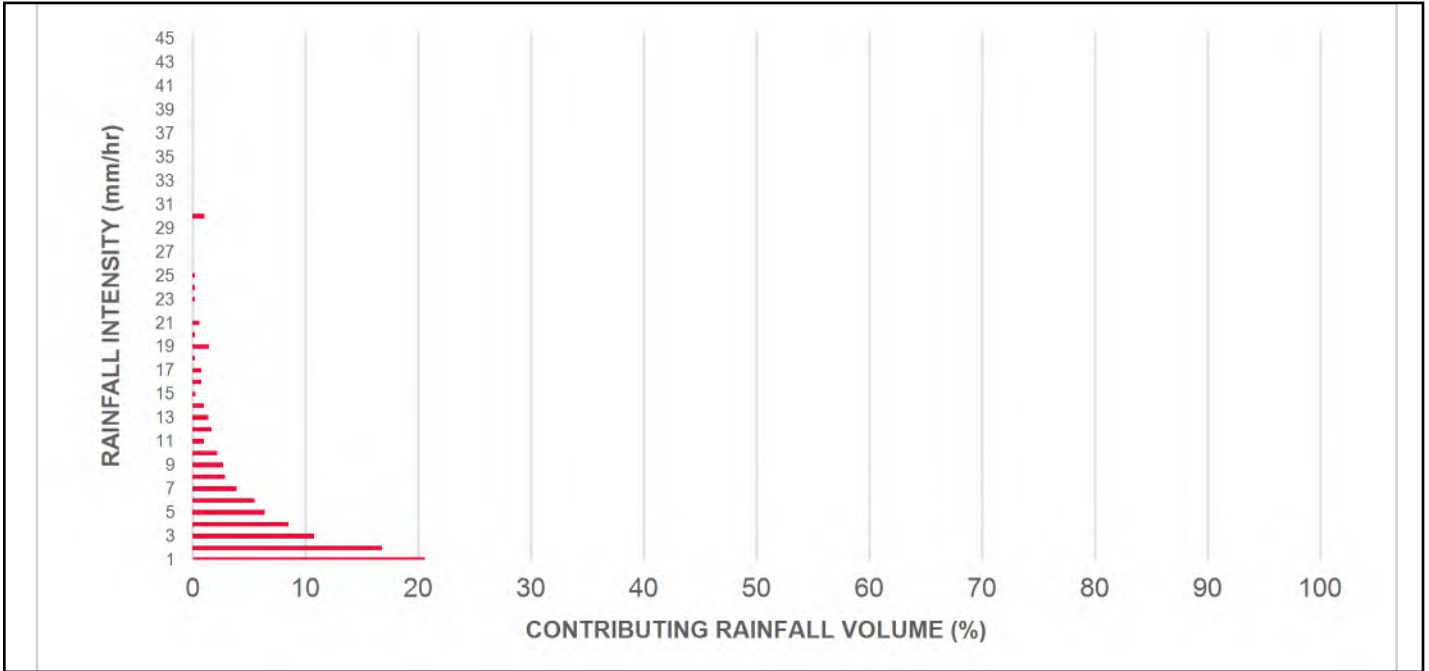
Rainfall Intensity (mm / hr)	Percent Rainfall Volume (%)	Cumulative Rainfall Volume (%)	Flow Rate (L/s)	Flow Rate (L/min)	Surface Loading Rate (L/min/m ²)	Removal Efficiency (%)	Incremental Removal (%)	Cumulative Removal (%)
0.5	8.5	8.5	10.37	622.0	132.0	92	7.8	7.8
1	20.6	29.1	20.74	1244.0	265.0	80	16.6	24.4
2	16.8	45.9	41.47	2488.0	529.0	68	11.4	35.8
3	10.8	56.7	62.21	3733.0	794.0	63	6.8	42.6
4	8.5	65.2	82.94	4977.0	1059.0	60	5.1	47.7
5	6.4	71.6	103.68	6221.0	1324.0	54	3.5	51.2
6	5.5	77.0	124.42	7465.0	1588.0	46	2.5	53.7
7	3.9	81.0	145.15	8709.0	1853.0	40	1.6	55.3
8	2.9	83.9	165.89	9953.0	2118.0	35	1.0	56.3
9	2.7	86.5	186.63	11198.0	2382.0	31	0.8	57.1
10	2.2	88.7	207.36	12442.0	2647.0	28	0.6	57.7
11	1.0	89.7	228.10	13686.0	2912.0	25	0.2	58.0
12	1.7	91.3	248.83	14930.0	3177.0	24	0.4	58.3
13	1.4	92.8	269.57	16174.0	3441.0	22	0.3	58.6
14	1.0	93.7	290.31	17418.0	3706.0	20	0.2	58.8
15	0.3	94.0	311.04	18663.0	3971.0	19	0.1	58.9
16	0.8	94.8	331.78	19907.0	4235.0	17	0.1	59.0
17	0.8	95.7	352.52	21151.0	4500.0	16	0.1	59.2
18	0.2	95.8	373.25	22395.0	4765.0	16	0.0	59.2
19	1.5	97.3	393.99	23639.0	5030.0	15	0.2	59.4
20	0.2	97.5	414.72	24883.0	5294.0	14	0.0	59.4
21	0.6	98.2	435.46	26128.0	5559.0	13	0.1	59.5
22	0.0	98.2	456.20	27372.0	5824.0	13	0.0	59.5
23	0.2	98.4	476.93	28616.0	6089.0	12	0.0	59.6
24	0.2	98.6	497.67	29860.0	6353.0	12	0.0	59.6
25	0.2	98.9	518.41	31104.0	6618.0	11	0.0	59.6
30	1.1	100.0	622.09	37325.0	7942.0	9	0.1	59.7
35	0.0	100.0	725.77	43546.0	9265.0	8	0.0	59.7
40	0.0	100.0	829.45	49767.0	10589.0	7	0.0	59.7
45	0.0	100.0	933.13	55988.0	11912.0	7	0.0	59.7
Estimated Net Annual Sediment (TSS) Load Reduction =								60 %

Climate Station ID: 6158731 Years of Rainfall Data: 20

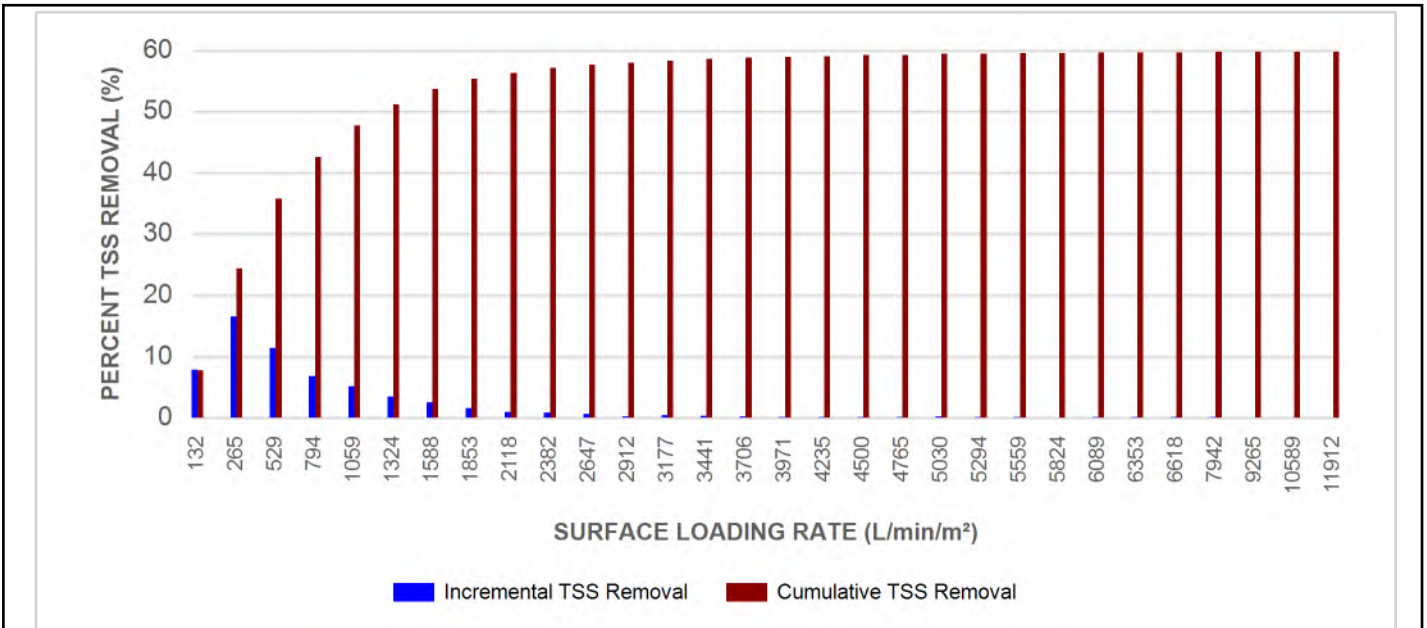


Stormceptor® EF Sizing Report

RAINFALL DATA FROM TORONTO INTL AP RAINFALL STATION



INCREMENTAL AND CUMULATIVE TSS REMOVAL FOR THE RECOMMENDED STORMCEPTOR® MODEL



Stormceptor® **EF** Sizing Report

Maximum Pipe Diameter / Peak Conveyance

Stormceptor EF / EFO	Model Diameter		Min Angle Inlet / Outlet Pipes	Max Inlet Pipe Diameter		Max Outlet Pipe Diameter		Peak Conveyance Flow Rate	
	(m)	(ft)		(mm)	(in)	(mm)	(in)	(L/s)	(cfs)
EF4 / EFO4	1.2	4	90	609	24	609	24	425	15
EF6 / EFO6	1.8	6	90	914	36	914	36	990	35
EF8 / EFO8	2.4	8	90	1219	48	1219	48	1700	60
EF10 / EFO10	3.0	10	90	1828	72	1828	72	2830	100
EF12 / EFO12	3.6	12	90	1828	72	1828	72	2830	100

SCOUR PREVENTION AND ONLINE CONFIGURATION

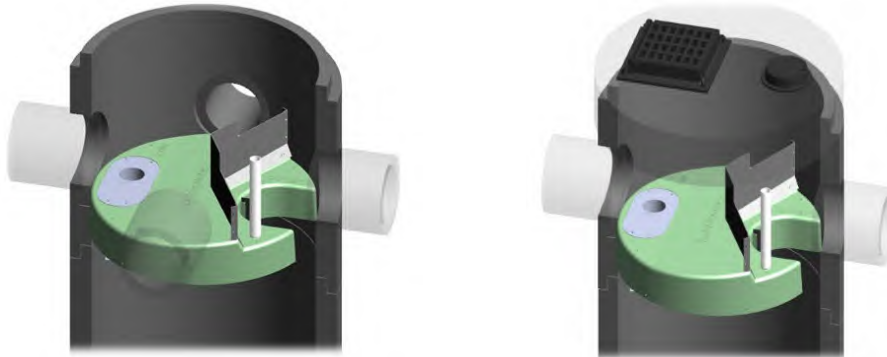
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DESIGN FLEXIBILITY

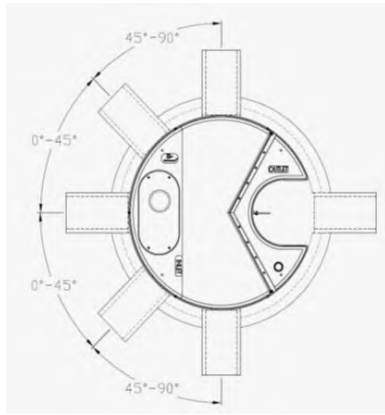
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Stormceptor® EF Sizing Report



INLET-TO-OUTLET DROP

Elevation differential between inlet and outlet pipe inverts is dictated by the angle at which the inlet pipe(s) enters the unit.

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45° - 90° : The inlet pipe is 2-inches (50mm) higher than the outlet pipe.

HEAD LOSS

The head loss through Stormceptor EF is similar to that of a 60-degree bend structure. The applicable K value for calculating minor losses through the unit is 1.1.

For submerged conditions the applicable K value is 3.0.

Pollutant Capacity

Stormceptor EF / EFO	Model Diameter		Depth (Outlet Pipe Invert to Sump Floor)		Oil Volume		Recommended Sediment Maintenance Depth *		Maximum Sediment Volume *		Maximum Sediment Mass **	
	(m)	(ft)	(m)	(ft)	(L)	(Gal)	(mm)	(in)	(L)	(ft³)	(kg)	(lb)
EF4 / EFO4	1.2	4	1.52	5.0	265	70	203	8	1190	42	1904	5250
EF6 / EFO6	1.8	6	1.93	6.3	610	160	305	12	3470	123	5552	15375
EF8 / EFO8	2.4	8	2.59	8.5	1070	280	610	24	8780	310	14048	38750
EF10 / EFO10	3.0	10	3.25	10.7	1670	440	610	24	17790	628	28464	78500
EF12 / EFO12	3.6	12	3.89	12.8	2475	655	610	24	31220	1103	49952	137875

*Increased sump depth may be added to increase sediment storage capacity

** Average density of wet packed sediment in sump = 1.6 kg/L (100 lb/ft³)

Feature	Benefit	Feature Appeals To
Patent-pending enhanced flow treatment and scour prevention technology	Superior, verified third-party performance	Regulator, Specifying & Design Engineer
Third-party verified light liquid capture and retention for EFO version	Proven performance for fuel/oil hotspot locations	Regulator, Specifying & Design Engineer, Site Owner
Functions as bend, junction or inlet structure	Design flexibility	Specifying & Design Engineer
Minimal drop between inlet and outlet	Site installation ease	Contractor
Large diameter outlet riser for inspection and maintenance	Easy maintenance access from grade	Maintenance Contractor & Site Owner

STANDARD STORMCEPTOR EF/EFO DRAWINGS

For standard details, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>

STANDARD STORMCEPTOR EF/EFO SPECIFICATION

For specifications, please visit <http://www.imbriumsystems.com/stormwater-treatment-solutions/stormceptor-ef>



Stormceptor® **EF** Sizing Report

**STANDARD PERFORMANCE SPECIFICATION FOR
“OIL GRIT SEPARATOR” (OGS) STORMWATER QUALITY TREATMENT DEVICE**

PART 1 – GENERAL

1.1 WORK INCLUDED

This section specifies requirements for selecting, sizing, and designing an underground Oil Grit Separator (OGS) device for stormwater quality treatment, with third-party testing results and a Statement of Verification in accordance with ISO 14034 Environmental Management – Environmental Technology Verification (ETV).

1.2 REFERENCE STANDARDS & PROCEDURES

ISO 14034:2016 Environmental management – Environmental technology verification (ETV)

Canadian Environmental Technology Verification (ETV) Program’s **Procedure for Laboratory Testing of Oil-Grit Separators**

1.3 SUBMITTALS

1.3.1 All submittals, including sizing reports & shop drawings, shall be submitted upon request with each order to the contractor then forwarded to the Engineer of Record for review and acceptance. Shop drawings shall detail all OGS components, elevations, and sequence of construction.

1.3.2 Alternative devices shall have features identical to or greater than the specified device, including: treatment chamber diameter, treatment chamber wet volume, sediment storage volume, and oil storage volume.

1.3.3 Unless directed otherwise by the Engineer of Record, OGS stormwater quality treatment product substitutions or alternatives submitted within ten days prior to project bid shall not be accepted. All alternatives or substitutions submitted shall be signed and sealed by a local registered Professional Engineer, based on the exact same criteria detailed in Section 3, in entirety, subject to review and approval by the Engineer of Record.

PART 2 – PRODUCTS

2.1 OGS POLLUTANT STORAGE

The OGS device shall include a sump for sediment storage, and a protected volume for the capture and storage of petroleum hydrocarbons and buoyant gross pollutants. The minimum sediment & petroleum hydrocarbon storage capacity shall be as follows:

2.1.1	4 ft (1219 mm) Diameter OGS Units:	1.19 m ³ sediment / 265 L oil
	6 ft (1829 mm) Diameter OGS Units:	3.48 m ³ sediment / 609 L oil
	8 ft (2438 mm) Diameter OGS Units:	8.78 m ³ sediment / 1,071 L oil
	10 ft (3048 mm) Diameter OGS Units:	17.78 m ³ sediment / 1,673 L oil
	12 ft (3657 mm) Diameter OGS Units:	31.23 m ³ sediment / 2,476 L oil

PART 3 – PERFORMANCE & DESIGN

3.1 GENERAL

The OGS stormwater quality treatment device shall be verified in accordance with ISO 14034:2016 Environmental management – Environmental technology verification (ETV). The OGS stormwater quality treatment device shall



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remove oil, sediment and gross pollutants from stormwater runoff during frequent wet weather events, and retain these pollutants during less frequent high flow wet weather events below the insert within the OGS for later removal during maintenance. The Manufacturer shall have at least ten (10) years of local experience, history and success in engineering design, manufacturing and production and supply of OGS stormwater quality treatment device systems, acceptable to the Engineer of Record.

3.2 SIZING METHODOLOGY

The OGS device shall be engineered, designed and sized to provide stormwater quality treatment based on treating a minimum of 90 percent of the average annual runoff volume and a minimum removal of an annual average 60% of the sediment (TSS) load based on the Particle Size Distribution (PSD) specified in the sizing report for the specified device. Sizing of the OGS shall be determined by use of a minimum ten (10) years of local historical rainfall data provided by Environment Canada. Sizing shall also be determined by use of the sediment removal performance data derived from the ISO 14034 ETV third-party verified laboratory testing data from testing conducted in accordance with the Canadian ETV protocol Procedure for Laboratory Testing of Oil-Grit Separators, as follows:

3.2.1 Sediment removal efficiency for a given surface loading rate and its associated flow rate shall be based on sediment removal efficiency demonstrated at the seven (7) tested surface loading rates specified in the protocol, ranging 40 L/min/m² to 1400 L/min/m², and as stated in the ISO 14034 ETV Verification Statement for the OGS device.

3.2.2 Sediment removal efficiency for surface loading rates between 40 L/min/m² and 1400 L/min/m² shall be based on linear interpolation of data between consecutive tested surface loading rates.

3.2.3 Sediment removal efficiency for surface loading rates less than the lowest tested surface loading rate of 40 L/min/m² shall be assumed to be identical to the sediment removal efficiency at 40 L/min/m². No extrapolation shall be allowed that results in a sediment removal efficiency that is greater than that demonstrated at 40 L/min/m².

3.2.4 Sediment removal efficiency for surface loading rates greater than the highest tested surface loading rate of 1400 L/min/m² shall assume zero sediment removal for the portion of flow that exceeds 1400 L/min/m², and shall be calculated using a simple proportioning formula, with 1400 L/min/m² in the numerator and the higher surface loading rate in the denominator, and multiplying the resulting fraction times the sediment removal efficiency at 1400 L/min/m².

The OGS device shall also have sufficient annual sediment storage capacity as specified and calculated in Section 2.1.

3.3 CANADIAN ETV or ISO 14034 ETV VERIFICATION OF SCOUR TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of third-party scour testing conducted in accordance with the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**.

3.3.1 To be acceptable for on-line installation, the OGS device must demonstrate an average scour test effluent concentration less than 10 mg/L at each surface loading rate tested, up to and including 2600 L/min/m².

3.4 LIGHT LIQUID RE-ENTRAINMENT SIMULATION TESTING

The OGS device shall have Canadian ETV or ISO 14034 ETV Verification of completed third-party Light Liquid Re-entrainment Simulation Testing in accordance with the Canadian ETV **Program's Procedure for Laboratory Testing of Oil-Grit Separators**, with results reported within the Canadian ETV or ISO 14034 ETV verification. This re-entrainment testing is conducted with the device pre-loaded with low density polyethylene (LDPE) plastic beads as a surrogate for light liquids such as oil and fuel. Testing is conducted on the same OGS unit tested for sediment removal to

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assess whether light liquids captured after a spill are effectively retained at high flow rates.

3.4.1 For an OGS device to be an acceptable stormwater treatment device on a site where vehicular traffic occurs and the potential for an oil or fuel spill exists, the OGS device must have reported verified performance results of greater than 99% cumulative retention of LDPE plastic beads for the five specified surface loading rates (ranging 200 L/min/m² to 2600 L/min/m²) in accordance with the Light Liquid Re-entrainment Simulation Testing within the Canadian ETV Program's **Procedure for Laboratory Testing of Oil-Grit Separators**. However, an OGS device shall not be allowed if the Light Liquid Re-entrainment Simulation Testing was performed with screening components within the OGS device that are effective at retaining the LDPE plastic beads, but would not be expected to retain light liquids such as oil and fuel.